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# 1. Introduction

#### 1.1 Overview

This report has been prepared by GHD Pty Ltd (GHD) on behalf of Delbest Pty Ltd to accompany a Part 3A concept application for the development of the Hi Tech Holistic Cancer and Medical Facility "Life City Wollongong".

Life City Wollongong (Life City) is to be situated on Nolan Street, Berkeley, NSW. (refer to Figure 1).

The report discusses the following:

- Existing Conditions a review of existing road features, adjacent developments, traffic volumes, pedestrian facilities, sight distances and crash data;
- Proposed Development

   a review of additional traffic generated from the proposed facility, proposed access/egress arrangements and parking facilities; and
- Development Impact— assessment of sightline distances and the performance of the existing intersections (queues, delays, level of service, safety).

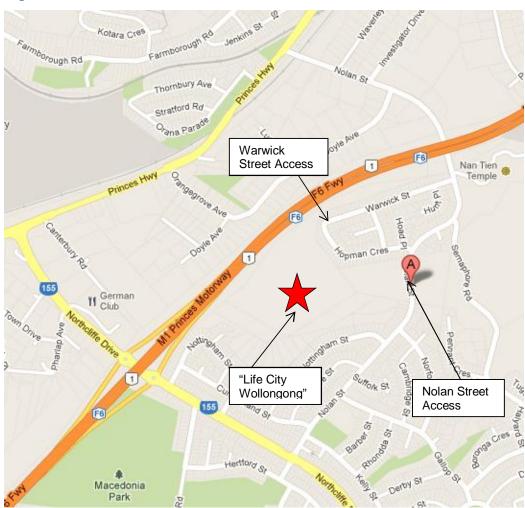


Figure 1 - Site Location

Source: Google Maps, 2012

#### 1.2 Study Scope

This report has been prepared by GHD for TCG Planning and Delbest Pty Ltd and may only be used and relied on by TCG Planning and Delbest Pty Ltd for the purpose agreed as set out in Section 1.1 of this report.

GHD otherwise disclaims responsibility to any person other than TCG Planning and Delbest Pty Ltd arising in connection with this report. GHD also excludes implied warranties and conditions, to the extent legally permissible.

The services undertaken by GHD in connection with preparing this report were limited to those specifically detailed in the report and are subject to the scope limitations set out in the report.

The opinions, conclusions and any recommendations in this report are based on conditions encountered and information reviewed at the date of preparation of the report. GHD has no responsibility or obligation to update this report to account for events or changes occurring subsequent to the date that the report was prepared.

The opinions, conclusions and any recommendations in this report are based on assumptions made by GHD described in Section 1.3 of this report. GHD disclaims liability arising from any of the assumptions being incorrect.

GHD has prepared this report on the basis of information provided by TCG Planning and others who provided information to GHD (including Government authorities), which GHD has not independently verified or checked beyond the agreed scope of work. GHD does not accept liability in connection with such unverified information, including errors and omissions in the report which were caused by errors or omissions in that information.

#### 1.3 Assumptions

This report has been based on the following assumptions:

- Data collection has been limited to traffic volumes surveys at key intersections; trip generation surveys of similar developments have not been undertaken.
- Some of the land uses proposed in the development are not addressed in the RMS Guide; for these, the traffic generation and assumptions used are those provided in the Traffic Impact Assessment undertaken by Bruce Conneeley of Traffic Impact Service Pty Ltd for the Preliminary Environmental Assessment.
- Traffic growth assumptions have been provided by Wollongong City Council in the form of model outputs from the Wollongong Shoalhaven (WOLSH) Strategic Traffic Model.
- Staging and configurations of each stage are based on those provided by TCG Planning and the plans provided by Boss Design dated September 2013, received by GHD on 9 October 2013.

# 2. Existing Conditions

This section outlines the existing conditions around the Life City site including traffic conditions on Nolan Street, Warwick Street, Northcliffe Drive and the Princes Highway. This includes the profile of the development, the existing transport and accessibility conditions and the existing road network performance.

#### 2.1 Existing Road Network Characteristics

The classification of roads on the existing road network can be used as an indication of the functional role each road plays with respect to the volume of traffic they should appropriately carry. The Roads and Maritime Services (RMS) have developed a set of road hierarchy classifications detailed in Table 1 indicating typical nominal volumes expressed in terms of average annual daily traffic (AADT) serviced by various classes of roads.

Table 1 Functional Classification of Roads

Type of Road	Traffic Volume (vpd*)	Peak Hour Volume (vph**)
Arterial Road	>20,000	>2,000
Sub-Arterial Road	10,000 - 20,000	1,000 - 2,000
Collector Road	2,000 - 10,000	200 – 1,000
Local Road	<2,000	0 – 200

Note: \* vpd – vehicles per day

#### 2.1.1 Nolan Street

Nolan Street performs the function of a collector road in Berkeley and has the following key features:

- The posted speed limit is 50 km/h, there is a 40 km/h school zone located on the southern end of Nolan Street; and
- The carriageway is sealed, with one lane marked in each direction.



Photo 1 - Nolan Street north of Warwick Street.

<sup>\*\*</sup> vph - vehicles per hour

#### 2.1.2 Warwick Street

Warwick Street performs the function of a local road in Berkeley and has the following key features:

- The posted speed limit is 50 km/h; and
- The carriageway is sealed approximately 10 metres wide with no road marking.



**Photo 2 -** Warwick Street looking west with the proposed development to be accessed at the western end of Warwick Street.

#### 2.1.3 Northcliffe Drive

Northcliffe Drive performs the function of a sub-arterial road in Berkeley and has the following key features:

- The posted speed limit is 70 km/h; and
- The carriageway is sealed, median divided with two lanes in each direction.



**Photo 3 -** Northcliffe Drive looking east to the east of the Southern Freeway south bound on and off ramps.

#### 2.1.4 Princes Highway

Princes Highway performs the function of a sub-arterial road in Berkeley and has the following key features:

- The posted speed limit is 70 km/h in the vicinity of Nolan Street; and
- The carriageway is sealed, with two marked lanes in each direction.



Photo 4 - The Princes Highway looking south to the south of Nolan Street

#### 2.2 Existing Traffic Volumes

This section provides an understanding of current traffic volumes.

#### 2.2.1 Existing Daily and Peak Hour Traffic

Intersection turning movement surveys were undertaken during the peak AM and PM at the following intersections on Tuesday the 9<sup>th</sup> October 2012 between 7–9 am and 4–6 pm:

- Warwick Street / Nolan Street (stop sign);
- Nolan Street / Northcliffe Drive (roundabout);
- Northcliffe Drive / Southern Freeway, Southbound on and off ramps (roundabout);
- Northcliffe Drive / Southern Freeway, Northbound on and off ramps (roundabout);
- Northcliffe Drive / Princes Highway (signalised); and
- Nolan Street / Princess Highway (signalised).

A copy of the traffic count data is provided in the Appendix A.

A network peak hour of 8 am to 9 am and 4 pm to 5 pm was selected based on analysis of peak period volumes across the study area. Peak hour and estimated daily traffic volumes are summarised in Table 2.

Table 2 Existing Traffic Volumes - October 2012

Location	AM Peak Hour (vph)	PM Peak Hour (vph)	Daily* (vpd)
Warwick Street west of Nolan Street	54	65	595
Nolan Street north of Warwick Street	426	500	4,630
Nolan Street South of Warwick Street	436	481	917
Nolan Street east of Princes Highway	377	438	4,075
Northcliffe Drive north of Nolan Street	888	1,201	10,445
Princes Highway west of Nolan Street	1,248	1,140	11,940

Note: \*daily traffic volumes have been estimates based on the assumption that the average peak hour traffic flow is approximately 10% of the daily traffic volume.

Based on the existing traffic volumes shown in Table 2 these roads all are within the expected functional classification as outlined in Table 1.

#### 2.2.2 Heavy and Light Vehicle Ratio

From the traffic volume surveys undertaken in September 2012 the heavy vehicle percentage is shown in Table 3.

Table 3 Heavy Vehicles - October 2012

Location	Vehicles per hour (vph)	HCV* (vph)	HCV* %
Nolan Street (south of Warwick St)	481	27	6%
Warwick Street	65	0	0%
Northcliffe Drive (west of Nolan St)	1,201	47	4%
Princes Highway (south of Nolan St)	1248	77	6%

Note: \* HCV - heavy commercial vehicles

#### 2.3 Existing Intersection Performance

The performance of the existing road network is largely dependent on the operating performance of key intersections, which are critical capacity control points on the road network. SIDRA intersection modelling software was used to assess the proposed peak hour operating performance of intersections on the surrounding road network. The criteria for evaluating the operational performance of intersections is provided by the RTA *Guide to Traffic Generating Developments* and reproduced in Table 4. The criteria for evaluating the operational performance of intersections is based on a qualitative measure (i.e. Level of Service), which is applied to each band of average vehicle delay.

Table 4 Performance Criteria at Intersections

Level of Service	Average Delay Per Vehicle (secs/vehicle)	Traffic Signals, Roundabout	Give-Way and Stop Signs
Α	Less than 14	Good Operation	Good Operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory but accident study required
D	43 to 56	Operating near capacity	Near capacity and other accident study required
E	57 to 70	At capacity; at signals incidents will cause excessive delays	At capacity and requires other control mode
F	Greater than 70	Roundabouts require other control mode	

#### Notes:

- a) The average delay for priority-controlled intersections is selected from the movement on the approach with the highest average delay.
- b) The level of service for priority-controlled intersections is based on the highest average delay per vehicle for the most critical movement.
- c) The degree of saturation is defined as the ratio of the arrival flow (demand) to the capacity of each approach.

Existing traffic flows were analysed using SIDRA to obtain the operation of key intersections in the study area.

Table 5 Intersection Performance AM and PM Peak Hour – Traffic Conditions 2012 (without development)

	Morning Peak			Evening Peak		
Intersection	Av Delay (s)	LoS	DoS	Av Delay (s)	LoS	DoS
Nolan St/Northcliffe Dr	13	Α	0.20	13	Α	0.24
Nolan St/Warwick St	14	Α	0.07	14	Α	0.05
Nolan St/Princes Hwy	18	В	0.67	18	В	0.60
Northcliffe Dr/Southern Fwy Southbound on/off ramps	16	В	0.25	15	В	0.29
Northcliffe Dr/Southern Fwy Northbound on/off ramps	15	В	0.15	15	В	0.12
Princes Hwy/Northcliffe Drive	22	В	0.71	19	В	0.78

Table 5 indicates that the existing intersections surrounding Life City currently operate satisfactorily in both the AM and PM peaks in 2012. Detailed performance of these intersections is in Appendix B.

#### 2.4 Crash Statistics

The Roads and Maritime Services (RMS) Crash Analysis Department supplied GHD with crash statistics for a five-year period from 2007 to 2011 for the length of Nolan Street covering the proposed access location to the development, and are summarised below:

In total, 22 crashes occurred over this 5 year period. Of these recorded crashes:

- 13 crashes resulted in injury;
- 16 of these crashes occurred in dry/daylight conditions;
- 16 were multi vehicle crashes;
- 14 crashes occurred at an intersection; and
- Speed was a contributing factor in 5 of the crashes.

Figure 2 shows the location of the crashes on Nolan Street. These crashes were spread along the route and not concentrated in one particular area. The crash statistics along Nolan Street do not indicate any pattern of crashes that would signify a specific safety issue along the route. Detailed crash reports are in Appendix D.

Figure 2 - Nolan Street Crash Map



#### 2.5 Pedestrian and Bicycle Environment

Nolan Street has a pedestrian footpath approximately 1.5 m wide on both sides of the road between Northcliffe Drive and pedestrian crossing to the school. The footpath then continues on the eastern side only from the school to Hopman Crescent, where it changes to the western side to the Princes Highway.

There are currently no on or off road bicycle facilities along Nolan Street.

#### 2.6 Public Transport

The Warrawong to Wollongong Bus Route 34 travels via Nolan Street. Figure 3 shows the route for this service and Table 6 shows the bus stop locations along Nolan Street.

Figure 3 - Bus Network Map



Source: http://www.premierillawarra.com.au/networkmap.html

Route 34 from Warrawong to Wollongong travels through Nolan Street every 20 – 30 mins from 6 am to 8:30 pm Monday to Friday. Services also operate on weekends and public holidays. Route 34 also provides a connection to Unanderra Railway Station.

As the bus route 34 from Warrawong to Wollongong already travels through Nolan Street, there could be an opportunity extend the bus route to include a loop through the Life City Development to encourage public transport usage to the site.

Table 6 Bus Stop Locations on Nolan Street (including Warwick St Loop)

Northbound (Along Nolan Street)	Southbound (Along Nolan Street)
Between Northcliffe to Cumberland Street	Near Investigator Drive
Between Cumberland Street to Gallop Street	On Warwick Street near Hunt Place
Nottingham Street to Norfolk Street	On Hopman Cres – near Newcombe Street
On Hopman Cres – near Warwick Street	Between Gallop Street and Suffolk Street
On Warwick St – near Newcombe Street	Between Kelly Street and Gallop Street
On Warwick between Hunt and Newcombe St	
Warwick Street to Nolan Bridge	
Near Investigator Drive	
Near the Rail Crossing	

# 3. Proposed Life City Development Impact

This section outlines the proposed development on the Life City development and summarises the traffic impact analysis of the development as well as any additional works that may be required to accommodate additional traffic generated by the development.

#### 3.1 Overview - Proposed 'Life City' Development

The proposed development is a 'Hi Tech Holistic Cancer and Medical Hospital Facility' which consists of the following land uses:

- Stage 1 Medical Centre, Day Surgery, Child Care Centre and Respite Care Centre;
- Stage 2 Medi Hostel and Medi Serviced Apartments;
- Stage 3 Hi-Tech Holistic Cancer and Medical Hospital;
- Stage 4 Self-care Seniors Housing;
- Stage 5 Residential Care Facility and Hostel; and
- Stage 6 Holistic Healthcare Course.

A plan of the proposed development and the layout of these land uses are shown below in Figure 4. Access to the development would initially be at the western end of Warwick Street for Stages 1 with a new intersection at Nolan Street located between Hopman Crescent and Norfolk Street being provided prior to the opening of Stage 2.

North

Gate – No through
Access (emergency
Access Only)

Proposed Nolan Street
Access

Figure 4 - Proposed Development

Source: Boss Design Overall Concept Plan September 2013

#### 3.2 Projected Traffic Generation

An indication of the potential traffic generation from the 'Life City' development has been obtained from the RTA *Guide to Traffic Generating Developments (2002)*. Some of the land uses are not covered in the RTA Guide, for these, the traffic generation and assumptions used are those provided in the *Traffic Impact Assessment* undertaken by Bruce Conneeley for the Preliminary Environmental Assessment.

Table 7 below outlines the traffic generation and assumptions used for each land use.

Table 7 Potential Traffic Generation

Stage	Component	Beds/Rooms/Persons	Peak Hour Traffic Generation Rate	Peak Hour Trips
1	Medical Centre, Day surgery and Respite Care Centre	26 consulting rooms (18 Specialists)	4 trips per specialist	72
		10 Beds	10 x 1 = 10	10
	Childcare Centre	70 Children	0.7 trips per child	49
2	Medi Hostel and Medi Serviced Apartments	12 x 2 bedroom 6 x 3 bedroom 24 hostel rooms	0.4-0.5/dwelling (RMS Medium density)	26
3	Hi-Tech Holistic Cancer and Medical Hospital	320 beds	*Assumption from Prelim assessment: 0.97 per bed reduced by 35 as staff accommodation on site	275
4	Self-care seniors housing	60 units 10 x 1 bedroom 40 x 2 bedroom 10 x 3 bedroom	0.4-0.5/dwelling (RMS Medium density)	25
5	Residential Care Facility and Hostel	130 beds	0.1/dwelling (RMS)	13
6	Holistic Health Care Course	On site use only	No traffic generation as for on-site use only	-
			Total	470

#### Notes:

Stage 6 the proposed Holistic Health Care Course will not generate additional traffic to/from the site as the course will only be available for use by staff and patients of the facility. It should also be noted that Stage 2 the Medi Hostel and Medi Serviced Apartments would only be available for use by staff, patients and visitors.

<sup>\*</sup>Assumptions are from the report titled *Traffic Impact Assessment* by Bruce Conneeley (August 2010) for the Preliminary Environmental Assessment.

<sup>\*\*</sup>vtph = vehicle trips per hour

Based on the above trip rates, the estimated peak hour traffic generation for the proposed development at each stage is provided in Table 8.

Table 8 Peak Hour Traffic Generation Potential

Stage	Morning Peak (vtph) IN	Morning Peak (vtph) OUT	Evening Peak (vtph) IN	Evening Peak (vtph) IN	Expected Year of Completion
1	66	66	66	66	2014-2015
2	13	13	13	13	2014-2015
3	137	137	137	137	2015-2016
4	13	13	13	13	2016-2017
5	6	6	6	6	2017-2018
6	-	-	-	-	2018-2019
Total	235	235	235	235	

The above table indicates that the peak movement to or from the proposed development would be in the order 470 vtph (one way) in the AM and PM peak period, which equates to 235 vph (two way).

#### 3.3 Development Access

Warwick Street would initially be used to access Stages 1. The proposed Nolan Street access would be implemented prior to the operation of Stage 2. From then onwards the Warwick Street access would only service Stage 1 (Medical Centre, Day surgery and Respite Care Centre). There would be no through access from Warwick Street to the rest of the development (except for emergency access).

Vehicular access to the site would be provided by direct access to Nolan Street. The proposed access on Nolan Street would be the main visual entrance to the development, and the most direct access from the freeway, via Northcliffe Drive. The majority of vehicles including service vehicles would access the site through the Nolan Street access.

#### 3.3.1 Sight Distance

In assessing this proposed development it is appropriate to assess the traffic safety of the proposed entry/exit locations to/from the site, by determining whether there is adequate longitudinal sight distance at the proposed access to allow drivers approaching the driveway sufficient sight distance to avoid potential conflicts. The specific sight distance criteria used in the report has been derived from the Austroads Publication "Guide to Traffic Engineering Practice, Part 5: Intersections at Grade" (2005) which specifies the following:

#### Approach Sight Distance (ASD)

This is the minimum requirement to provide the driver of a vehicle adequate distance to observe the road layout, including pavement markings, kerbs, islands, etc, in sufficient time to react and stop if necessary before entering the conflict area. Approach Sight Distance is measured from driver eye height (1.15m to 0.0m, i.e. the road surface).

#### Safe Intersection Sight Distance (SISD)

It provides sufficient sight distance for a driver of a vehicle on the major road to observe a vehicle from the minor road approach moving into a collision situation (e.g. in the worst case stalling across the traffic lanes), and to decelerate to stop before reaching the collision point. Safe Intersection Sight Distance is measured from the driver eye height (1.05m) to (1.05m).

A comparison of the required and available ASD and SISD for vehicles approaching and departing the proposed access road is set out in Table 4 for the vehicle speed of 50 km/h.

Table 9 Sight Distance Requirements

Driveway (Entry/Exit)	ASD (Minimu	m Requirement	SISD (Desirable	Requirement)
	Required Measured		Required	Measured
Nolan Street	50 m	160 m (north)	90 m	160 m (north)
50 km/h		90 m (south)		90 m (south)

From the above assessment, it can be concluded that the proposed access arrangement meets a satisfactory level of traffic safety because it satisfies the sight distance requirements for both (ASD) and (SISD) measured in accordance with the definition set out in the Austroads publication.



Photo 5 - Nolan Street looking northeast with the proposed development site on the left.

#### 3.4 Parking Requirements

#### 3.4.1 Parking Provision

Wollongong Development Control Plan 2009 Chapter E3: Carparking, Access, Servicing/loading Facilities and Traffic Management provides car parking requirements, based on a minimum parking provision, for various land uses. Where requirements could not be obtained in the DCP, the RTA Guide to Traffic Generating Developments 2002 has been used as an alternative reference.

Table 10 Parking Requirements - Life City Wollongong

Stage	Component	Beds/Rooms/ Persons/GFA	Parking Requirements	No. Spaces (WCC DCP)	No. Spaces Provided
1	Medical Centre and day surgery Childcare Centre	3,664 m <sup>2</sup> GFA 70 Children	4 spaces per consulting room and 1 space per 3 employees	135 20	162
			<ul><li>1 per 6 children</li><li>1 per staff present</li><li>1 accessible</li><li>2 large spaces</li></ul>		
2	Medi Hostel and Medi Service Apartments	12 x 2 bedroom 6 x 3 bedroom 24 hostel rooms	housing attached dwelling (1 space per (<70 m2) hostel rooms) (1.5 per (70 - 110m2) 2 bedroom)(2 per >110m2) plus 0.2 per dwelling for visitors	56	56
3	Hi Tech Holistic Cancer and Medical Hospital	320 beds	1 space per 2 beds plus 1 space per practitioner and 1 per 2 employees	260	260 + 6 ambulance
4	Self-care seniors housing	10 x 1 bedroom 40 x 2 bedroom 10 x 3 bedroom	Seniors Housing 0.5 per bedroom	60	60
5	Residential Care Facility and Hostel	139 beds	1 space per 10 beds plus 1 per 2 employees	27	23
6	Holistic Health Care Course		on site use only no additional parking requirement	-	19
Total				558	580
Surplus P	arking Spaces				+22

Notes: \*Where no guidance from the DCP is provided an assumption has been made.

A total of 580 parking spaces in total would be provided within this development, which exceeds the required 558 parking spaces required under Wollongong City Councils DCP. At each stage of the development sufficient parking spaces would be provided on site as required under the DCP.

State Environmental Planning Policies – Housing for Seniors & People with a Disability (SEPP 5) does not include parking space measurement standards. Therefore, in accordance with the Australian Standards AS 2890.1:2004 Parking Facilities, Part 1: Off-street Car Parking, the parking space measurements will be as follows:

- 2.4 m wide;
- 4.8 m long (car parking is controlled by a kerb which allows overhang); and
- Aisle width of 5.8 m.

Accessible parking spaces are to be provided in accordance with AS 2890.6 (2009) as outlined in Schedule 2 of the DCP's chapter E3, 1 space is required for every 100 car parking spaces. Therefore a total of 6 accessible parking spaces will be provided in this development.

Accessible parking measurements will be in accordance with AS 2890.6 (2009) Section 2.2.1, which states a dedicated parking space of 2.4 m wide, 5.4 m long and must include a shared area on one side of the space that is also 2.4 m wide and 5.4 m long.

#### 3.4.2 Service and Delivery Vehicles

A private contractor would be engaged to undertake refuse collection. A bin bay and service delivery area is located within each stage. Service areas and refuse collection would be accessed through the internal road network. Turning movements have been assessed in Auto Turn for the internal road network which has confirmed that an 8.8 m refuse vehicle and rigid truck movements can be accommodated on site and that the trucks would be able to exit the site in a forward direction.

Auto Turn was also used to assess turning movement requirements for an ambulance accessing the front of the hospital this can be accommodated (swept turning paths are attached in Appendix E).

#### 3.4.3 Bicycle / Motorcycle Parking

Bicycle parking is to be included within this development and designed in accordance with AS 2890.3 and provided as per Schedule 1 of the DCP's Chapter E3. Spaces provided for each stage of the development are shown in Table 11.

Table 11 Bicycle / Motorcycle Parking

Stage	Component	No. of Bicycle Parking Spaces	No. of Motorcycle Parking Spaces
1	Medical centre and day surgery and childcare centre	9	-
2	Medi Hostel and Medi Serviced Apartments	4	-
3	Hi Tech Holistic Cancer and medical Hospital	52	10
4	Self-care seniors living	-	-
5	Residential Care Facility and hostel	-	-
6	Holistic Health Care Course	-	-
	Total	65	10

#### 3.5 Intersection Performance

#### 3.5.1 Background Traffic Growth

Background traffic growth has been obtained from the Wollongong Shoalhaven (WOLSH) strategic traffic model. Background traffic growth has been obtained for 2021 (the year of full development) and also the 10 year horizon year, 2031. Background traffic growth has been added to observed 2012 volumes and traffic generated by the development added to the forecast future year volumes.

A summary of peak hour traffic growth from the WOLSH model on each the key roads in the study area by 2031 is shown in Table 12.

Table 12 WOLSH Model Forecast Traffic Flows

Street	AM Peak Hour (vph)	PM Peak Hour (vph)
Warwick Street west of Nolan Street	0	0
Nolan Street north of Warwick Street	+71	+109
Nolan Street South of Warwick Street	+71	+109
Nolan Street east of Princes Highway	+66	+117
Northcliffe Drive north of Nolan Street	+477	+547
Princes Highway west of Nolan Street	+397	+483

#### 3.5.2 Proposed Traffic Distribution

Vehicles can access the development site via Nolan Street and Warwick Street, either from the south, Southern Freeway via Northcliffe Drive or the north from Princes Highway.

The Nolan Street access would be developed as the main access to the site, all visitors, deliveries, service vehicles would be expected to use this access. The Warwick Street access would provide access to Stage 7 – the residential care facility and to the northern section of Stage 1 – the medical centre. There would be no through access (except for emergency access) from Warwick Street to the rest of the development once the Nolan Street access is operational, and this would be controlled by a gate. Based on this, it has been assumed that:

- 72% of the development traffic would enter and exit via the Nolan Street access; and
- 28% of the development traffic would enter and exit via the Warwick Street access.

The existing AM and PM peak traffic distributions on Nolan Street have been reviewed to determine the expected distribution for the development traffic, taking into consideration the surrounding road network and accessibility to the M1 Motorway it has been assumed that:

- 60% of development traffic would be to/from Northcliffe Drive (Southern Freeway); and
- 40% of development traffic would be to/from the Princes Highway.

Figure 5 shows how the proposed development traffic has been assigned on the surrounding road network.

Detailed traffic assignment for the 2021 and 2031 forecast years are provided in Appendix F.

40%
40%
40%
11%
29%
17%
5%
Proposed Life City Development
43%
43%
17%
43%
17%
43%
17%
43%
17%

Figure 5 - Proposed Traffic Distribution

#### 3.5.3 2021 Intersection Operating Performance

The future traffic volumes have been calculated by adding the traffic growth from the WOLSH model to the 2012 surveyed traffic flows presented in Table 2 and then adding the development traffic flows. Estimated future 2021 traffic flows were analysed using SIDRA to obtain the operation of key intersections in the study area. A summary of the 2021 intersection performance is shown in Table 13.

Table 13 Intersection Performance AM and PM Peak Hour –Traffic Conditions 2021 (with development)

	Morning Peak			Evening Peak			
Intersection	Av Delay (s)	LoS	DoS	Av Delay (s)	LoS	DoS	
Nolan St/Life City Access	26	В	0.5	26	В	0.5	
Nolan St/Northcliffe Dr	15	В	0.5	15	В	0.6	
Nolan St/Warwick St	20	В	0.3	21	В	0.3	
Nolan St/Princes Hwy	23	В	0.9	23	В	0.8	
Northcliffe Dr/Southern Fwy Southbound on/off ramps	16	В	0.5	17	В	0.7	
Northcliffe Dr/Southern Fwy Northbound on/off ramps	15	В	0.5	15	В	0.5	
Princes Hwy/Northcliffe Drive	26	В	0.9	22	В	0.8	

#### 3.5.4 2031 Intersection Operating Performance

Estimated future 2031 traffic flows were analysed using SIDRA to obtain the operation of key intersections in the study area. A summary of the 2031 intersection performance is shown in Table 14.

Table 14 Intersection Performance AM and PM Peak Hour -Traffic Conditions 2031 (with development)

	Morning Peak			Evening Peak			
Intersection	Av Delay (s)	LoS	DoS	Av Delay (s)	LoS	DoS	
Nolan St/Life City Access	28	В	0.6	29	С	0.6	
Nolan St/Northcliffe Dr	16	В	0.5	17	В	0.7	
Nolan St/Warwick St	21	В	0.3	23	В	0.3	
Nolan St/Princes Hwy	26	В	0.9	26	В	0.8	
Northcliffe Dr/Southern Fwy Southbound on/off ramps	16	В	0.6	27	В	0.9	
Northcliffe Dr/Southern Fwy Northbound on/off ramps	23	В	0.7	20	В	0.7	
Princes Hwy/Northcliffe Drive	37	С	0.9	31	С	0.9	

In the previous traffic assessment (GHD May 2013) the Nolan Street/Life City Access results showed that the proposed new intersection would operate near to capacity by 2031 and therefore a proposed roundabout arrangement was assessed. Due to a proposed reduction in development at this site and hence a reduction in traffic generation analysis now shows that the intersection would continue to operate satisfactorily in their existing configurations up to 2031 in both the AM and PM peaks

Based on the intersection analysis undertaken in this study, it can be concluded that traffic generated by the 'Life City' development under the 2021 and 2031 horizon years can be accommodated on the existing network.

#### 3.5.5 2031 Midblock Operating Performance

A summary of the midblock flows at key points on key roads in the study area is shown in Table 15.

Table 15 2031 Forecast Midblock Traffic Volumes

Location	AM Peak Hour (vph)	PM Peak Hour (vph)	Daily* (vpd)
Warwick Street west of Nolan Street	186	198	1,920
Nolan Street north of Warwick Street	685	798	7,415
Nolan Street South of Warwick Street	724	808	7,660
Nolan Street east of Princes Highway	725	840	7,825
Northcliffe Drive north of Nolan Street	1,591	1,976	17,835
Princes Highway west of Nolan Street	1,645	1,623	16,340

Note: \*daily traffic volumes have been estimates based on the assumption that the average peak hour traffic flow is approximately 10% of the daily traffic volume.

Analysis of the 2031 forecast midblock traffic flows that the forecast traffic flows on key streets in the study area remain within acceptable environmental flow limits for their function within the road hierarchy.

#### 3.6 Sensitivity Testing

In GHD's previous traffic assessment dated May 2013 the intersection assessment undertaken in section 3.5 was based on mode share assumptions previously identified in proposed land uses of the development which generally reflected an increased proportion of trips using public transport. Therefore in the previous assessment a sensitivity test was undertaken to determine whether the intersection requirements would change if these mode share assumptions were not realised and mode share to public transport stayed the same as is currently observed in the area. However, due to changes in the land uses in this proposed development there are no longer any land uses within the development that are assuming a proportion of public transport. Section 3.5 now identifies the worst case scenario for traffic generation assuming no public transport usage.

#### 3.6.1 Future Public and Active Transport Opportunities

Life City aims to enhance opportunities for walking, cycling and public transport. To achieve this, the development will include:

- Pedestrian paths throughout the Life City development connecting into the existing pedestrian network on Nolan Street;
- Bicycle parking areas to encourage cycling; and
- Bus stops throughout Life City development so that existing bus services can be extended to include Life City.

In order to support public transport usage there is an opportunity for Life City to implement workplace travel plans. It is recommended that the following elements of workplace travel plans be integrated into the development of Life City:

- Providing a shuttle bus service to and from Unanderra and/or Kembla Grange railway station;
- Setting up a car sharing scheme;
- Developing public transport information programs and promotions;
- Providing and encouraging the use of the local pedestrian environment;
- Providing and encouraging the use of cycle facilities;
- Negotiating service requirements with transport agencies; and
- Developing attractive flexible working practises.

## 4. Conclusions

The following conclusions are made based on the above investigations:

#### 4.1 Access Arrangements

- Vehicular access to the site would be provided by direct access to Nolan Street as well as a
  secondary access from Warwick Street. The proposed access on Nolan Street would be the
  main visual entrance to the development, and the most direct access from the freeway, via
  Northcliffe Drive. The majority of vehicles including servicing vehicles would access the site
  through the Nolan Street access. The Warwick Street access would only service the Medical
  Centre (Stage 1). There would be no through access from Warwick Street to the rest of the
  development, except for emergency access.
- The proposed Life City Access with Nolan Street provides a satisfactory level of traffic safety under the current speed environment, as it satisfies the minimum sight distance requirement (ASD) and the desirable sight distance (SISD) measured in accordance with the definition set out in the Austroads publication.
- An assessment of crash data in the vicinity of the site showed that there were 22 crashes
  recorded over a 5 year period on Nolan Street. These crashes appear to be spread over the
  route and not concentrated in one particular area and are not indicative of any traffic safety
  issues on the study area.
- Auto Turn assessments show that an ambulance can access the hospital and that an 8.8 m refuse truck/rigid vehicle can access the site along the proposed internal access roads.

#### 4.2 Parking Arrangements

- A total of 580 parking spaces in total would be provided within this development, which
  exceeds the parking spaces required under Wollongong City Councils DCP. Ambulances
  spaces are provided directly in front of the hospital.
- The proposed car parking spaces and vehicular access arrangements will be designed in accordance with AS 2890.1:2004 Parking Facilities, Part 1: Off-street Car Parking.

#### 4.3 Traffic Impact

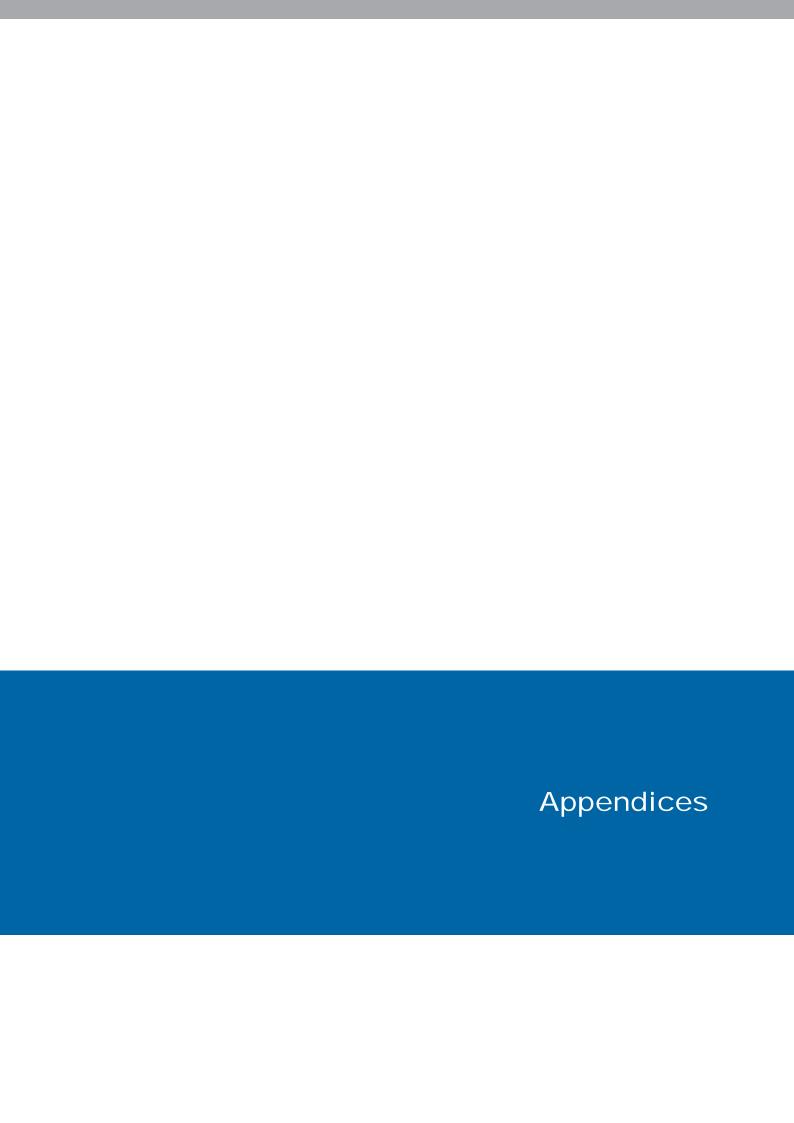
- The peak movement to or from the proposed development would be in the order 470 vtph (one way) in the AM and PM peak period, which equates to 235 vph (two way).
- In the previous traffic assessment (GHD May 2013) the Nolan Street/Life City Access results showed that the proposed new intersection would operate near to capacity by 2031 and therefore a proposed roundabout arrangement was assessed. Due to a proposed reduction in development at this site and hence a reduction in traffic generation analysis now indicates that the intersection will continue to operate satisfactorily in 2031 in both the AM and PM peaks.
- Warwick Street would only provide access to the Medical Centre (Stage 1). No access through to the rest of the development (except emergency access) would ensure that the amenity of Warwick Street would not adversely be impacted.
- The performance outputs from SIDRA intersection analysis indicate that the road network surrounding the Life City development would operate satisfactorily under forecast 2021 and 2031 traffic flows, including background traffic growth and traffic generated by the development during both AM and PM peak periods.

- Analysis of the 2031 forecast midblock traffic flows that the forecast traffic flows on key streets in the study area remain within acceptable environmental flow limits for their function within the road hierarchy.
- The intersection analysis has been undertaken assuming no public transport usage to or from the development, which provides a worst case scenario. Life City aims to enhance opportunities for walking, cycling and public transport through connections from the development to the existing road network.

#### 4.4 Public Transport

- As the bus route 34 from Warrawong to Wollongong already travels through Nolan Street, there could be the opportunity extend the bus route to include a loop through the Life City Development to encourage public transport usage to the site.
- Life City could provide a shuttle bus service to and from Unanderra and/or Kembla grange railway stations in order to encourage additional public transport usage.
- There is an opportunity for Life City to implement workplace travel plans in order to support higher public transport usage.

Therefore, it is concluded that the existing road network in the study area has sufficient capacity to accommodate the proposed Life City Wollongong Development and would continue to operate satisfactorily under the forecast 2021 and 2031 AM and PM peak.



# Appendix A - Traffic Volumes



TCS Instruments - People and Vehicle Counting

#### **SURVEY DETAILS**

DAY /DATE	FINE				
WEATHER	11/10/2012				
survey period		7	То	9	
		16	То	18	
			То		
Intersection	Interposition		PRINCESS HWY		
intersection	١	OLAN STR	EET		
Client reference					
Description:	Traffic Counts (15mins & 1hour Data)				

#### VARIABLE HOURLY VEHICLE SUMMARY

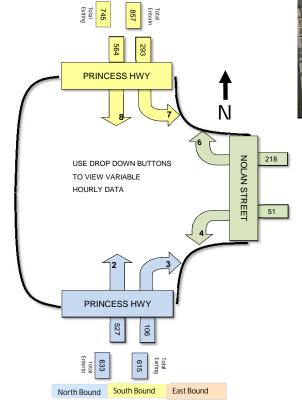
Hour Starting	8:00	•	All vehicl	es			
	VEHICLE PEAKS						
AM Peak	7:45	to	8:45	2144			
PM Peak	16:00	to	17:00	2052			

DAILY TOTAL	AM	PM
Cars	2665	3001
Trucks	158	55
Buses	75	43
Articulated	0	0
Moterbikes	0	0

TOTALS	2898		3099
Total Vehicles through intersection all			5997
day			3331

	AM	PM
PEDESTRIANS		
Total Pedestrians through inter	rsection all	
day		0

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North Bound

399

Total Entering



TCS Instruments - People and Vehicle Counting

#### **SURVEY DETAILS**

DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	F	PRINCESS HWY		
intersection	١	NOLAN STREET		
Client reference				
Description:	Traffic Counts (15mins & 1hour Data)			

#### VARIABLE HOURLY VEHICLE SUMMARY

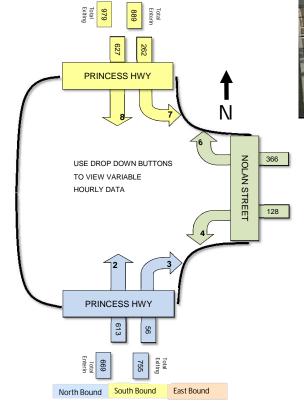
16:00	•	All vehic	les			
VEHICLE PEAKS						
7:45	to	8:45	2144			
16:00	to	17:00	2052			
	V 7:45	VEHICL 7:45 to	VEHICLE PEAKS 7:45 to 8:45	VEHICLE PEAKS 7:45 to 8:45 2144		

DAILY TOTAL	AM	PM
Cars	2665	3001
Trucks	158	55
Buses	75	43
Articulated	0	0
Moterbikes	0	0

TOTALS	2898	3099
Total Vehicles through inters day	5997	

	AM	PM
PEDESTRIANS		
Total Pedestrians through inte		
dav		0

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North Bound

318
Total Entering



TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

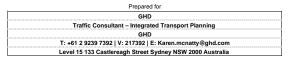
DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	NOLAN ST			
Intersection	ENTER STREET			
Client reference				
Description:	Traffic Counts (15mins & 1hour Data)			

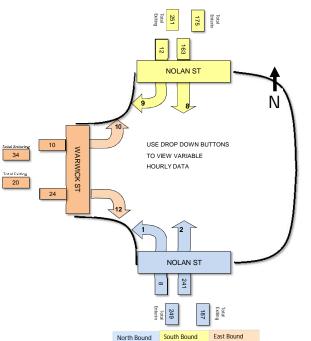
Hour Starting	8:00	•	All vehicle	es 🔻		
	VEHICLE PEAKS					
AM Peak	7:45	to	8:45	573		
PM Peak	16:00	to	17:00	657		

DAILY TOTAL	AM	PM
Cars	774	944
Trucks	26	19
Buses	35	18
Articulated	0	0
Moterbikes	0	0

TOTALS	835	981
Total Vehicles through inters day	1816	

PEDESTRIANS			
Total Pedestrians through intersection all			
day			0









TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

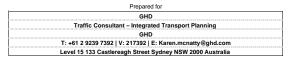
DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	NOLAN ST			
intersection	ENTER STREET			
Client reference				
Description:	Traffic Counts (15mins & 1hour Data)			

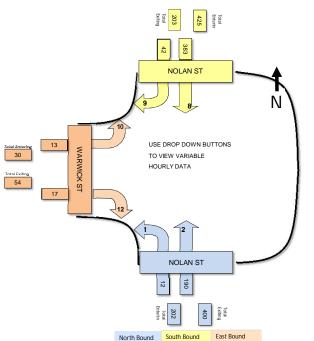
Hour Starting	16:00	•	All vehic	es 🔻		
	VEHICLE PEAKS					
AM Peak	7:45	to	8:45	573		
PM Peak	16:00	to	17:00	657		

DAILY TOTAL	AM	PM
Cars	774	944
Trucks	26	19
Buses	35	18
Articulated	0	0
Moterbikes	0	0

TOTALS	835	981
Total Vehicles through inters day	1816	

	AW		PIVI
PEDESTRIANS			
Total Pedestrians through intersection all			
day			0









TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

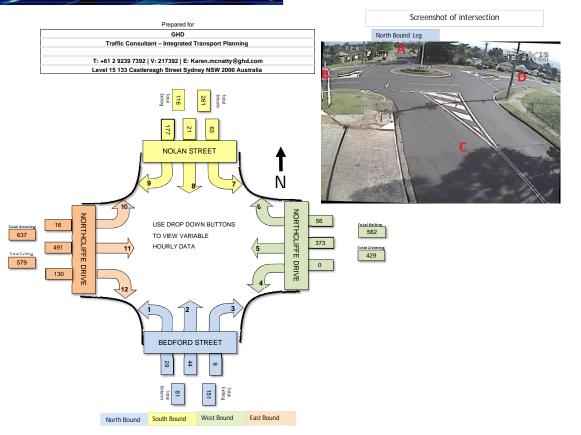
DAY/DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	BEDFORD STREET			
intersection	NORTHCLIFFE DRIVE			
Client reference				
Description:	Pedestrians & Traffic Counts (15mins & 1hour Data)			

Hour Starting	8:00	•	All vehic	les	
VEHICLE PEAKS					
AM Peak	7:45	to	8:45	1729	
PM Peak	16:00	to	17:00	2043	

DAILY TOTAL	AM	PM
Cars	2226	3001
Trucks	110	35
Buses	47	24
Articulated	0	0
Moterbikes	0	0

TOTALS	2383	3060
Total Vehicles through inters day	ection all	5443

	AM	PM
PEDESTRIANS		
Total Pedestrians through intersection all		
day		





TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

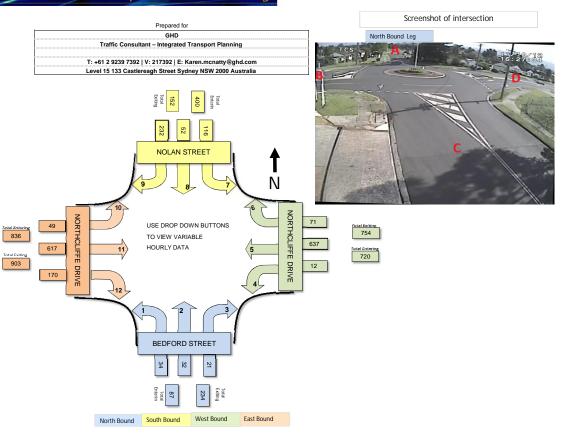
DAY/DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	BEDFORD STREET			
intersection	NORTHCLIFFE DRIVE			
Client reference				
Description:	Pedestrians & Traffic Counts (15mins & 1hour Data)			

Hour Starting	16:00	•	All vehicle	es 🔻	
VEHICLE PEAKS					
AM Peak	7:45	to	8:45	1729	
PM Peak	16:00	to	17:00	2043	

DAILY TOTAL	AM	PM
Cars	2226	3001
Trucks	110	35
Buses	47	24
Articulated	0	0
Moterbikes	0	0

TOTALS	2383	3060
Total Vehicles through intersection all day		5443

	AM	PM	
PEDESTRIANS			
Total Pedestrians through inter			
day		0	





TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

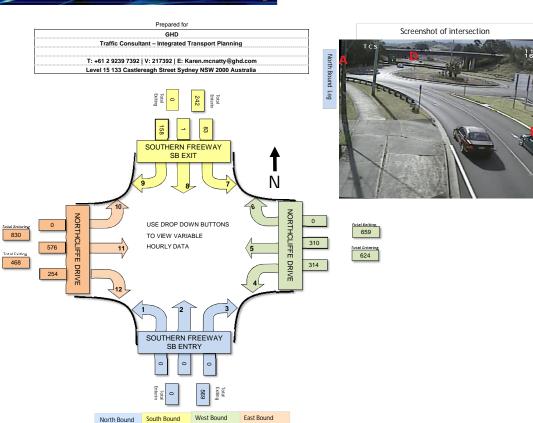
DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	SOUTHERN FREEWAY SB ENTRY			
intersection	NORTHCLIFFE DRIVE			
Client reference				
Description:	Traffic Counts (15mins & 1hour Data)			& 1hour

Hour Starting	8:00	•	All vehicl	es 🔻			
	VEHICLE PEAKS						
AM Peak	7:45	to	8:45	2108			
PM Peak	16:00	to	17:00	2530			

DAILY TOTAL	AM	PM
Cars	2686	3748
Trucks	191	67
Buses	47	11
Articulated	0	0
Moterbikes	0	0

TOTALS	2924	3826
Total Vehicles through inters day	ection all	6750

PEDESTRIANS			
Total Pedestrians through intersection all		_	
day			0





TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

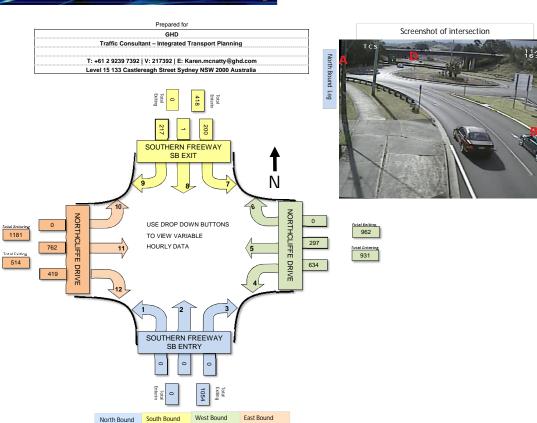
DAY/DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	SOUTHERN FREEWAY SB ENTRY			
intersection	NORTHCLIFFE DRIVE			
Client reference				
Description:	Traffic Counts (15mins & 1hour Data)			

Hour Starting	16:00	•	All vehicl	es 🔻	
VEHICLE PEAKS					
AM Peak	7:45	to	8:45	2108	
PM Peak	16:00	to	17:00	2530	

DAILY TOTAL	AM	PM
Cars	2686	3748
Trucks	191	67
Buses	47	11
Articulated	0	0
Moterbikes	0	0

TOTALS	2924	3826
Total Vehicles through intersed	ection all	6750

	AM		PM
PEDESTRIANS			
Total Pedestrians through intersection all			
day			0





TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	SOUTHERN FREEWAY NB EXIT			
		RTHCLIFFE DRIVE		
Client reference				
Description:	Pedestrians & Traffic Counts (15mins & 1hour Data)			

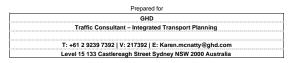
#### VARIABLE HOURLY VEHICLE SUMMARY

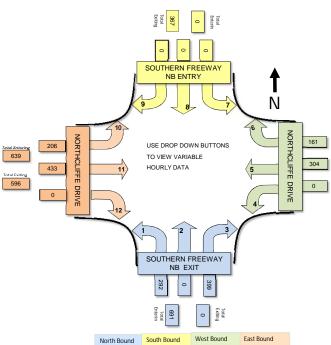
Hour Starting	8:00	•	All vehic	es		
VEHICLE PEAKS						
AM Peak	7:45	to	8:45	2225		
PM Peak	16:00	to	17:00	2173		

DAILY TOTAL	AM	PM
Cars	2900	3137
Trucks	245	84
Buses	32	14
Articulated	0	0
Moterbikes	0	0

TOTALS	3177	3235
Total Vehicles through intersed	6412	

	AW		PIVI
PEDESTRIANS			
Total Pedestrians through intersection all			
day			0





#### Screenshot of intersection



North Bound Leg







TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

DAY /DATE	FINE			
WEATHER	11/10/2012			
survey period		7	То	9
		16	То	18
			То	
Intersection	SOUTHERN FREEWAY NB EXIT			
		RTHCLIFFE DRIVE		
Client reference				
Description:	Pedestrians & Traffic Counts (15mins & 1hour Data)			

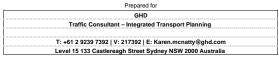
#### VARIABLE HOURLY VEHICLE SUMMARY

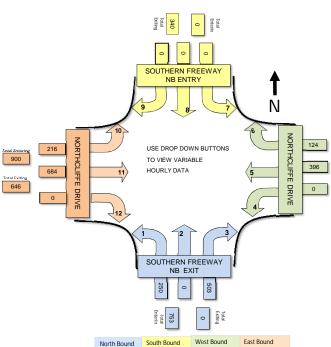
Hour Starting	16:00	•	All vehic	es		
VEHICLE PEAKS						
AM Peak	7:45	to	8:45	2225		
PM Peak	16:00	to	17:00	2173		

DAILY TOTAL	AM	PM
Cars	2900	3137
Trucks	245	84
Buses	32	14
Articulated	0	0
Moterbikes	0	0

TOTALS	3177	3235
Total Vehicles through inters day	6412	

	AW	PIVI
PEDESTRIANS		
Total Pedestrians through inter		
day		0





#### Screenshot of intersection



North Bound Leg

1187

Total Entering



TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

	001112122111120				
DAY /DATE	FINE				
WEATHER		11/10/201	2		
		7	То	9	
survey peri	survey period		То	18	
			То		
Intersection		RINCES HIGHWAY			
		RTHCLIFFE DRIVE			
Client reference					
Description:	Traffic Counts (15mins & 1hour Data)				

#### VARIABLE HOURLY VEHICLE SUMMARY

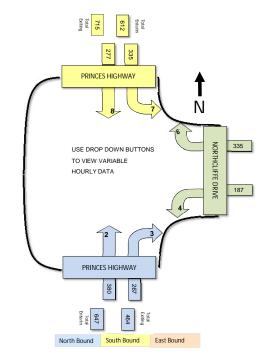
Hour Starting	8:00	•	All vehic	es 🔻		
U	VEHICLE PEAKS					
AM Peak	7:45	to	8:45	2270		
PM Peak	16:00	to	17:00	2405		

DAILY TOTAL	AM	PM
Cars	2949	3437
Trucks	257	101
Buses	58	36
Articulated	0	0
Moterhikes	0	0

TOTALS	3264	3574
Total Vehicles through intersed	ection all	6838

	AM	PM
PEDESTRIANS		
Total Pedestrians through inter	section all	
day		0

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Screenshot of intersection

North Bound



602

Total Entering 522



TCS Instruments - People and Vehicle Counting

#### SURVEY DETAILS

DAY /DATE		FINE			
WEATHER	11/10/2012				
		7	То	9	
survey peri	od	16	То	18	
			То		
Intersection	PRINCES HIGHWAY				
intoroccion	NORTHCLIFFE DRIVE				
Client reference					
Description:	Traffic C	Counts (15m Data)	nins	& 1hour	

#### VARIABLE HOURLY VEHICLE SUMMARY

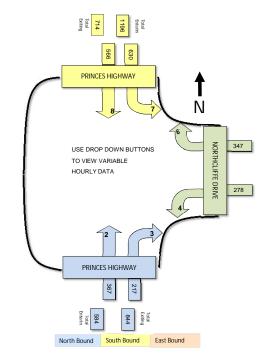
Hour Starting	16:00	•	All vehic	les
	v	EHIC	E PEAKS	
AM Peak	7:45	to	8:45	2270
PM Peak	16:00	to	17:00	2405

DAILY TOTAL	AM	PM
Cars	2949	3437
Trucks	257	101
Buses	58	36
Articulated	0	0
Moterbikes	0	0

TOTALS	3264	3574
Total Vehicles through inters day	ection all	6838

	AW	PIN
PEDESTRIANS		
Total Pedestrians through inte	rsection all	
day		0

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Screenshot of intersection

North Bound



847

Total Entering 625

# Appendix B – SIDRA Outputs

Site: Life City Access/Nolan Street 2021 + Growth + Dev AM

Life City Access/Nolan Drive Stop (Two-Way)

		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/r
South: N	Nolan Str	eet South									
1	L	106	0.0	0.057	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
2	Т	327	6.0	0.174	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	434	4.5	0.174	2.0	NA	0.0	0.0	0.00	0.16	56.9
North: N	Nolan Stre	et North									
8	Т	336	5.0	0.178	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R	72	0.0	0.077	10.4	LOS A	0.3	2.1	0.46	0.73	46.5
Approac	ch	407	4.1	0.178	1.8	NA	0.3	2.1	0.08	0.13	57.1
West: Li	ife City A	ccess									
10	L	72	0.0	0.520	25.8	LOS B	2.9	20.2	0.73	1.13	35.9
12	R	106	0.0	0.520	25.7	LOS B	2.9	20.2	0.73	1.13	36.0
Approac	ch	178	0.0	0.520	25.7	LOS B	2.9	20.2	0.73	1.13	35.9
All Vehi	cles	1019	3.6	0.520	6.1	NA	2.9	20.2	0.16	0.32	51.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

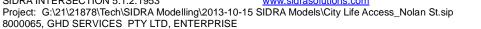
Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: Life City Access/Nolan Street 2021 + Growth + Dev PM

Life City Access/Nolan Drive Stop (Two-Way)

Moven	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Nolan Stre	eet South									
1	L	107	0.0	0.058	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
2	Т	243	12.0	0.134	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	351	8.3	0.134	2.5	NA	0.0	0.0	0.00	0.20	56.1
North: N	Nolan Stre	et North									
8	Т	419	2.0	0.218	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R	73	0.0	0.081	10.0	LOS A	0.3	2.0	0.42	0.71	46.9
Approac	ch	492	1.7	0.218	1.5	NA	0.3	2.0	0.06	0.10	57.7
West: Li	ife City Ad	ccess									
10	L	73	0.0	0.520	25.6	LOS B	2.9	20.5	0.70	1.09	36.0
12	R	107	0.0	0.520	25.5	LOS B	2.9	20.5	0.70	1.12	36.0
Approac	ch	180	0.0	0.520	25.6	LOS B	2.9	20.5	0.70	1.11	36.0
All Vehic	cles	1022	3.7	0.520	6.1	NA	2.9	20.5	0.15	0.32	51.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\City Life Access\_Nolan St.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE



Site: Life City Access/Nolan Street 2031 + Growth + Dev AM

Life City Access/Nolan Drive Stop (Two-Way)

		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/r
South: N	Nolan Str	eet South									
1	L	106	0.0	0.057	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
2	Т	352	6.0	0.187	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	458	4.6	0.187	1.9	NA	0.0	0.0	0.00	0.15	57.0
North: N	Nolan Stre	et North									
8	Т	346	5.0	0.183	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R	72	0.0	0.079	10.6	LOS A	0.3	2.2	0.47	0.74	46.4
Approac	ch	418	4.1	0.183	1.8	NA	0.3	2.2	0.08	0.13	57.1
West: Li	ife City A	ccess									
10	L	72	0.0	0.553	27.7	LOS B	3.1	21.8	0.76	1.18	34.8
12	R	106	0.0	0.553	27.6	LOS B	3.1	21.8	0.76	1.14	34.9
Approac	ch	178	0.0	0.553	27.6	LOS B	3.1	21.8	0.76	1.16	34.9
All Vehi	cles	1054	3.6	0.553	6.2	NA	3.1	21.8	0.16	0.31	51.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\City Life Access\_Nolan St.sip



Site: Life City Access/Nolan Street 2031 + Growth + Dev PM

Life City Access/Nolan Drive Stop (Two-Way)

		formance - \ Demand		Deg.	Average	Level of	95% Back	of Ougue	Prop.	C#cotive	Avorage
Mov ID	Turn	Flow	HV	Satn	Average Delay	Service	Vehicles	Distance	Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	Sec	Service	verlicies veh	Distance	Queueu	per veh	km/h
South: I	Nolan Stre		70	V/ O	300		V 311			per veri	KITI/TI
1	L	107	0.0	0.058	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
2	Т	266	12.0	0.147	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	374	8.6	0.147	2.4	NA	0.0	0.0	0.00	0.19	56.4
North: N	Nolan Stre	et North									
8	Т	448	2.0	0.233	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R	73	0.0	0.082	10.2	LOS A	0.3	2.0	0.43	0.72	46.8
Approa	ch	521	1.7	0.233	1.4	NA	0.3	2.0	0.06	0.10	57.7
West: L	ife City Ad	ccess									
10	L	73	0.0	0.569	28.6	LOS C	3.3	23.1	0.74	1.15	34.3
12	R	107	0.0	0.569	28.5	LOS C	3.3	23.1	0.74	1.15	34.3
Approa	ch	180	0.0	0.569	28.6	LOS C	3.3	23.1	0.74	1.15	34.3
All Vehi	cles	1075	3.8	0.569	6.3	NA	3.3	23.1	0.15	0.31	51.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\City Life Access\_Nolan St.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE

Site: Nolan / Northcliffe 2021 Existing + Growth + Dev AM

Nolan St / Northcliffe Drive Roundabout

Movem	ent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back ( Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	hcliffe Drive	70	V/C	Sec		ven	111		per veri	KIII/II
21	L	1	0.0	0.271	8.7	LOS A	1.9	14.1	0.67	0.71	52.4
22	Т	462	8.0	0.271	9.2	LOS A	1.9	14.1	0.67	0.69	52.7
23	R	99	13.0	0.271	14.8	LOS B	1.7	12.9	0.68	0.86	47.5
Approac	ch	562	8.9	0.271	10.2	LOS A	1.9	14.1	0.68	0.72	51.7
North Ea	ast: Nolai	n Street									
24	L	101	11.0	0.474	7.6	LOS A	2.7	19.4	0.61	0.70	43.7
25	Т	23	0.0	0.474	5.0	LOS A	2.7	19.4	0.61	0.58	42.2
26	R	318	3.0	0.474	13.8	LOS A	2.7	19.4	0.61	0.88	41.5
Approac	ch	442	4.7	0.474	11.9	LOS A	2.7	19.4	0.61	0.82	42.0
North W	est: Nort	hcliffe Drive									
27	L	144	0.0	0.219	6.8	LOS A	1.4	9.9	0.35	0.54	54.1
28	Т	261	9.0	0.219	7.1	LOS A	1.4	9.9	0.35	0.49	55.2
29	R	208	11.0	0.219	12.2	LOS A	1.3	9.9	0.37	0.69	48.5
Approac	ch	614	7.6	0.219	8.8	LOS A	1.4	9.9	0.36	0.57	52.5
South W	/est: Bed	ford Street									
30	L	31	3.0	0.117	8.0	LOS A	0.5	3.9	0.64	0.74	44.2
31	Т	46	2.0	0.117	5.5	LOS A	0.5	3.9	0.64	0.62	42.8
32	R	8	0.0	0.117	14.3	LOS A	0.5	3.9	0.64	0.91	41.7
Approac	ch	85	2.2	0.117	7.3	LOS A	0.5	3.9	0.64	0.69	43.2
All Vehic	cles	1703	7.0	0.474	10.0	LOS A	2.7	19.4	0.54	0.69	48.5

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Tuesday, 15 October 2013 1:49:00 PM SIDRA INTERSECTION 5.1.2.1953

Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Nolan St\_Northcliffe Drive.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE

Site: Nolan / Northcliffe 2021 Existing + Growth + Dev PM

Nolan St / Northcliffe Drive Roundabout

Mover	nent Per	formance - \	/ehicles								
		Demand	1111	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Courth I	- oot: Nort	veh/h hcliffe Drive	%	v/c	sec		veh	m		per veh	km/h
	ası. Noru		0.0	0.351	9.0	LOS A	0.7	19.0	0.74	0.75	F0.0
21	L +	13	0.0				2.7		-	0.75	52.0
22	T	611	1.0	0.351	9.4	LOS A	2.7	19.0	0.75	0.73	52.1
23	R	95	11.0	0.351	15.1	LOS B	2.4	17.4	0.75	0.89	47.4
Approa	ıch	718	2.3	0.351	10.2	LOS A	2.7	19.0	0.75	0.75	51.5
North E	ast: Nola	n Street									
24	L	141	4.0	0.604	9.3	LOS A	4.2	29.6	0.72	0.91	43.0
25	Т	45	3.0	0.604	6.8	LOS A	4.2	29.6	0.72	0.83	41.5
26	R	340	1.0	0.604	15.6	LOS B	4.2	29.6	0.72	1.01	40.3
Approa	ich	526	2.0	0.604	13.1	LOS A	4.2	29.6	0.72	0.97	41.1
North V	Vest: Nort	hcliffe Drive									
27	L	182	0.0	0.284	6.7	LOS A	1.9	13.5	0.35	0.54	54.1
28	Т	438	2.0	0.284	6.9	LOS A	1.9	13.5	0.36	0.49	55.2
29	R	219	7.0	0.284	12.0	LOS A	1.8	13.4	0.37	0.72	48.9
Approa	ich	839	2.9	0.284	8.2	LOS A	1.9	13.5	0.36	0.56	53.2
South \	Nest: Bed	Iford Street									
30	L	24	0.0	0.106	8.5	LOS A	0.5	3.6	0.69	0.78	43.8
31	Т	27	0.0	0.106	6.0	LOS A	0.5	3.6	0.69	0.68	42.3
32	R	17	6.0	0.106	15.0	LOS B	0.5	3.6	0.69	0.91	41.2
Approa	ıch	68	1.5	0.106	9.1	LOS A	0.5	3.6	0.69	0.77	42.5
All Vehi	icles	2152	2.4	0.604	10.1	LOS A	4.2	29.6	0.59	0.73	48.6

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Tuesday, 15 October 2013 1:50:54 PM SIDRA INTERSECTION 5.1.2.1953

Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Nolan St\_Northcliffe Drive.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE

Site: Nolan / Northcliffe 2031 Existing + Growth + Dev AM

Nolan St / Northcliffe Drive Roundabout

Moven	nent Per	formance - \	Vehicles								
	_	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 1 5		veh/h	%	v/c	sec		veh	m		per veh	km/h
		hcliffe Drive					_		_	_	
21	L	1	0.0	0.339	9.3	LOS A	2.5	18.9	0.75	0.75	52.0
22	Т	545	8.0	0.339	9.9	LOS A	2.5	18.9	0.75	0.74	52.1
23	R	112	13.0	0.339	15.6	LOS B	2.2	17.0	0.75	0.89	46.8
Approac	ch	658	8.8	0.339	10.9	LOS A	2.5	18.9	0.75	0.77	51.1
North E	ast: Nola	n Street									
24	L	103	11.0	0.528	8.7	LOS A	3.2	23.2	0.68	0.83	43.3
25	Т	24	0.0	0.528	6.0	LOS A	3.2	23.2	0.68	0.72	41.7
26	R	325	3.0	0.528	14.9	LOS B	3.2	23.2	0.68	0.97	40.8
Approac	ch	453	4.7	0.528	13.0	LOS A	3.2	23.2	0.68	0.92	41.3
North W	Vest: Nor	thcliffe Drive									
27	L	153	0.0	0.279	6.9	LOS A	1.9	13.6	0.39	0.56	53.9
28	Т	345	9.0	0.279	7.2	LOS A	1.9	13.6	0.39	0.50	54.9
29	R	276	11.0	0.279	12.3	LOS A	1.8	13.6	0.42	0.68	48.3
Approac	ch	774	7.9	0.279	9.0	LOS A	1.9	13.6	0.40	0.58	52.2
South V	Vest: Bed	ford Street									
30	L	31	3.0	0.128	8.4	LOS A	0.6	4.4	0.69	0.78	44.0
31	Т	46	2.0	0.128	5.9	LOS A	0.6	4.4	0.69	0.66	42.6
32	R	8	0.0	0.128	14.7	LOS B	0.6	4.4	0.69	0.93	41.4
Approac	ch	85	2.2	0.128	7.7	LOS A	0.6	4.4	0.69	0.73	42.9
All Vehi	cles	1969	7.2	0.528	10.5	LOS A	3.2	23.2	0.59	0.73	48.4

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Tuesday, 15 October 2013 1:53:13 PM SIDRA INTERSECTION 5.1.2.1953

Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Nolan St\_Northcliffe Drive.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE

Site: Nolan / Northcliffe 2031 Existing + Growth + Dev PM

Nolan St / Northcliffe Drive Roundabout

Moven	nent Per	rformance - \	/ehicles								
		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11 5	- , 11 ,	veh/h	%	v/c	sec		veh	m		per veh	km/h
	ast: Nort	hcliffe Drive									
21	L	14	0.0	0.427	9.7	LOS A	3.5	24.9	0.83	0.79	51.5
22	Т	697	1.0	0.427	10.2	LOS A	3.5	24.9	0.83	0.78	51.5
23	R	104	11.0	0.427	16.2	LOS B	3.2	22.9	0.82	0.93	46.5
Approa	ch	815	2.3	0.427	11.0	LOS A	3.5	24.9	0.83	0.80	50.8
North E	ast: Nola	n Street									
24	L	149	4.0	0.687	11.0	LOS A	5.3	37.7	0.80	1.01	41.7
25	Т	48	3.0	0.687	8.5	LOS A	5.3	37.7	0.80	0.97	40.3
26	R	358	1.0	0.687	17.3	LOS B	5.3	37.7	0.80	1.08	39.3
Approa	ch	556	2.0	0.687	14.8	LOS B	5.3	37.7	0.80	1.05	39.9
North V	Vest: Nort	thcliffe Drive									
27	L	196	0.0	0.342	6.8	LOS A	2.5	17.5	0.39	0.55	53.9
28	Т	537	2.0	0.342	7.0	LOS A	2.5	17.5	0.40	0.50	54.9
29	R	268	7.0	0.342	12.1	LOS A	2.4	17.4	0.42	0.71	48.8
Approa	ch	1001	2.9	0.342	8.3	LOS A	2.5	17.5	0.40	0.57	52.9
South V	Vest: Bed	ford Street									
30	L	24	0.0	0.117	9.0	LOS A	0.6	4.1	0.74	0.82	43.5
31	Т	27	0.0	0.117	6.6	LOS A	0.6	4.1	0.74	0.74	42.0
32	R	17	6.0	0.117	15.5	LOS B	0.6	4.1	0.74	0.93	40.8
Approa	ch	68	1.5	0.117	9.6	LOS A	0.6	4.1	0.74	0.81	42.2
All Vehi	cles	2440	2.5	0.687	10.7	LOS A	5.3	37.7	0.64	0.76	48.2

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

Processed: Tuesday, 15 October 2013 1:54:48 PM SIDRA INTERSECTION 5.1.2.1953

Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Nolan St\_Northcliffe Drive.sip 8000065, GHD SERVICES PTY LTD, ENTERPRISE

Site: Northcliffe/Northbd on/off ramps 2021 Existing + Growth + **Dev AM** 

Northcliffe Dr / Southern Fwy on/off ramps Northbound Roundabout

Mover	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	hcliffe Drive									
22	Т	395	11.0	0.203	6.6	LOS A	0.0	0.0	0.00	0.48	58.9
23	R	292	6.0	0.203	15.4	LOS B	0.0	0.0	0.00	0.77	55.4
Approa	ch	686	8.9	0.203	10.3	LOS A	0.0	0.0	0.00	0.60	57.2
North V	North West: Northcliffe D										
27	L	215	13.0	0.473	16.0	LOS B	3.6	27.9	0.84	0.96	53.6
28	Т	463	9.0	0.473	13.7	LOS A	3.6	27.9	0.84	0.95	49.6
Approa	ch	678	10.3	0.473	14.4	LOS A	3.6	27.9	0.84	0.96	51.0
South V	Vest: Nort	hbound OFF	Ramp								
30	L	400	8.0	0.448	10.0	LOS A	2.2	16.6	0.60	0.83	48.8
31	Т	1	0.0	0.448	9.9	LOS A	2.2	16.6	0.60	0.77	52.9
32	R	571	4.0	0.498	15.2	LOS B	2.7	19.9	0.59	0.90	45.0
Approa	ch	972	5.6	0.498	13.1	LOS A	2.7	19.9	0.60	0.87	46.4
All Vehi	cles	2336	7.9	0.498	12.7	LOS A	3.6	27.9	0.49	0.82	50.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Northcliffe Dr\_Freeway ON-OFF ramp

NTHBD.sip



Site: Northcliffe/Northbd on/off ramps 2021 Existing + Growth + **Dev PM** 

Northcliffe Dr / Southern Fwy on/off ramps Northbound Roundabout

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	ncliffe Drive									
22	Т	403	5.0	0.170	6.4	LOS A	0.0	0.0	0.00	0.47	58.9
23	R	194	1.0	0.170	15.2	LOS B	0.0	0.0	0.00	0.82	55.4
Approac	ch	597	3.7	0.170	9.3	LOS A	0.0	0.0	0.00	0.58	57.5
North W	/est: Nort	hcliffe Drive									
27	L	184	4.0	0.501	14.6	LOS B	4.0	29.1	0.82	0.93	54.9
28	Т	637	3.0	0.501	12.4	LOS A	4.0	29.1	0.82	0.93	50.9
Approac	ch	821	3.2	0.501	12.9	LOS A	4.0	29.1	0.82	0.93	51.9
South V	Vest: Nort	hbound OFF F	Ramp								
30	L	295	7.0	0.335	9.1	LOS A	1.4	10.3	0.53	0.73	49.2
31	Т	1	0.0	0.335	9.0	LOS A	1.4	10.3	0.53	0.68	53.5
32	R	596	3.0	0.504	14.9	LOS B	2.8	20.1	0.57	0.86	45.1
Approac	ch	892	4.3	0.504	13.0	LOS A	2.8	20.1	0.56	0.82	46.3
All Vehi	cles	2309	3.8	0.504	12.0	LOS A	4.0	29.1	0.51	0.80	50.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: Northcliffe/Northbd on/off ramps 2031 Existing + Growth + **Dev AM** 

Northcliffe Dr / Southern Fwy on/off ramps Northbound Roundabout

Moven	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	hcliffe Drive									
22	Т	565	11.0	0.278	6.6	LOS A	0.0	0.0	0.00	0.47	58.9
23	R	376	6.0	0.278	15.4	LOS B	0.0	0.0	0.00	0.78	55.4
Approa	ch	941	9.0	0.278	10.1	LOS A	0.0	0.0	0.00	0.60	57.2
North V	North West: Northcliffe										
27	L	299	13.0	0.729	25.1	LOS B	9.1	70.2	0.99	1.23	45.6
28	Т	641	9.0	0.729	23.3	LOS B	9.1	70.2	0.98	1.22	41.2
Approa	ch	940	10.3	0.729	23.9	LOS B	9.1	70.2	0.98	1.22	42.7
South V	Vest: Nort	thbound OFF	Ramp								
30	L	408	8.0	0.506	11.5	LOS A	2.7	20.3	0.67	0.90	47.3
31	Т	1	0.0	0.506	11.5	LOS A	2.7	20.3	0.67	0.89	51.9
32	R	581	4.0	0.540	16.4	LOS B	3.2	23.3	0.66	0.96	44.2
Approa	ch	991	5.6	0.540	14.4	LOS A	3.2	23.3	0.67	0.94	45.4
All Vehi	cles	2872	8.3	0.729	16.1	LOS B	9.1	70.2	0.55	0.92	47.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Northcliffe Dr\_Freeway ON-OFF ramp NTHBD.sip



Site: Northcliffe/Northbd on/off ramps 2031 Existing + Growth + **Dev PM** 

Northcliffe Dr / Southern Fwy on/off ramps Northbound Roundabout

Moven	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: North	ncliffe Drive									
22	Т	617	5.0	0.248	6.4	LOS A	0.0	0.0	0.00	0.47	58.9
23	R	253	1.0	0.248	15.2	LOS B	0.0	0.0	0.00	0.83	55.4
Approa	ch	869	3.8	0.248	9.0	LOS A	0.0	0.0	0.00	0.58	57.7
North V	Vest: Nortl	hcliffe Drive									
27	L	224	4.0	0.673	19.6	LOS B	7.7	55.2	0.95	1.12	49.9
28	Т	773	3.0	0.673	17.8	LOS B	7.7	55.2	0.94	1.12	45.6
Approa	ch	997	3.2	0.673	18.2	LOS B	7.7	55.2	0.94	1.12	46.6
South V	Vest: Nort	hbound OFF R	Ramp								
30	L	312	7.0	0.390	10.4	LOS A	1.7	13.0	0.61	0.84	48.5
31	Т	1	0.0	0.390	10.3	LOS A	1.7	13.0	0.61	0.80	52.9
32	R	628	3.0	0.570	16.2	LOS B	3.6	25.6	0.66	0.97	44.3
Approa	ch	941	4.3	0.570	14.3	LOS A	3.6	25.6	0.64	0.92	45.5
All Vehi	cles	2807	3.8	0.673	14.0	LOS A	7.7	55.2	0.55	0.89	49.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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NTHBD.sip



Site: Northcliffe/Princes Hwy 2021 Existing + Growth + Dev AM

Northcliffe Dr / Princes Hwy

Signals - Fixed Time Cycle Time = 50 seconds (Practical Cycle Time)

Moven	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northcliffe	Drive									
1	L	271	19.0	0.239	12.6	LOS A	1.6	13.2	0.40	0.74	53.0
3	R	496	7.0	0.876	37.4	LOS C	15.1	112.2	1.00	1.03	32.8
Approa	ch	766	11.2	0.876	28.6	LOS C	15.1	112.2	0.79	0.93	38.2
East: P	rinces Hw	y East									
4	L	435	8.0	0.247	9.6	Χ	X	X	Χ	0.65	54.6
5	Т	349	8.0	0.786	27.8	LOS B	4.8	36.2	1.00	0.93	36.5
Approa	ch	784	8.0	0.786	17.7	LOS B	4.8	36.2	0.45	0.77	44.4
West: P	rinces Hw	vy West									
12	R	301	10.0	0.868	40.5	LOS C	9.1	68.9	1.00	1.02	33.0
Approa	ch	301	10.0	0.868	40.5	LOS C	9.1	68.9	1.00	1.02	33.0
All Vehi	cles	1852	9.7	0.876	25.9	LOS B	15.1	112.2	0.68	0.88	39.6

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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Site: Northcliffe/Princes Hwy 2021 Existing + Growth + Dev PM

Northcliffe Dr / Princes Hwy

Signals - Fixed Time Cycle Time = 50 seconds (Practical Cycle Time)

Movem	nent Perf	ormance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: N	Northcliffe		/0	V/C	366		Veri	'''		per veri	KIII/II
1	L	294	4.0	0.254	13.0	LOS A	2.3	16.4	0.48	0.75	52.1
3	R	402	6.0	0.806	33.1	LOS C	10.8	79.8	0.99	0.96	35.0
Approac	ch	696	5.2	0.806	24.6	LOS B	10.8	79.8	0.77	0.87	40.9
East: Pr	inces Hwy	/ East									
4	L	605	3.0	0.333	9.5	Χ	X	X	Χ	0.65	54.6
5	Т	560	3.0	0.813	26.5	LOS B	7.8	55.7	1.00	0.97	37.2
Approac	ch	1165	3.0	0.813	17.7	LOS B	7.8	55.7	0.48	0.80	44.3
West: P	rinces Hw	y West									
12	R	233	6.0	0.726	34.6	LOS C	6.0	44.5	0.99	0.89	36.0
Approac	ch	233	6.0	0.726	34.6	LOS C	6.0	44.5	0.99	0.89	36.0
All Vehic	cles	2094	4.0	0.813	21.9	LOS B	10.8	79.8	0.63	0.83	42.1

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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Site: Northcliffe/Princes Hwy 2031 Existing + Growth + Dev AM

Northcliffe Dr / Princes Hwy

Signals - Fixed Time Cycle Time = 90 seconds (Practical Cycle Time)

Movem	nent Perf	ormance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: N	Northcliffe		,,	.,,			7 0.1.			po. 1011	,
1	L	298	19.0	0.235	12.7	LOS A	2.7	21.7	0.30	0.72	53.1
3	R	545	7.0	0.867	48.7	LOS D	26.4	196.2	1.00	0.97	28.3
Approac	ch	843	11.2	0.867	36.0	LOS C	26.4	196.2	0.75	0.88	34.2
East: Pr	inces Hw	y East									
4	L	478	8.0	0.272	9.6	Χ	X	Χ	Χ	0.65	54.6
5	Т	385	8.0	0.850	49.9	LOS D	9.5	71.3	1.00	0.97	27.6
Approac	ch	863	8.0	0.850	27.6	LOS B	9.5	71.3	0.45	0.79	37.5
West: P	rinces Hw	y West									
12	R	497	10.0	0.890	55.0	LOS D	25.6	194.3	1.00	0.98	27.3
Approac	ch	497	10.0	0.890	55.0	LOS D	25.6	194.3	1.00	0.98	27.3
All Vehic	cles	2203	9.7	0.890	37.0	LOS C	26.4	196.2	0.69	0.87	33.5

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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Site: Northcliffe/Princes Hwy 2031 Existing + Growth + Dev PM

Northcliffe Dr / Princes Hwy

Signals - Fixed Time Cycle Time = 70 seconds (Practical Cycle Time)

Moven	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northcliffe	Drive									
1	L	376	4.0	0.304	13.5	LOS A	4.0	28.8	0.44	0.75	51.6
3	R	511	6.0	0.873	44.0	LOS D	20.4	150.3	1.00	0.99	30.0
Approa	ch	886	5.2	0.873	31.1	LOS C	20.4	150.3	0.76	0.89	36.8
East: P	rinces Hw	y East									
4	L	672	3.0	0.369	9.5	Χ	X	X	Χ	0.65	54.6
5	T	623	3.0	0.877	39.7	LOS C	12.6	90.6	1.00	1.03	31.1
Approa	ch	1295	3.0	0.877	24.0	LOS B	12.6	90.6	0.48	0.84	39.6
West: P	rinces Hw	vy West									
12	R	367	6.0	0.903	52.7	LOS D	15.8	116.0	1.00	1.02	28.0
Approa	ch	367	6.0	0.903	52.7	LOS D	15.8	116.0	1.00	1.02	28.0
All Vehi	cles	2548	4.2	0.903	30.6	LOS C	20.4	150.3	0.65	0.88	36.6

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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Site: Northcliffe Dr / Southern FWY Southbd on/off ramps 2021 Existing + Growth + Dev AM

Northcliffe Drive / Southern Freeway SOUTHBOUND on/off ramps Roundabout

Movem	nent Per	rformance -	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nort	thcliffe Drive	,,,	• • • • • • • • • • • • • • • • • • • •			7011			por vori	1311/11
21	L	429	6.0	0.468	12.3	LOS A	3.2	23.3	0.68	0.80	56.1
22	Т	495	6.0	0.468	10.1	LOS A	3.2	23.3	0.69	0.79	52.7
Approac	ch	924	6.0	0.468	11.1	LOS A	3.2	23.3	0.69	0.79	54.4
North Ea	ast: Sout	thbound OFF I	Ramp								
24	L	196	10.0	0.248	10.1	LOS A	1.0	7.3	0.59	0.80	48.8
25	Т	1	100.0	0.232	12.5	LOS A	0.9	7.2	0.57	0.61	55.4
26	R	215	14.0	0.232	15.6	LOS B	0.9	7.2	0.57	0.88	45.0
Approac	ch	412	12.3	0.248	13.0	LOS A	1.0	7.3	0.58	0.84	46.7
North W	est: Nort	thcliffe Drive									
28	Т	721	5.0	0.299	6.4	LOS A	0.0	0.0	0.00	0.47	58.9
29	R	313	10.0	0.299	15.6	LOS B	0.0	0.0	0.00	0.82	55.4
Approac	ch	1034	6.5	0.299	9.2	LOS A	0.0	0.0	0.00	0.58	57.6
All Vehic	cles	2369	7.3	0.468	10.6	LOS A	3.2	23.3	0.37	0.71	54.2

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Northcliffe Dr\_Freeway ON-OFF ramp

STHBD.sip



Site: Northcliffe Dr / Southern FWY Southbd on/off ramps 2021 Existing + Growth + Dev PM

Northcliffe Drive / Southern Freeway SOUTHBOUND on/off ramps Roundabout

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nort	hcliffe Drive									
21	L	683	2.0	0.667	15.6	LOS B	7.0	49.9	0.83	0.98	53.2
22	Т	387	4.0	0.476	11.3	LOS A	3.2	23.3	0.73	0.86	52.4
Approac	ch	1071	2.7	0.667	14.0	LOS A	7.0	49.9	0.79	0.94	53.0
North E	ast: Sout	hbound OFF R	amp								
24	L	287	2.0	0.283	9.4	LOS A	1.2	8.6	0.61	0.76	48.8
25	Т	1	0.0	0.283	9.5	LOS A	1.2	8.6	0.61	0.71	52.9
26	R	231	5.0	0.295	16.5	LOS B	1.2	8.7	0.63	0.89	44.1
Approac	ch	519	3.3	0.295	12.5	LOS A	1.2	8.7	0.62	0.82	46.5
North W	lest: Nort	hcliffe Drive									
28	Т	803	3.0	0.348	6.4	LOS A	0.0	0.0	0.00	0.48	58.9
29	R	419	3.0	0.348	15.3	LOS B	0.0	0.0	0.00	0.81	55.4
Approac	ch	1222	3.0	0.348	9.4	LOS A	0.0	0.0	0.00	0.59	57.5
All Vehic	cles	2812	3.0	0.667	11.8	LOS A	7.0	49.9	0.42	0.76	53.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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STHBD.sip



Site: Northcliffe Dr / Southern FWY Southbd on/off ramps 2031 Existing + Growth + Dev AM

Northcliffe Drive / Southern Freeway SOUTHBOUND on/off ramps Roundabout

Moven	nent Per	formance -	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	hcliffe Drive									
21	L	529	6.0	0.628	15.6	LOS B	6.1	44.8	0.83	0.98	53.5
22	Т	594	6.0	0.628	13.7	LOS A	6.1	44.8	0.83	0.99	49.7
Approac	ch	1123	6.0	0.628	14.6	LOS B	6.1	44.8	0.83	0.99	51.6
North E	ast: Soutl	hbound OFF I	Ramp								
24	L	238	10.0	0.328	11.1	LOS A	1.4	10.4	0.64	0.84	47.8
25	Т	2	100.0	0.328	14.1	LOS A	1.4	10.4	0.64	0.70	55.6
26	R	295	14.0	0.328	16.2	LOS B	1.4	11.0	0.63	0.90	44.5
Approac	ch	535	12.6	0.328	13.9	LOS A	1.4	11.0	0.63	0.87	45.9
North W	lest: Nort	hcliffe Drive									
28	Т	818	5.0	0.340	6.4	LOS A	0.0	0.0	0.00	0.47	58.9
29	R	356	10.0	0.340	15.6	LOS B	0.0	0.0	0.00	0.82	55.4
Approac	ch	1174	6.5	0.340	9.2	LOS A	0.0	0.0	0.00	0.58	57.6
All Vehi	cles	2832	7.5	0.628	12.2	LOS A	6.1	44.8	0.45	0.80	52.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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STHBD.sip



Site: Northcliffe Dr / Southern FWY Southbd on/off ramps 2031 Existing + Growth + Dev PM

Northcliffe Drive / Southern Freeway SOUTHBOUND on/off ramps Roundabout

Moven	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: Nortl	ncliffe Drive									
21	L	801	2.0	0.866	26.7	LOS B	16.7	119.2	1.00	1.37	43.8
22	Т	440	4.0	0.609	15.2	LOS B	5.4	38.9	0.85	1.03	48.2
Approac	ch	1241	2.7	0.866	22.6	LOS B	16.7	119.2	0.95	1.25	45.1
North E	ast: Soutl	nbound OFF R	amp								
24	L	358	2.0	0.362	10.1	LOS A	1.7	12.2	0.66	0.84	48.5
25	Т	1	0.0	0.362	10.2	LOS A	1.7	12.2	0.66	0.78	52.5
26	R	311	5.0	0.416	18.0	LOS B	2.0	14.7	0.69	0.96	43.0
Approac	ch	669	3.4	0.416	13.7	LOS A	2.0	14.7	0.67	0.89	45.7
North W	lest: Nort	hcliffe Drive									
28	Т	884	3.0	0.383	6.4	LOS A	0.0	0.0	0.00	0.48	58.9
29	R	462	3.0	0.383	15.3	LOS B	0.0	0.0	0.00	0.81	55.4
Approac	ch	1346	3.0	0.383	9.4	LOS A	0.0	0.0	0.00	0.59	57.5
All Vehi	cles	3257	3.0	0.866	15.4	LOS B	16.7	119.2	0.50	0.90	49.6

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Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Project: G:\21\21878\Tech\SIDRA Modelling\2013-10-15 SIDRA Models\Northcliffe Dr\_Freeway ON-OFF ramp STHBD.sip

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Site: Nolan St / Princes Hwy 2021 Existing + Growth + Dev AM

Nolan Street / Princes Hwy

Signals - Fixed Time Cycle Time = 60 seconds (Practical Cycle Time)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy South									
2	Т	692	5.0	0.606	11.7	LOS A	12.5	90.9	0.77	0.67	47.9
3	R	139	15.0	0.606	29.8	LOS C	6.0	45.7	0.96	0.84	35.9
Approac	ch	831	6.7	0.606	14.7	LOS B	12.5	90.9	0.80	0.70	45.6
East: No	olan Stree	et									
4	L	57	14.0	0.205	17.6	LOS B	0.9	7.2	0.58	0.71	37.6
6	R	340	8.0	0.726	32.1	LOS C	9.9	74.2	0.97	0.89	30.3
Approac	ch	397	8.9	0.726	30.0	LOS C	9.9	74.2	0.91	0.87	31.2
North: F	rinces H	wy North									
7	L	462	7.0	0.837	28.6	LOS C	15.6	115.1	0.96	1.02	36.2
8	Т	698	5.0	0.837	24.8	LOS B	17.1	124.5	0.98	0.99	36.9
Approac	ch	1160	5.8	0.837	26.3	LOS B	17.1	124.5	0.97	1.00	36.6
All Vehic	cles	2387	6.6	0.837	22.9	LOS B	17.1	124.5	0.90	0.88	38.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	53	24.3	LOS C	0.1	0.1	0.90	0.90				
P3	Across E approach	53	18.4	LOS B	0.1	0.1	0.78	0.78				
All Pede	estrians	106	21.4	LOS C			0.84	0.84				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Nolan St / Princes Hwy 2021 Existing + Growth + Dev PM

Nolan Street / Princes Hwy

Signals - Fixed Time Cycle Time = 60 seconds (Practical Cycle Time)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy South									
2	Т	651	3.0	0.465	12.2	LOS A	8.6	61.4	0.74	0.63	47.5
3	R	64	23.0	0.465	24.9	LOS B	6.2	46.0	0.83	0.90	40.9
Approac	ch	715	4.8	0.465	13.3	LOS A	8.6	61.4	0.75	0.65	46.9
East: No	olan Stree	et									
4	L	129	4.0	0.409	16.5	LOS B	2.0	14.8	0.57	0.73	38.2
6	R	459	2.0	0.836	35.6	LOS C	15.0	107.0	1.00	0.98	28.9
Approac	ch	588	2.4	0.836	31.4	LOS C	15.0	107.0	0.90	0.93	30.5
North: P	rinces H	wy North									
7	L	376	6.0	0.804	27.3	LOS B	12.7	92.6	0.96	0.98	37.2
8	Т	634	2.0	0.804	23.6	LOS B	14.4	102.4	0.98	0.95	37.6
Approac	ch	1009	3.5	0.804	25.0	LOS B	14.4	102.4	0.97	0.96	37.5
All Vehic	cles	2313	3.6	0.836	23.0	LOS B	15.0	107.0	0.88	0.86	37.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	s					
		Demand	Average		Average Back		Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	22.5	LOS C	0.1	0.1	0.87	0.87
P3	Across E approach	53	20.0	LOS C	0.1	0.1	0.82	0.82
All Pede	estrians	106	21.3	LOS C			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Nolan St / Princes Hwy 2031 Existing + Growth + Dev AM

Nolan Street / Princes Hwy

Signals - Fixed Time Cycle Time = 60 seconds (Practical Cycle Time)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy South									
2	Т	781	5.0	0.693	12.6	LOS A	15.3	111.6	0.82	0.73	46.9
3	R	157	15.0	0.693	32.4	LOS C	6.6	51.1	0.98	0.89	34.3
Approac	ch	938	6.7	0.693	15.9	LOS B	15.3	111.6	0.84	0.76	44.4
East: No	olan Stree	et									
4	L	57	14.0	0.205	17.6	LOS B	0.9	7.2	0.58	0.71	37.6
6	R	340	8.0	0.726	32.1	LOS C	9.9	74.2	0.97	0.89	30.3
Approac	ch	397	8.9	0.726	30.0	LOS C	9.9	74.2	0.91	0.87	31.2
North: F	rinces H	wy North									
7	L	482	7.0	0.881	34.2	LOS C	19.2	142.0	0.99	1.08	33.0
8	Т	737	5.0	0.881	29.5	LOS C	19.7	144.0	1.00	1.07	34.3
Approac	ch	1219	5.8	0.881	31.4	LOS C	19.7	144.0	0.99	1.08	33.8
All Vehic	cles	2554	6.6	0.881	25.5	LOS B	19.7	144.0	0.92	0.93	36.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movem	nent Performance -	Pedestrian	s					
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	24.3	LOS C	0.1	0.1	0.90	0.90
P3	Across E approach	53	18.4	LOS B	0.1	0.1	0.78	0.78
All Pede	estrians	106	21.4	LOS C			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Nolan St / Princes Hwy 2031 Existing + Growth + Dev PM

Nolan Street / Princes Hwy

Signals - Fixed Time Cycle Time = 70 seconds (Practical Cycle Time)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Princes H	wy South									
2	Т	777	3.0	0.585	15.6	LOS B	13.6	97.5	0.80	0.69	44.2
3	R	76	23.0	0.585	30.6	LOS C	8.6	64.3	0.90	0.87	36.9
Approac	ch	853	4.8	0.585	16.9	LOS B	13.6	97.5	0.81	0.71	43.5
East: No	olan Stree	et									
4	L	140	4.0	0.501	18.0	LOS B	2.6	18.7	0.57	0.74	37.2
6	R	489	2.0	0.814	36.2	LOS C	17.5	124.6	0.98	0.95	28.7
Approac	ch	629	2.4	0.814	32.1	LOS C	17.5	124.6	0.89	0.90	30.2
North: F	Princes H	vy North									
7	L	412	6.0	0.838	32.0	LOS C	18.0	131.1	0.97	1.00	34.3
8	Т	716	2.0	0.838	28.2	LOS B	19.6	139.6	0.99	0.99	35.0
Approac	ch	1127	3.5	0.838	29.6	LOS C	19.6	139.6	0.98	0.99	34.8
All Vehic	cles	2609	3.6	0.838	26.0	LOS B	19.6	139.6	0.90	0.88	35.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	s					
		Demand	Average		Average Back		Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	23.2	LOS C	0.1	0.1	0.81	0.81
P3	Across E approach	53	20.8	LOS C	0.1	0.1	0.77	0.77
All Pede	estrians	106	22.0	LOS C			0.79	0.79

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Warwick / Nolan 2021 Existing + Growth + Dev AM

Warrick St / Nolan St Stop (Two-Way)

		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Nolan Stre	eet South								· ·	
1	L	51	0.0	0.211	6.4	LOS A	0.0	0.0	0.00	0.86	43.3
2	Т	347	5.0	0.211	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approa	ch	398	4.4	0.211	0.8	NA	0.0	0.0	0.00	0.11	49.0
North: N	Nolan Stre	et North									
8	Т	272	9.0	0.190	2.3	LOS A	1.5	10.9	0.54	0.00	43.6
9	R	40	0.0	0.190	9.1	LOS A	1.5	10.9	0.54	0.90	42.4
Approach		312	7.8	0.190	3.2	NA	1.5	10.9	0.54	0.12	43.5
West: V	Varrick St	reet									
10	L	38	10.0	0.297	20.0	LOS B	1.2	8.9	0.67	0.99	34.7
12	R	67	0.0	0.297	19.4	LOS B	1.2	8.9	0.67	1.05	34.8
Approa	ch	105	3.6	0.297	19.6	LOS B	1.2	8.9	0.67	1.03	34.7
All Vehicles		815	5.6	0.297	4.2	NA	1.5	10.9	0.29	0.23	44.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: Warwick / Nolan 2021 Existing + Growth + Dev PM

Warrick St / Nolan St Stop (Two-Way)

Moven	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed
South: I	Nolan Stre	veh/h	%	v/c	sec		veh	m		per veh	km/h
JOUIII. 1	ı volalı olle		0.0	0.470	C 4	1.00.4	0.0	0.0	0.00	0.05	40.0
1	L	52	0.0	0.173	6.4	LOS A	0.0	0.0	0.00	0.85	43.3
2	T	262	12.0	0.173	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approa	ch	314	10.0	0.173	1.1	NA	0.0	0.0	0.00	0.14	48.8
North: N	Nolan Stre	et North									
8	Т	438	2.0	0.287	2.1	LOS A	2.4	17.2	0.55	0.00	43.5
9	R	61	0.0	0.287	8.9	LOS A	2.4	17.2	0.55	0.88	42.7
Approa	ch	499	1.8	0.287	2.9	NA	2.4	17.2	0.55	0.11	43.4
West: V	Varrick Str	eet									
10	L	38	0.0	0.298	21.2	LOS B	1.2	8.4	0.67	0.94	33.7
12	R	57	0.0	0.298	21.0	LOS B	1.2	8.4	0.67	1.05	33.8
Approa	ch	95	0.0	0.298	21.1	LOS B	1.2	8.4	0.67	1.00	33.8
All Vehi	cles	907	4.4	0.298	4.2	NA	2.4	17.2	0.37	0.21	43.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: Warwick / Nolan 2031 Existing + Growth + Dev AM

Warrick St / Nolan St Stop (Two-Way)

		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Nolan Stre	eet South									
1	L	51	0.0	0.224	6.4	LOS A	0.0	0.0	0.00	0.87	43.3
2	Т	372	5.0	0.224	0.0	LOS A	0.0	0.0	0.00	0.00	50.0
Approa	ch	422	4.4	0.224	0.8	NA	0.0	0.0	0.00	0.10	49.1
North: N	Nolan Stre	et North									
8	Т	273	9.0	0.192	2.5	LOS A	1.5	11.2	0.56	0.00	43.4
9	R	40	0.0	0.192	9.3	LOS A	1.5	11.2	0.56	0.91	42.3
Approach		313	7.8	0.192	3.4	NA	1.5	11.2	0.56	0.12	43.3
West: V	Varrick St	reet									
10	L	38	10.0	0.312	20.9	LOS B	1.3	9.4	0.69	1.02	34.2
12	R	67	0.0	0.312	20.3	LOS B	1.3	9.4	0.69	1.05	34.3
Approa	ch	105	3.6	0.312	20.5	LOS B	1.3	9.4	0.69	1.04	34.2
All Vehicles		840	5.6	0.312	4.2	NA	1.5	11.2	0.29	0.23	44.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: Warwick / Nolan 2031 Existing + Growth + Dev PM

Warrick St / Nolan St Stop (Two-Way)

		formance - \ Demand		Dog	Average	l avial af	95% Back	of Outque	Duan	Etta ationa	A.,
Mov ID	Turn	Flow	HV	Deg.	Average Delay	Level of	Vehicles	Distance	Prop.	Effective	Average
	Tuill	veh/h	%	Satn v/c	Sec	Service	venicies veh	Distance	Queued	Stop Rate per veh	Speed km/h
South: I	Nolan Stre		70	V/C	360		Ven			per veri	KIII/II
1	1	52	0.0	0.185	6.4	LOS A	0.0	0.0	0.00	0.85	43.3
2	T	284	12.0	0.185	0.0	LOSA	0.0	0.0	0.00	0.00	50.0
Approa	ch	336	10.2	0.185	1.0	NA	0.0	0.0	0.00	0.13	48.8
North: N	Nolan Stre	et North									
8	Т	457	2.0	0.299	2.3	LOS A	2.6	18.6	0.58	0.00	43.2
9	R	61	0.0	0.299	9.1	LOS A	2.6	18.6	0.58	0.90	42.6
Approach		518	1.8	0.299	3.1	NA	2.6	18.6	0.58	0.11	43.1
West: V	Varrick St	reet									
10	L	38	0.0	0.324	23.0	LOS B	1.3	9.3	0.70	0.97	32.8
12	R	57	0.0	0.324	22.8	LOS B	1.3	9.3	0.70	1.06	32.9
Approa	ch	95	0.0	0.324	22.9	LOS B	1.3	9.3	0.70	1.02	32.9
All Vehicles		948	4.6	0.324	4.3	NA	2.6	18.6	0.39	0.21	43.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

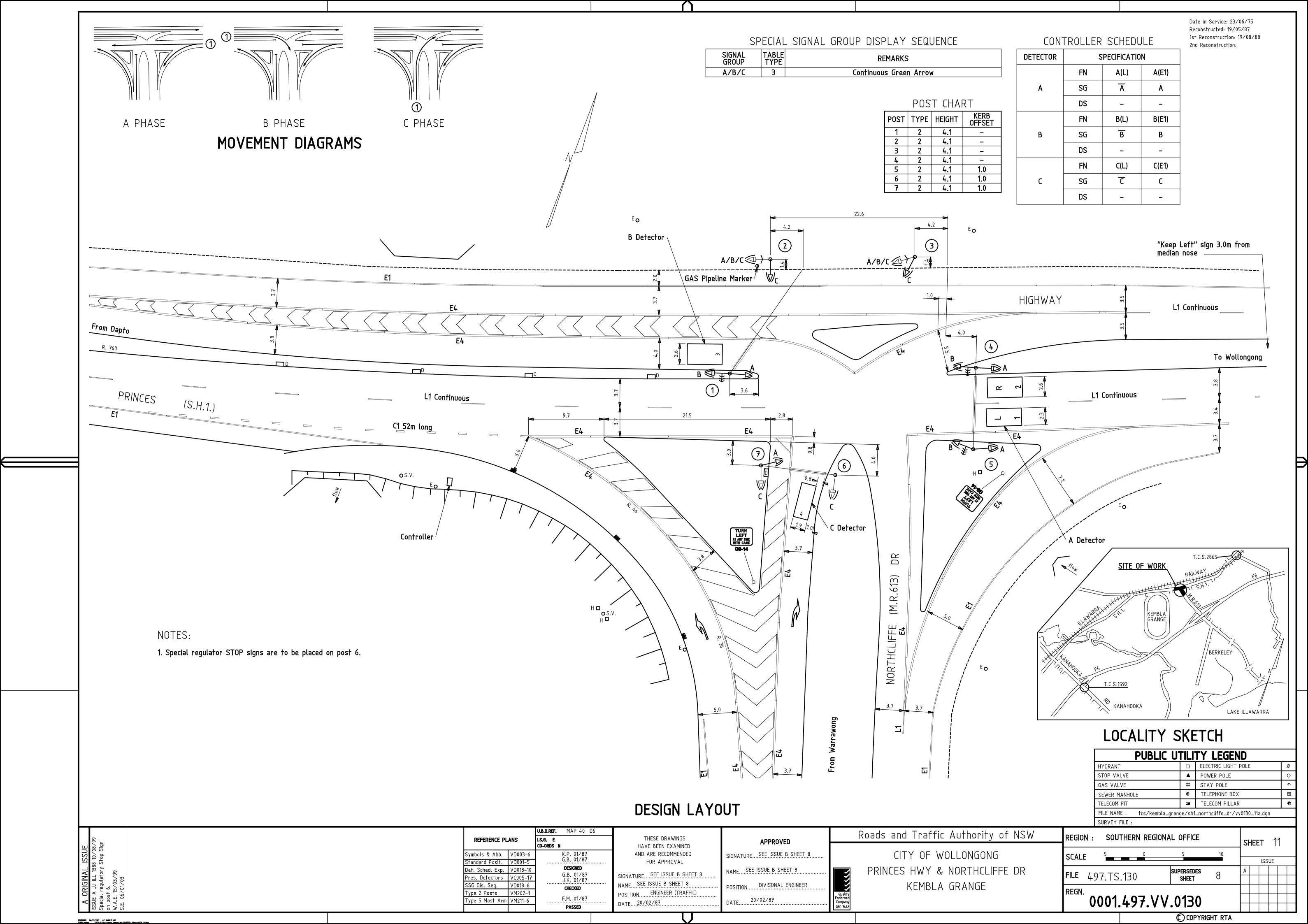
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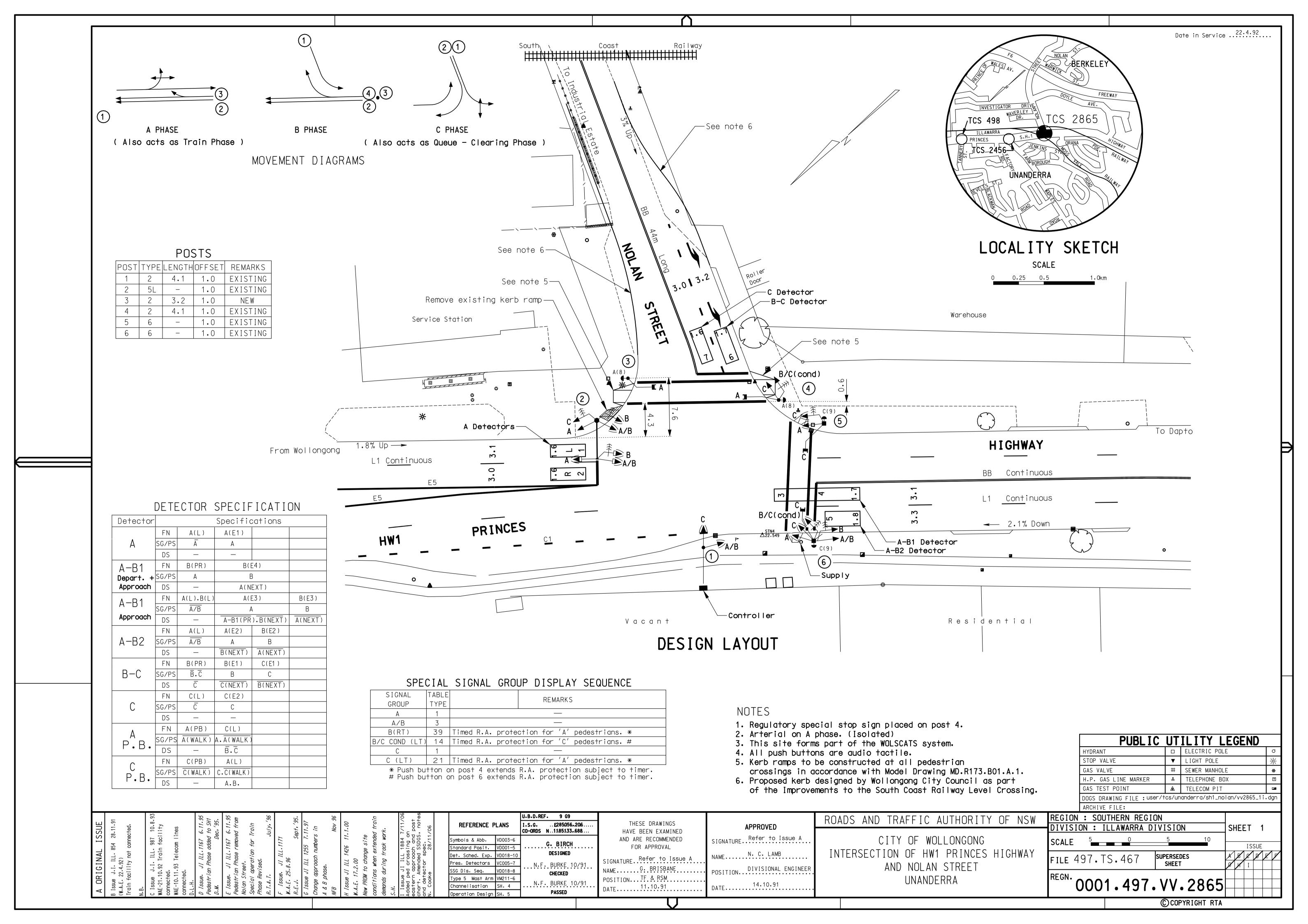
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# Appendix C – RMS Signal Plans





# Appendix D - Crash Data

### **Detailed Crash Report**



Crash No.	Date	Day of Week	Time	Distance	Feature	Loc Type	Alignment	Weather	Surface Condition	) eq	or I us	Type/Obj	/de/Sex	Street Travelling	Speed Travelling	anoeuvre	Degree of Crash	rilled Injured	Factors
ပັ	ے ت	۵	Ē	٥			₹	>	တ် ပိ	<u> </u>	<u>8</u>	<u>L</u>	ď	ı t	Ş <u>⊢</u>	Ž	<u>~~</u>	2 =	
																			ASF
	n Region																		
	ngong City keley	LGA																	
	cheshire St																		
	20/01/2007	Sat	09:43		at NOLAN ST	TJN	STR	Fine	Dry	50	1 1	ITE	M21	N in NOLAN ST	70 Proceed	ing in land	I C	) 1	s
E29443004	20/01/2007	Jai	09.43		at NOLAN ST	RUM:	73	Off rd rght =>	•	30		ree/bu		N III NOLAN 31	70 1100000	ing in lane	1	, ,	3
	Sallop St					TOW!	7.5	On rungin =>	ODj		'	166/00	usii						
	25/06/2010	Fri	10:15		at NOLAN ST	TJN	STR	Fine	Dry	50	2 C	AR	F78	N in NOLAN ST	20 Turning	riaht	1 (	) 1	
E42292416						RUM:	21	Right through	•					S in NOLAN ST	40 Proceed	•			
	23/07/2010	Fri	09:20		at NOLAN ST	TJN	STR	Fine	Dry	50	2 C			W in GALLOP ST	5 Turning	•	1 (	0 1	
E41898971						RUM:	13	Right near			٧	VAG	M51	S in NOLAN ST	40 Proceed	ing in lane			
H	lopman Cre	es																	
763906	15/08/2011	Mon	18:30		at NOLAN ST	TJN	STR	Fine	Dry	50	2 C	AR	M19	S in NOLAN ST	20 Proceed	ing in lane	1 (	) 1	
E45319347						RUM:	32	Right rear			C	AR	M63	S in NOLAN ST	0 Wait turi	n right			
	lolan St																		
561738	16/03/2007	Fri	19:10	5 m	N CHESHIRE ST	TJN	CRV	Raining	Wet	50				S in NOLAN ST	50 Proceed	•	N C	0	
E29818479	00/04/0044			0.45		RUM:		Right rear						S in NOLAN ST	15 Turning	•			
759875	29/04/2011	Fri	19:30	245 m	N HOPMAN CRES	2WY	CRV	0	Wet	50				N in NOLAN ST	20 Proceed	ing in lane	N C	0 0	S
E44280154	0.4/0.0/0.000		47.00		· NODEOLK OT	RUM:	87	Off lft/lft bnd=	-	50		uildin	_	W: NODEOLKOT	5.0				
607693	04/02/2008	IVION	17:00		at NORFOLK ST	TJN	STR	- 3	Wet	50				W in NORFOLK ST		ing in lane	1 (	) 2	
E33010558 607319	08/12/2007	Sat	00:20	75 m	N NORFOLK ST	RUM: 2WY	10 CRV	Cross traffic Fine	Dry	50		AR RK		S in NOLAN ST W in NOLAN ST	40 Proceed	ing in lane from drive	1 (	) 1	Α
E32110776	00/12/2007	Jai	00.20	73111	IN NORTOLK 31	RUM:	47	Emerging from	•	30		1/C		N in NOLAN ST	Unk Along fo		1	, ,	Α
666514	07/05/2009	Thu	22:15	300 m	N NOTTINGHAM ST	2WY	CRV	Fine	Dry	60				S in NOLAN ST	60 Proceed	•	I C	) 3	s
E37505703	0.700/2000			000		RUM:		Off lft/lft bnd=	,			RK		S in NOLAN ST	0 Parked	gae			Ü
621933	25/04/2008	Fri	07:30		at ROCHE PL	TJN	STR	Fine	Dry	50			M20	N in NOLAN ST	30 Incorrec	t side	I C	) 1	F
E33340475						RUM:	20	Head on	-		4	WD	M42	S in NOLAN ST	15 Proceed	ing in lane			
763461	30/07/2011	Sat	14:00		at WARWICK ST	TJN	STR	Fine	Dry	60	2 C	AR	M24	E in WARWICK ST	5 Turning	right	N C	0 0	
E47750088						RUM:	11	Right far			C	AR	F31	S in NOLAN ST	60 Proceed	ing in lane			
Una	anderra																		
	Ooyle Ave																		

### **Detailed Crash Report**



Crash No.	<b>t</b> e	y of Week	Time	Distance ID Feature	c Type	Alignment	Weather	Surface Condition	Speed Limit	ΣŽ		Street Travelling	Speed Travelling	anoeuvre	Degree of Crash	Killed	Injured	Factors
ຮັ	Date	Day	_ <u>≒</u>	ă <u>Q</u>	Loc	₹	×	တ္တ လိ	Sp	2	Ag	Str	SP FE	Ma	تَ ق	茎	Ē	. <u>Б</u>
																		ASF
779836	21/11/2011	Mon	10:30	at NOLAN ST	TJN	STR	Overcast	Dry	50 2	CAR	M21	E in DOYLE AVE	30 Proceeding	in lane	N	0	0	
E48158085					RUM:	32	Right rear			UTE	M66	E in DOYLE AVE	0 Wait turn ri	ght				
l:	nvestigator	Dr																
628015	20/06/2008	Fri	11:29	at NOLAN ST	TJN	STR	Fine	Dry	60 2	4WD	M55	S in INVESTIGATOR DR	10 Turning rig	nt	N	0	0	
E34170506					RUM:	13	Right near			CAR	F62	E in NOLAN ST	60 Proceeding	in lane				
755772	07/06/2011	Tue	10:15	at NOLAN ST	TJN	STR	Fine	Dry	50 2	TRK	M68	S in INVESTIGATOR DR	30 Proceeding	in lane	N	0	0	
E46588980					RUM:	10	Cross traffic			CAR	F54	W in NOLAN ST	50 Proceeding	in lane				
N	Nolan St																	
748469	09/04/2011	Sat	21:30	100 m E DOYLE AVE	2WY	CRV	Fine	Dry	60 1	CAR	M18	E in NOLAN ST	Unk Proceeding	in lane	N	0	0	ASF
E44345340					RUM:	81	Off left/rt bnd	=>obj		Fenc	е							
688268	03/11/2009	Tue	17:50	20 m E PRINCES HWY	2WY	STR	Fine	Dry	50 2	CAR	M84	W in NOLAN ST	40 Proceeding	in lane	I	0	1	Α
E39597642					RUM:	30	Rear end			CAR		W in NOLAN ST	0 Stationary					
602449	10/11/2007	Sat	07:00	60 m E PRINCES HWY	2WY	STR	Raining	Wet	60 1	P/C	Fυ	NOLAN ST	Proceeding	in lane	I	0	1	
E32542158					RUM:		On road-out											
671162	15/06/2009	Mon	09:55	200 m E PRINCES HWY	2WY	STR	Fine	Dry	60 2			E in NOLAN ST	40 Turning rigi		I	0	1	
E38169067					RUM:		Right through			CAR		W in NOLAN ST	40 Proceeding					
767815	17/09/2011	Sat	09:20	175 m N SOUTHERN EXPRE OP	2WY	CRV	Fine	Dry	60 1	CAR	_	N in NOLAN ST	Unk Proceeding	in lane	ı	0	1	SF
E47868084					RUM:		Off rt/lft bnd=	•		Fenc								
667678	24/02/2009	Tue	17:00	at WAVERLEY DR	TJN	STR	Fine	Dry	50 1	M/C	M44	W in NOLAN ST	20 Proceeding	in lane	I	0	1	
E37801177					RUM:		On road-out									_		
685358	08/10/2009	Thu	08:50	at WAVERLEY DR	TJN	STR	Fine	Dry	50 2			S in WAVERLEY DR	20 Turning rigi		N	0	0	
E39176674	00/0=/00/				RUM:		Right near	_		CAR		E in NOLAN ST	40 Proceeding			_	_	
759420	08/07/2011	Fri	14:10	at WAVERLEY DR	TJN	STR	Fine	Dry	50 2			W in NOLAN ST	20 Turning rigi		N	0	0	
E44915006					RUM:	39	Other same of			TRK	M38	W in NOLAN ST	5 Proceeding					
Report To	otals:	7	Total Cra	ishes: 22 Fatal Cras	shes: 0		Injury	/ Crashes	: 13			Killed: 0	Injured:	16				
Crookid d	otocot Nolon	C+ E	Porkolovi	lon 2007 to Dog 2011														

Crashid dataset Nolan St - Berkeley - Jan 2007 to Dec 2011

### **Summary Crash Report**



# Crash Type		
Car Crash	18	81.8%
Light Truck Crash	5	22.7%
Rigid Truck Crash	1	4.5%
Articulated Truck Crash	0	0.0%
'Heavy Truck Crash	(1)	(4.5%)
Bus Crash	1	4.5%
"Heavy Vehicle Crash	(2)	(9.1%)
Emergency Vehicle Crash	0	0.0%
Motorcycle Crash	2	9.1%
Pedal Cycle Crash	1	4.5%
Pedestrian Crash	0	0.0%
' Pigid or Artic Truck " Howay Truck	or U	Duo Puo

Rigid or Artic. Truck " Heavy Truck or Heavy Bus # These categories are NOT mutually exclusive

Location Type		
*Intersection	14	63.6%
Non intersection	8	36.4%

\* Up to 10 metres from an intersection

~ 07:30-09:30 or 14:30-17:00 on school days

Collision Type		
Single Vehicle	6	27.3%
Multi Vehicle	16	72.7%

Road Classification												
Freeway/Motorway	0	0.0%										
State Highway	0	0.0%										
Other Classified Road	0	0.0%										
Unclassified Road	22	100.0%										

Contributin	g Factor	s
Speeding	5	22.7%
Fatigue	3	13.6%
Alcohol	3	13.6%
Weat	her	
Fine	17	77.3%
Rain	4	18.2%
Overcast	1	4.5%
Fog or mist	0	0.0%
Other	0	0.0%
Road Surface	e Conditi	on
Wet	4	18.2%
Dry	18	81.8%
Snow or ice	0	0.0%
Natural L	ighting	
Dawn	0	0.0%
Daylight	16	72.7%
Dusk	0	0.0%
Darkness	6	27.3%

	Crash Movement				CRAS	HES	;	22		CASU
Intersect	ion, adjacent approaches		6	27.3%	Fatal crash		0	0.0%	Kil	lled
Head-on	(not overtaking)		1	4.5%	Injury crash		13	59.1%	lnj	ured
Opposin	g vehicles; turning		2	9.1%	Non-casualty cras	h	9	40.9%	) ^ l	Jnrestraine
U-turn			0	0.0%	^ Belt fitted but not wor	n, No	restrai	nt fitted to	posi	tion OR No he
Rear-end	I		4	18.2%	Time Group		%	of Day		Crashes
Lane cha	inge		0	0.0%	00:01 - 02:59	1	4.5%	12.5%		8 :
Parallel I	anes; turning		0	0.0%	03:00 - 04:59	0	0.0%	8.3%		2
Vehicle I	eaving driveway		1	4.5%	05:00 - 05:59	0	0.0%	4.2%		5
Overtaki	ng; same direction		0	0.0%	06:00 - 06:59	0	0.0%	4.2%		3 :
Hit parke	d vehicle		0	0.0%	07:00 - 07:59	2	9.1%	4.2%		4
Hit railwa	ay train		0	0.0%	08:00 - 08:59	1	4.5%	4.2%		
Hit pedes	strian		0	0.0%	09:00 - 09:59	4	18.2%	4.2%		
Permane	nt obstruction on road		0	0.0%	10:00 - 10:59	3	13.6%	4.2%		
Hit anima	al		0	0.0%	11:00 - 11:59	1	4.5%	4.2%		~ School
Off road,	on straight		0	0.0%	12:00 - 12:59	0	0.0%	4.2%	In	volvement
Off road	on straight, hit object		1	4.5%	13:00 - 13:59	0	0.0%	4.2%		
Out of co	ontrol on straight		2	9.1%	14:00 - 14:59	2	9.1%	4.2%	M	cLean Perio
Off road,	on curve		0	0.0%	15:00 - 15:59	0	0.0%	4.2%	A	2
Off road	on curve, hit object		4	18.2%	16:00 - 16:59	0	0.0%	4.2%	В	1
Out of co	ontrol on curve		0	0.0%	17:00 - 17:59	3	13.6%	4.2%	С	7
Other cra	ash type		1	4.5%	18:00 - 18:59	1	4.5%	4.2%	D	3
					19:00 - 19:59	2	9.1%	4.2%	E	0
	~ 40km/h or less		0	0.0%	20:00 - 21:59	1	4.5%	8.3%	F	4
0.0%	80 km/h zone	0		0.0%	22:00 - 24:00	1	4.5%	8.3%	G	2
68.2%	90 km/h zone	0		0.0%					Н	0
31.8%	100 km/h zone	0		0.0%	Street Lighting Of	f/Nil	% c	of Dark	ı	0

Speed Limit			~ 40km/h or	less	0	0.0%	20:00 - 21:59	1	4.5%	8.3%
40 km/h or less	0	0.0%	80 km/h zone	0		0.0%	22:00 - 24:00	1	4.5%	8.3%
50 km/h zone	15	68.2%	90 km/h zone	0		0.0%				
60 km/h zone	7	31.8%	100 km/h zone	0		0.0%	Street Lighting	Off/Nil	% of	Dark
70 km/h zone	0	0.0%	110 km/h zone	0		0.0%	0 of	6 in E	Dark	0.0%
			5							

22		CAS	SUALTI	ES	16
.0%		Killed		0	0.0%
.1%		Injured		16	100.0%
.9%		^ Unrestrain	ned	0	0.0%
itted to	ן כ	position OR No	helmet w	orn	
Day		Crashes		Ca	sualties
2.5%		8	2011		2
3.3%		2	2010		2
1.2%		5	2009		6
1.2%		3	2008		3
1.2%		4	2007		3
1.2%					

McLea	n Perio	ds	% Week
Α	2	9.1%	17.9%
В	1	4.5%	7.1%
С	7	31.8%	17.9%
D	3	13.6%	3.5%
E	0	0.0%	3.6%
F	4	18.2%	10.7%
G	2	9.1%	7.1%
Н	0	0.0%	7.1%
I	0	0.0%	12.5%
J	3	13.6%	10.7%

~ School Travel Time

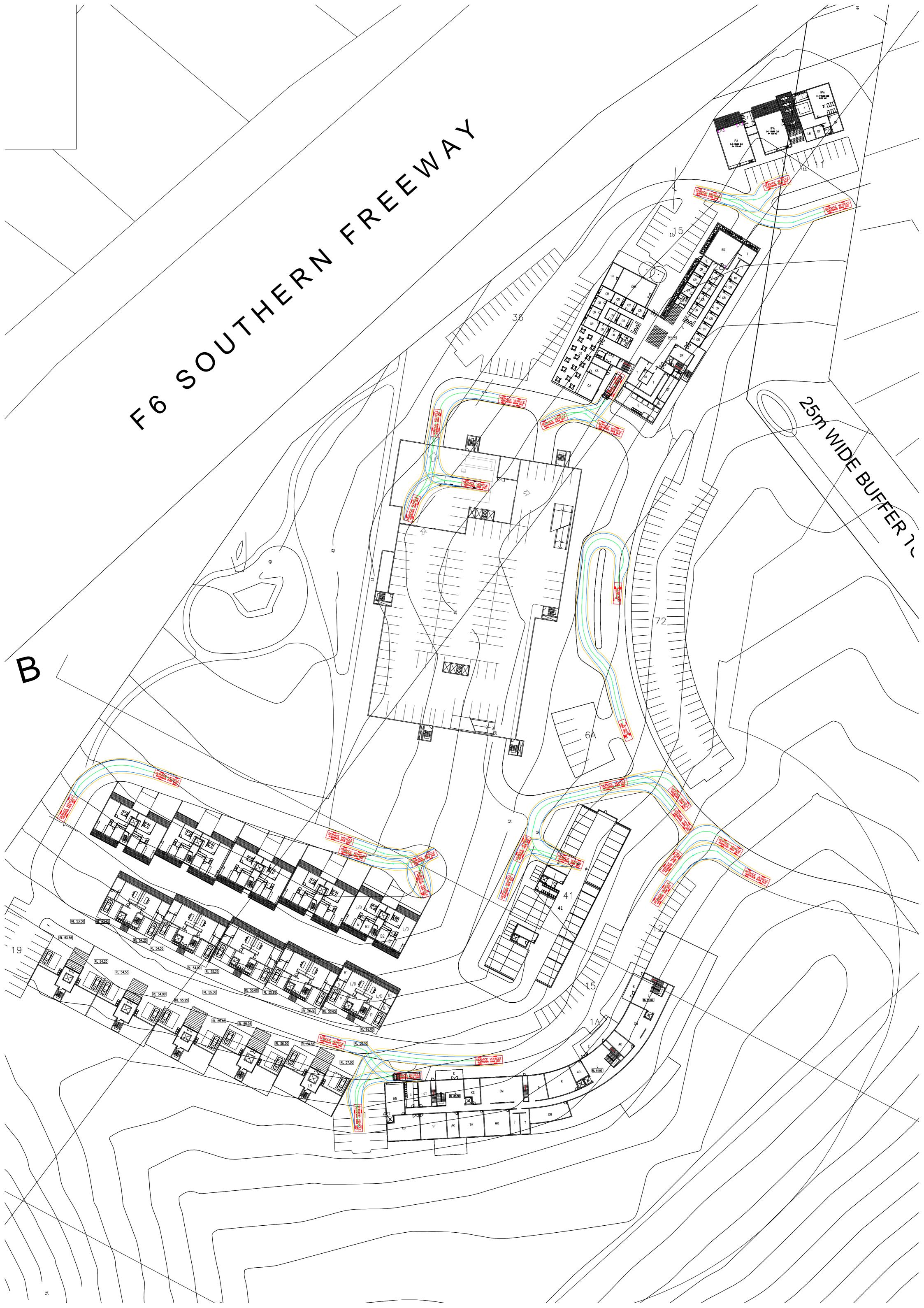
3 13.6%

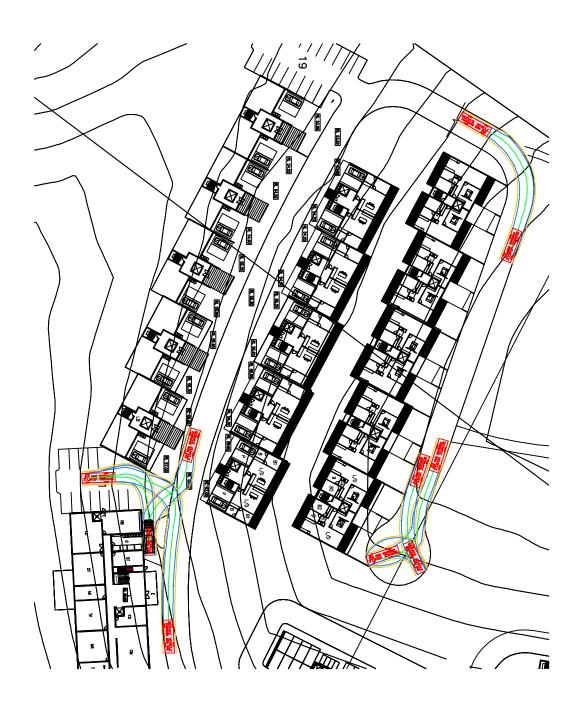
Day of the V	Neek						# Holida	y Periods	New Year	0	0.0%	Queen's BD	0	0.0%	Easter SH	2	9.1%
Monday	4	18.2%	Thursday	2	9.1%	Sunday	0	0.0%	Aust. Day	0	0.0%	Labour Day	0	0.0%	June/July SH	1	4.5%
Tuesday	3	13.6%	Friday	7	31.8%	WEEKDAY	16	72.7%	Easter	0	0.0%	Christmas	0	0.0%	Sept./Oct. SH	1	4.5%
Wednesday	0	0.0%	Saturday	6	27.3%	WEEKEND	6	27.3%	Anzac Day	1	4.5%	January SH	1	4.5%	December SH	0	0.0%

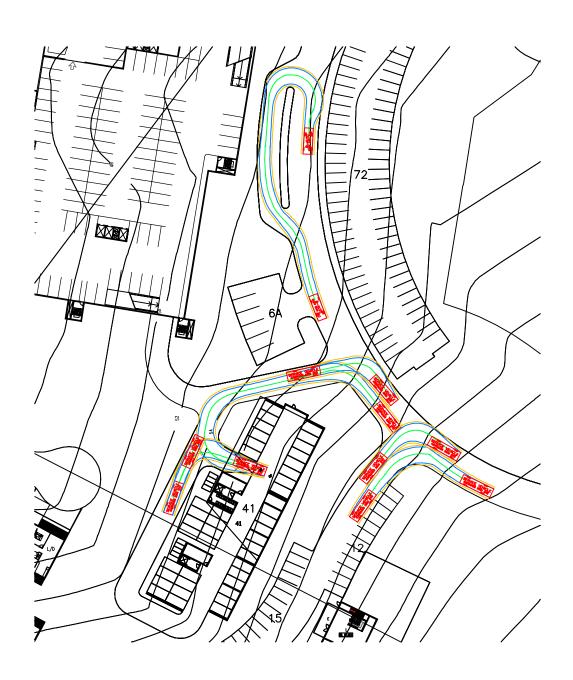
Crashid dataset Nolan St - Berkeley - Jan 2007 to Dec 2011

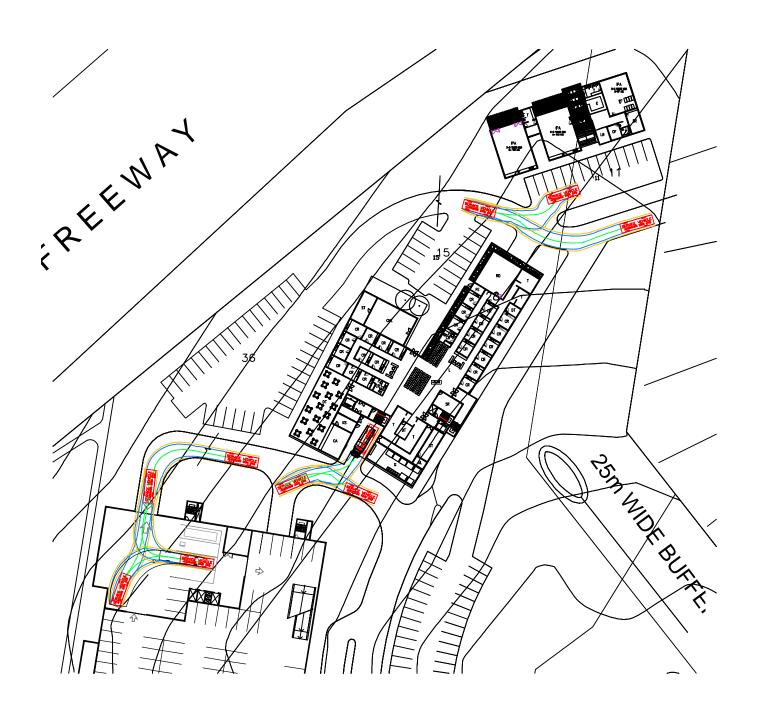
Percentages are percentages of all crashes. Unknown values for each category are not shown on this report.

# Appendix E – Auto Turn Plans

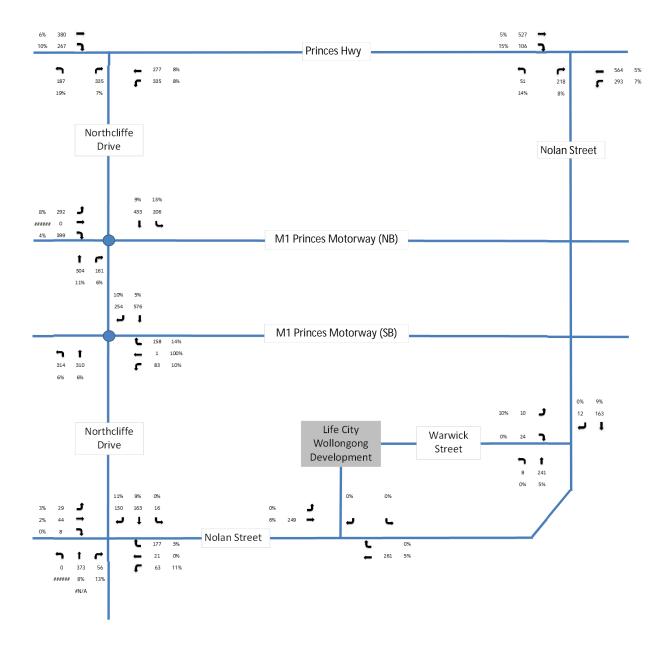


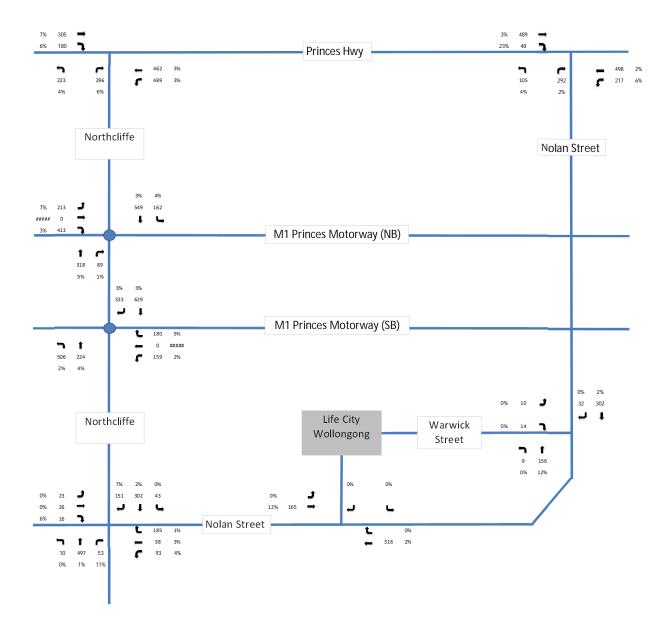


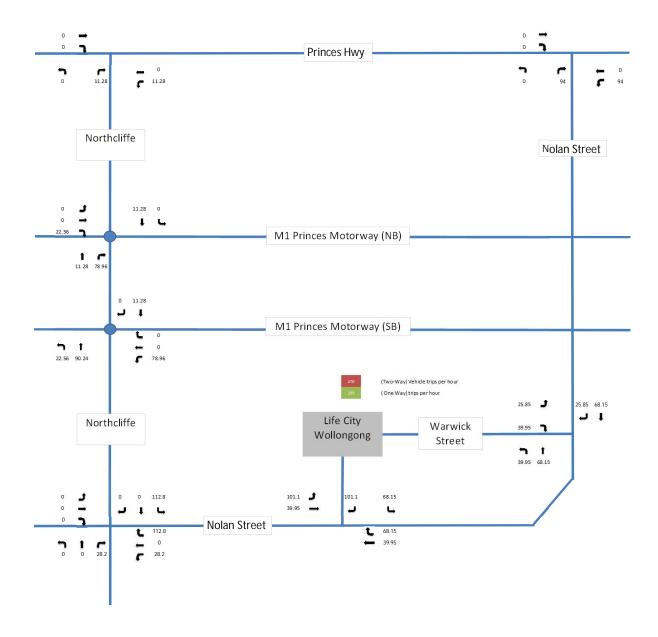


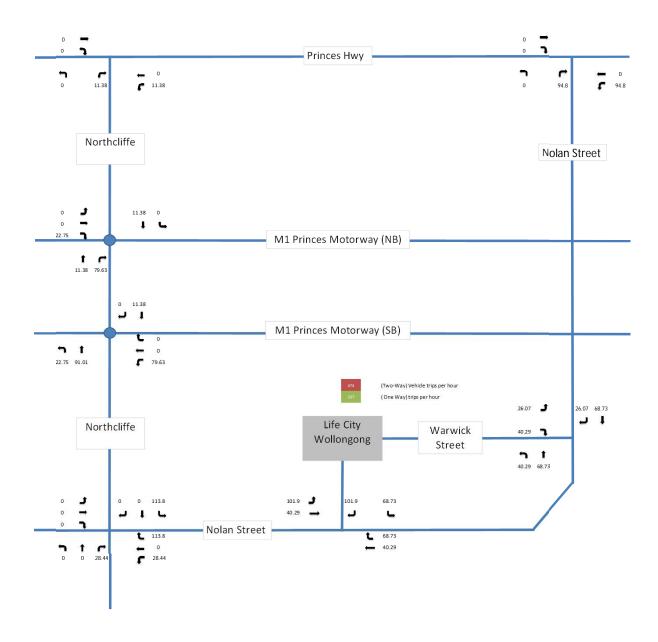


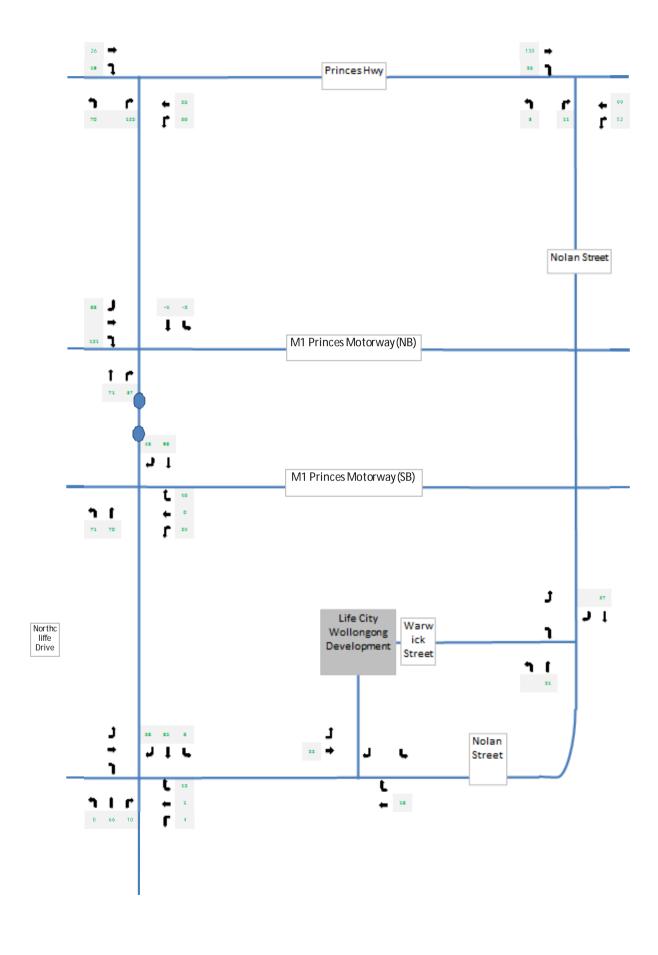
## Appendix F – Detailed Traffic Distribution

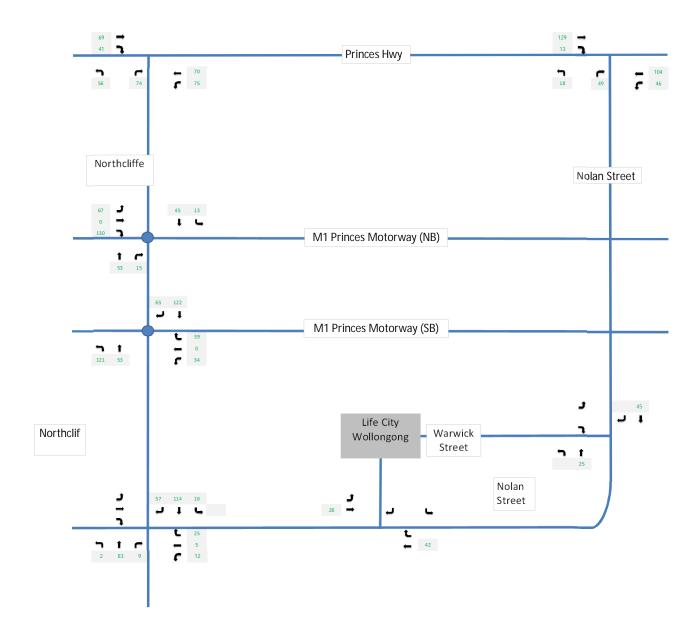


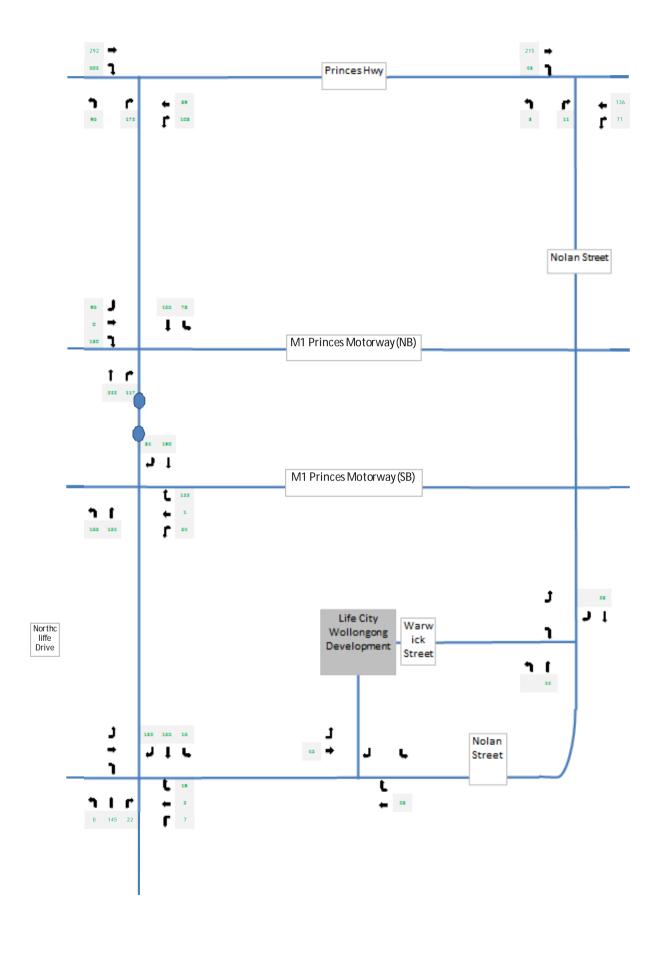


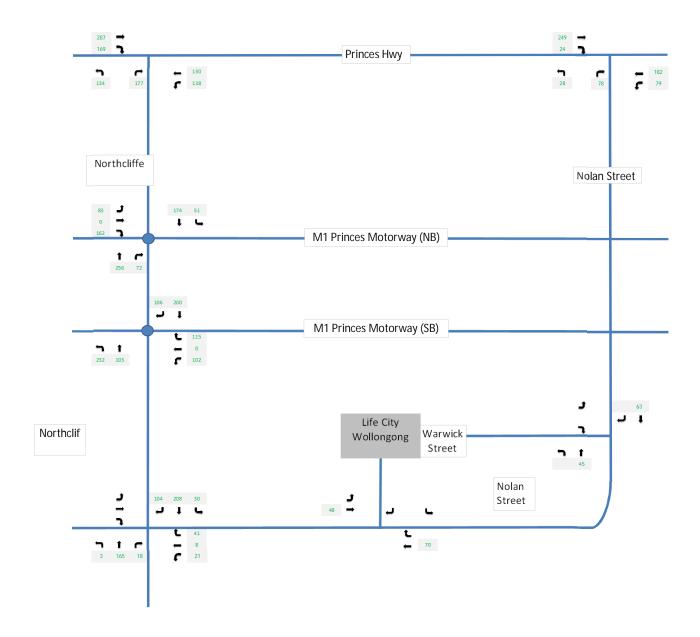


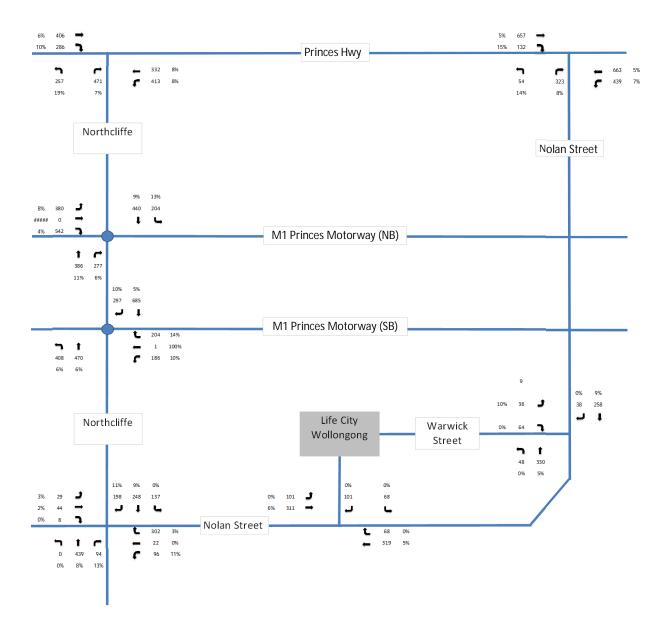


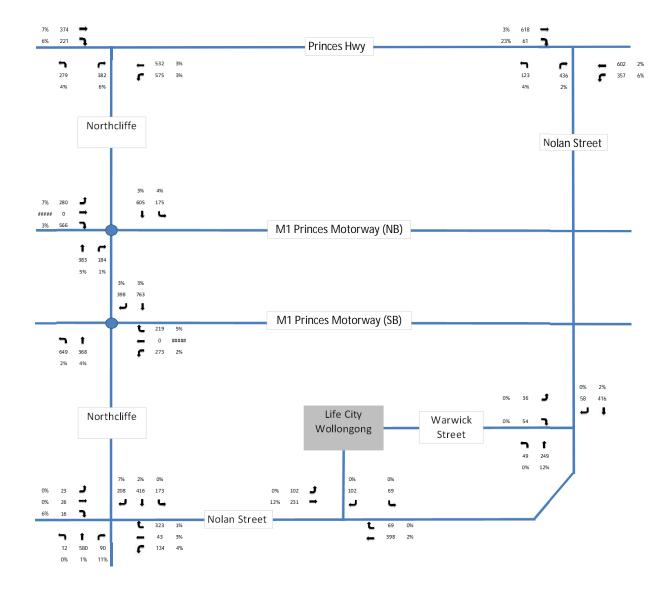


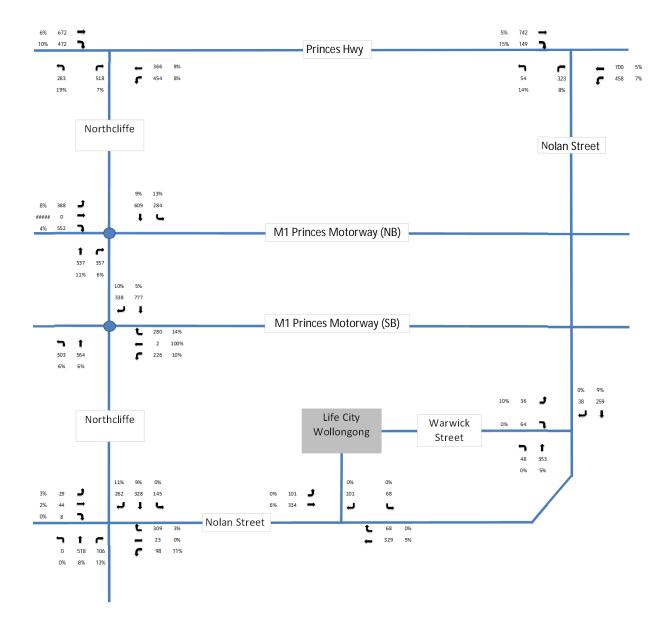


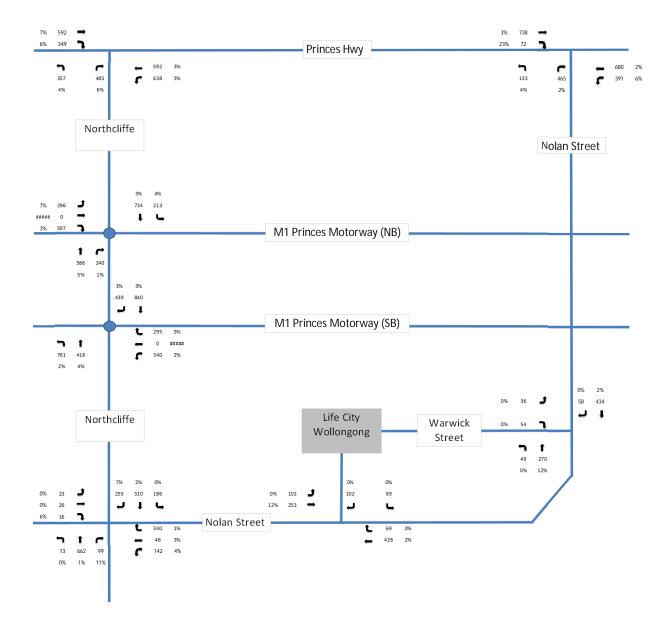














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#### **Document Status**

Rev No.	Author	Reviewer		Approved for Issue		
		Name	Signature	Name	Signature	Date
2	K McNatty	I Smith	85	S Konstas	Ular	23/11/12
3	J.Ticinovic	S.Konstas	Uhr	S Konstas	Ulse	06/02/13
4	K McNatty	I Smith	85	S Konstas	Ular	23/05/13
5	K McNatty	I Smith	85	I Smith	25	22/10/13

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