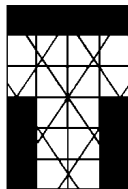


TRAFFIX



**OAKDALE CENTRAL  
PROJECT APPLICATION NO.1  
TRAFFIC IMPACT ASSESSMENT**  
*OF THE PROPOSED*  
**ESTATE ROADS + END-USER FACILITY (DHL)**  
*AT*  
**OLD WALLGROVE ROAD, EASTERN CREEK**

*Prepared on behalf of*

GOODMAN INTERNATIONAL LIMITED

*Prepared by*

**TRAFFIX**

TRAFFIC AND TRANSPORT PLANNERS

*Ref: 07098 PA v6  
May 2008*



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APPENDIX A: REDUCED PLANS  
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# 1. INTRODUCTION

**T R A F F I X** has been commissioned by Goodman International to undertake a traffic impact assessment for a Part 3A Project Application (No. 1) for Estate Works and the development of two building sites within Oakdale Central. These sites are referred to as Site 1A and Site 2A and lie on the eastern boundary of the Oakdale Central area. The report should therefore be read in conjunction with the Oakdale Central Concept Plan Traffic Impact Assessment prepared by TRAFFIX, dated February 2008.

Site 1A is for an end-user development (the DHL Facility) which is located within the northern part of the site that is the subject of this Project Application. Site 2A occupies the remaining southern part of the site.

The development proposal relates to a total building area of 54,350m<sup>2</sup> and provides parking for a total of 390 cars. The development is therefore required to be referred to the RTA under the provisions of SEPP (Infrastructure) 2007.

The report is structured as follows:

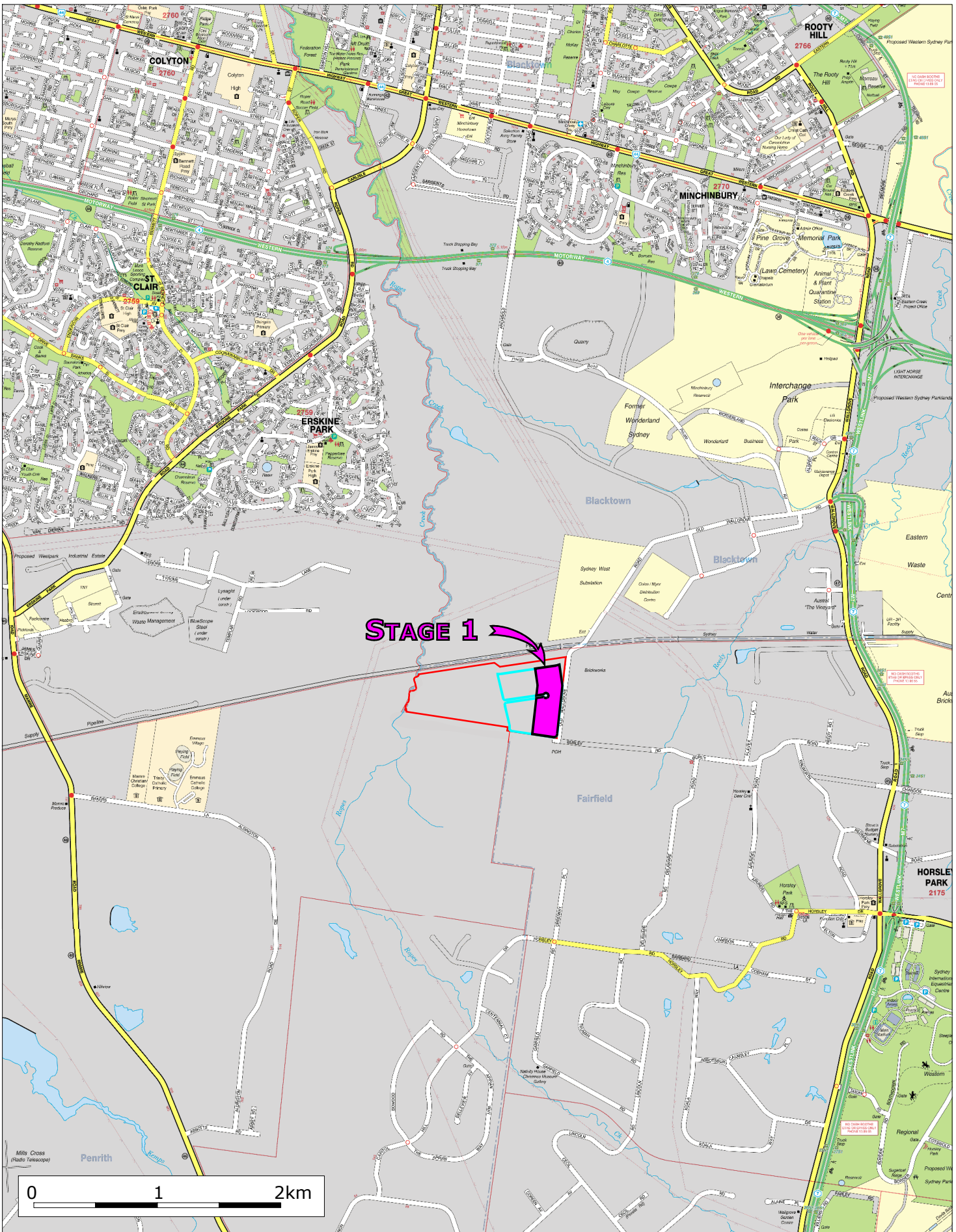
- Section 2: Describes the site and its location
- Section 3: Documents existing traffic conditions
- Section 4: Describes the proposed development
- Section 5: Assesses the traffic impacts
- Section 6: Assesses parking requirements
- Section 7: Discusses access and internal design aspects
- Section 8: Presents the overall study conclusions.

## 2. LOCATION AND SITE

The site is located within the eastern part of the Oakdale Central Concept Plan area, to the south-west of the existing alignment (and termination) of Old Wallgrove Road. It is therefore to the immediate south west of the M7 Business Hub. The site is within Area 8 of the 656ha site identified as the “Area South of the Sydney Water Pipeline” which forms one of the 10 sites that comprise the Western Sydney Employment Hub. It is the last of these development areas to proceed.

The site that is the subject of this Project Application occupies a total site area of 12.58ha with a total leased area of 109,900m<sup>2</sup> (about 11ha) and has frontage to a proposed new road that will form the extension of Old Wallgrove Road and will ultimately be widened to connect to Mamre Road in the west, as discussed in the Oakdale Central Concept Plan Application. Hence, all entry and exit movements will occur via Old Wallgrove Road, connecting to Wallgrove Road and the M7 Sydney Orbital. The minor subdivisional roads that traverse the eastern and southern boundaries of the site will also be constructed including temporary roads serving Sites 1A and 2A.

The overall site incorporating Sites 1A and 2A has a generally rectangular configuration and a Location Plan is presented in **Figure 1**, with a Site Plan presented in **Figure 2**.



Source: UBD 2006



**STAGE 1 PROJECT APPLICATION**  
**PROPOSED END-USER FACILITY (DHL)**  
**OLD WALLGROVE ROAD, EASTERN CREEK**  
 Prepared on behalf of Goodman International Limited

Figure 1

**LOCATION**

TRAFFIX Traffic & Transport Planners: Level 2, 55 Mountain Street, Broadway, 2007.



0 100 200m

design

Oakdale  
Eastern Creek

Oakdale Central  
Lot 2 DP120679

Masterplan

22 Oct 07 JC

SP DHL SK44 (A)



STAGE 1 PROJECT APPLICATION  
**PROPOSED END-USER FACILITY (DHL)**  
**OLD WALLGROVE ROAD, EASTERN CREEK**  
Prepared on behalf of Goodman International Limited

Figure 2

**SITE**

TRAFFIX Traffic & Transport Planners: Level 2, 55 Mountain Street, Broadway, 2007.

### 3. DESCRIPTION OF PROPOSED DEVELOPMENT

A detailed description of the proposed development is provided in the Project Application Report prepared on behalf of Goodman International. This is based on the plans prepared by Goodman International dated 4<sup>th</sup> April 2008 which are reproduced at reduced scale in **Appendix A**. In summary, the development for which approval is now sought has a total building area of 53,350m<sup>2</sup>, as follows: These will all operate 24 hours per day, 7 days per week.

#### *Site 1A (The DHL Facility – Warehouse 1)*

- 20,000m<sup>2</sup> of warehouse area;
- 820m<sup>2</sup> of ancilliary office area; and
- 150 parking spaces;

#### *Site 2A (Warehouse 2)*

- 32,550m<sup>2</sup> of warehouse area;
- 980m<sup>2</sup> of office area; and
- 240 parking spaces;

A total of 390 parking spaces are proposed. All access to the overall site is via a staged perimeter roads as shown in **Appendix A**. This includes the construction of a temporary access road (“Temporary Access Road No. 1”) along the northern site boundary that lies within the future link road corridor. This will be constructed to a standard that will enable the future development of the balance of the Oakdale Central Project area. This road also traverses part of the western site boundary and is to terminate at a cul-de-sac within “Estate Road Lot 3” as shown on Drawing SK69 (A). The cul-de-sac lies between Sites 1A and 1B which is on the alignment of “Estate Road Lot 2”, which will be partially constructed. This temporary access road will be closed and all access provided by “Estate Road Lot 2” in the longer term.

Access is proposed by 25m long B Doubles to both warehouse buildings.

It is emphasised that the total office space for the overall development is 1,800m<sup>2</sup> which represents only 3.3% of the total floor area. This is substantially below the 20% limit for ancilliary office area within an industrial development.

The traffic and parking impacts arising from the development are discussed in Sections 5 and 6.

## 4. ASSESSMENT OF TRAFFIC IMPACTS

### 4.1 Traffic Generation Under RTA Guideline Trip Rates

The proposed development embodies areas as shown in Table 1 below, with trip rates adopted by the Roads and Traffic Authority for the relevant land use components.

**Table 1: Traffic Generation Under RTA Trip Rates (Peak Periods)**

Floor Space Component	Area (m <sup>2</sup> )	RTA Trip Rate	Trips Per Hour
Free Standing Office <sup>1</sup>	Nil	2.0/ 100m <sup>2</sup>	-
Warehouse <sup>2</sup>	54,350 (100%)	0.5/100m <sup>2</sup>	272
Total	158,650 (100%)		272

Note 1: No Free Standing Office Proposed

Note 2: Includes 1,800m<sup>2</sup> ancillary office areas 3% of total area)

It can be seen that a total of 272 vehicle trips per hour would result from application of the Roads and Traffic Authority's 'generic' trip rates; with 80% in the direction of peak flow as follows:

- 272 vehicle trips per hour in the morning peak (218 in, 54 out);
- 272 vehicle trips per hour in the evening peak (54 in, 218 out);

However these are average rates over the metropolitan area and more recent data is available.

### 4.2 Warehouse Distribution Trip Rates

The rate of 15 trips per hectare has been generally adopted by the RTA in other comparable locations (including Eastern Creek and the M7 Business Hub), where there is a similar high proportion of warehouse and distribution facilities. The basis of this rate is not fully appreciated but it arguably does not take full account of public transport improvements as would occur if the 10% target set under SEPP 59 was achieved. That is, it is a worst-case scenario which in the long term (pending implementation of public transport initiatives) is likely to overstate the traffic generation arising from Oakdale in general, and the DHL Facility in particular. Application of this rate to the 10.99ha for this development results in the following trips:

- 164 vehicle trips per hour in the morning peak (131 in, 33 out);
- 164 vehicle trips per hour in the evening peak (33 in, 131 out);

These trips are considered appropriate for adoption and are preferred to the RTA's generic rate. In addition, many of the development applications within Erskine Park and the M7 Business Hub have demonstrated trip rates that are lower than 15 trips/ha and this is a direct consequence of the following factors:

- Large warehouse developments typically operate 24 hours per day and 7 days per week, thereby spreading traffic loads and minimising peak period generation;
- Warehouse staff are usually rostered over this 24 hour shift with changeover times that do not generally coincide with the on-street peak period; and
- Peak period travel is usually associated with administrative staff, which is a small proportion of the overall workforce.

Accordingly, the adoption of 164 veh/hr is considered to represent a worst-case scenario, which is nevertheless appropriate for assessment. With a total of 390 parking spaces, this represents about 0.43 trips/space/hr which is also within the expected range.

### 4.3 Impacts of Generated Traffic

The traffic generated by Project Application 1 (164 veh/hr) represents 27% of the overall traffic generation associated with the Oakdale Central Concept Plan. These are moderate volumes and the main issue of potential concern relates to the right turn movement from Wallgrove Road into Old Wallgrove Road (north to west) which is expected to increase from 118 veh/hr presently to 188 veh/hr (i.e. an additional 70 veh/hr, which represents 54% of total arrivals).

The existing route and intersection performance was assessed in the Concept Plan Application report and was found to be satisfactory subject to minor improvements. The impact of the Project Application No. 1 has been assessed based on the existing geometry and the results are provided in **Appendix C**. It is evident that the intersection will continue to perform satisfactorily with no improvements required at this intersection. It does not therefore depend upon any intersection improvements external to those works that are planned under the Project Application.

The road system serving the Project Application No.1 site comprises a 2 lane two-way temporary roadway that connects to Wallgrove Road to the south of the existing bridge over the Sydney Water Supply Pipeline, where it forms the stem of a priority-controlled 'T' junction. The proposed 7.0 metre wide carriageway with one lane in each direction will readily accommodate the traffic generated under the Project Application (164 veh/hr) and the design is more dictated by geometric requirements.

The temporary intersection with Old Wallgrove Road will operate very satisfactorily, with minimal delays arising from the predicted traffic volumes, which equate to a maximum arrival

in the AM Peak of about 2-3 vehicles per minute; with a maximum departure in the PM peak also of about 2-3 vehicles per minute.

The improved link road will traverse the bridge over the Sydney Water Pipeline and this will need to be designed to accommodate a 7m wide road carriageway, with the ability to widen this to 4 lanes at a future time to facilitate the completion of all development within Oakdale, as discussed in the Concept Plan Application submitted separately. The horizontal and vertical geometry at this bridge will however need to be assessed and improvements only undertaken if they are required to achieve compliance with relevant standards and guidelines, including the RTA's Road Design Guide.

These volumes are in addition to the traffic volumes presently using Old Wallgrove Road which principally relate to the existing quarrying activities associated with PGH Bricks (south of Burley Road) and Austral Bricks (opposite the site on Old Wallgrove Road). These sites together generate moderate volumes which are estimated to be about 30 veh/hr during peak periods. A Section 96 (1A) modification has also been lodged for the Bedford Quarry (DA-260-8-2002) which is estimated to generate an additional 15 veh/hr associated with the western pit, as discussed in the SEE. This pit lies to the immediate west of the subject site and will be accessed at the southern end of the site. These volumes are moderate and raise no concern for the operation of the temporary access road, assuming priority (stop) sign control. The access incorporates passing bays for southbound through traffic along Old Wallgrove Road, so that this traffic is not delayed by right turn entering vehicles.

The through movement along Old Wallgrove Road north of the site will be about 200 veh/hr during peak periods (combined flow in both directions) which can be readily accommodated by Old Wallgrove Road, which has a mid-block capacity of about 1,200 veh/hr for a single undivided lane under interrupted flow conditions. Therefore a 7.0 metre wide undivided carriageway with one lane in each direction will readily accommodate the traffic generated under the Project Application.

This may require upgrading of sections of Old Wallgrove Road, subject to further detailed assessment. As mentioned, the bridge over the Sydney Water Pipeline will also need to provide a minimum 7m wide road carriageway and this appears to be achievable without widening the existing structure (subject to further detailed assessment). Local widening at bends may also be required to safely accommodate B Doubles and this will need to be assessed at the detailed design stage.

In summary the traffic generated by this Project Application No. 1 can be readily accommodated with no external traffic improvements required, including the proposed works associated with the M7 Hub which are not relied upon.

## 5. PARKING REQUIREMENTS

The requirements of Fairfield Council's City Wide DCP (Chapter 12) have been reviewed as well as the rates embodied in the RTA's document entitled "Guide to Traffic Generating Developments." The application of Council and RTA parking rates to the proposed development under the Project Application results in parking requirements as shown in Table 2.

**Table 2: Comparative Parking Requirements**

Site	Area (m <sup>2</sup> )	Council DCP Requirement		RTA Guideline Requirement	
		Rate	Spaces	Rate	Spaces
	Warehouse	1/80m <sup>2</sup>		1/300m <sup>2</sup>	
	Office	1/70m <sup>2</sup>		1/300m <sup>2</sup>	
1A	20,000 820		250 12		70
			<b>262</b>		
2A	32,550 980		407 14		112
			421		
Total Warehouse	52,550		657		
Total Office	1,800		26		182
TOTAL	54,350		683		182

It can be seen that the development would require between 182 spaces based on the RTA's requirements and 683 spaces if based on Council's DCP requirement. It is noted that the above parking rates are 'generic' rates that are averaged across the LGA/metropolitan area and do not take due account of the particular requirements of specific tenants or the wide variation in surveyed parking demands for industrial uses. Accordingly, a significant degree of flexibility is required.

In this regard, the plans incorporate 390 spaces (150 for Site 1A and 240 for Site 2A) which are substantially more than the RTA's requirement which demonstrates that all parking demands will be fully accommodated within the site. Application of Council's rate is considered excessive and is not considered representative of a modern distribution facility.

## 6. ACCESS & INTERNAL DESIGN ASPECTS

### 6.1 Access Design

The access arrangements comply with all relevant standards. The following factors are noted:

- The northern access roadway is located to the south of the existing bridge crossing of Old Wallgrove Road, at the north-eastern corner of the site. This is located on the outside of (and south of) the existing bend on approach to the bridge crossing. In this regard, all traffic will enter the driveway via a right turn and will exit via a left turn;
- Sight distances at this driveway to/from the south are excellent and substantially exceed the requirements of AS 2890.2;
- Sight distances at this driveway to/from the north are less. A truck exiting the site has an available sight distance of 90 metres to the north. This compares with a requirement for 83 metres assuming an approach speed of 60km/h, with a 5 second gap. This is therefore acceptable;
- The available sight distance for a driver travelling southbound on approach to the driveway is 70 metres. This compares with a Stopping Sight Distance of 65 metres for an approach speed of 60km/h, which is also acceptable;
- It is noted that the above assumes that a B Double turning left out of the site does not encroach onto the opposite carriageway, which would reduce the available sight distance to a relocated 'conflict point'. This requires the provision of generous splays as provided for;
- It is recommended that a 60km/h speed signposting be installed facing southbound traffic in the vicinity of the existing bridge, to achieve satisfactory sight distances; and
- It is proposed that a passing bay be provided for southbound traffic to enable passing of vehicles waiting to turn right into the site. This incorporates a 6 metre wide southbound carriageway with appropriate diverge and merge tapers based on the RTA's Road Design Guide. This involves 25 metre long tapers (both sides) with a 40 metre long passing lane, based on site entry by a 25 metre B Double. This local widening can occur within the existing road reserve.

Finally, it is noted that the above access driveway is an interim arrangement, pending the construction of the new subdivisional road system to the west of the site under subsequent development stages. The ultimate arrangement will involve a substantial upgrade of Old Wallgrove Road on the general alignment of the temporary access road.

## 6.2 Internal Design

The internal design complies with the requirements of AS 2890.1 and AS 2890.2 and incorporates the following elements:

- Both sites incorporate a one-way clockwise flow-through system which is safe and efficient;
- Access is facilitated by the cul-de-sac
- Provision of sufficient clearances to accommodate a B Double operating with a 12.5 metre radius turn, as defined by Austroad Guidelines and as shown in **Appendix B**. The detailed design of these areas will require further assessment at construction certificate stage, taking account also of Fairfield Council's requirements for driveway crossings;
- Extensive internal queuing capacity is provided for both sites;
- Cars and trucks are provided with separate access driveways and are separated internally, providing maximum safety for both car drivers and pedestrians;
- Available sight distances at all driveways will be satisfactory, subject to the road verge being landscaped with appropriate species; and
- The Parking bays and aisles comply with the requirements of AS 2890.1 and generally incorporate bays of minimum width 2.5 metres with aisles of minimum width 6.0 metres;

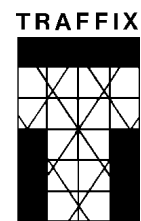
In summary, the internal design is considered to be satisfactory and will provide a very good level of safety, convenience and amenity. It will however be necessary to review truck movements prior to construction to ensure that the detailed designs for vertical and horizontal geometry comply with relevant standards.

## 7. CONCLUSIONS

The following matters are noteworthy:

- *The traffic generation arising from Project Application No. 1 (164 veh/hr combined in both directions at peak times) can be accommodated on the road network with a 7.0 metre wide road carriageway, providing single lane traffic flow in each direction. This will require improvements to some existing sections of Old Wallgrove Road, subject to a detailed road conditions audit;*
- *The temporary access roads to be relied upon are readily able to accommodate the required traffic volumes;*
- *Parking is provided for 390 spaces which is substantially more than required under the RTA's Guideline (182 spaces) and less than Fairfield Council's requirements (683 spaces). This is considered will ensure that on-street parking does not occur;*
- *The proposed means of site access is considered satisfactory and traffic will be able to enter and exit the site safely and efficiently in a forward direction, including B-Doubles. In addition, cars and heavy vehicles are physically separated to a significant extent; and*
- *The internal design arrangements comply with the requirements of AS 2890.1 and AS 2890.2 and will accommodate all required vehicles. The detailed design of the facility, including driveway crossing levels and on-street parking controls, will need to be reviewed prior to construction; and*
- *Provision will be made for visitors as well as disabled parkers;*

It is concluded that the proposed development is supportable on traffic planning grounds and will operate satisfactorily.

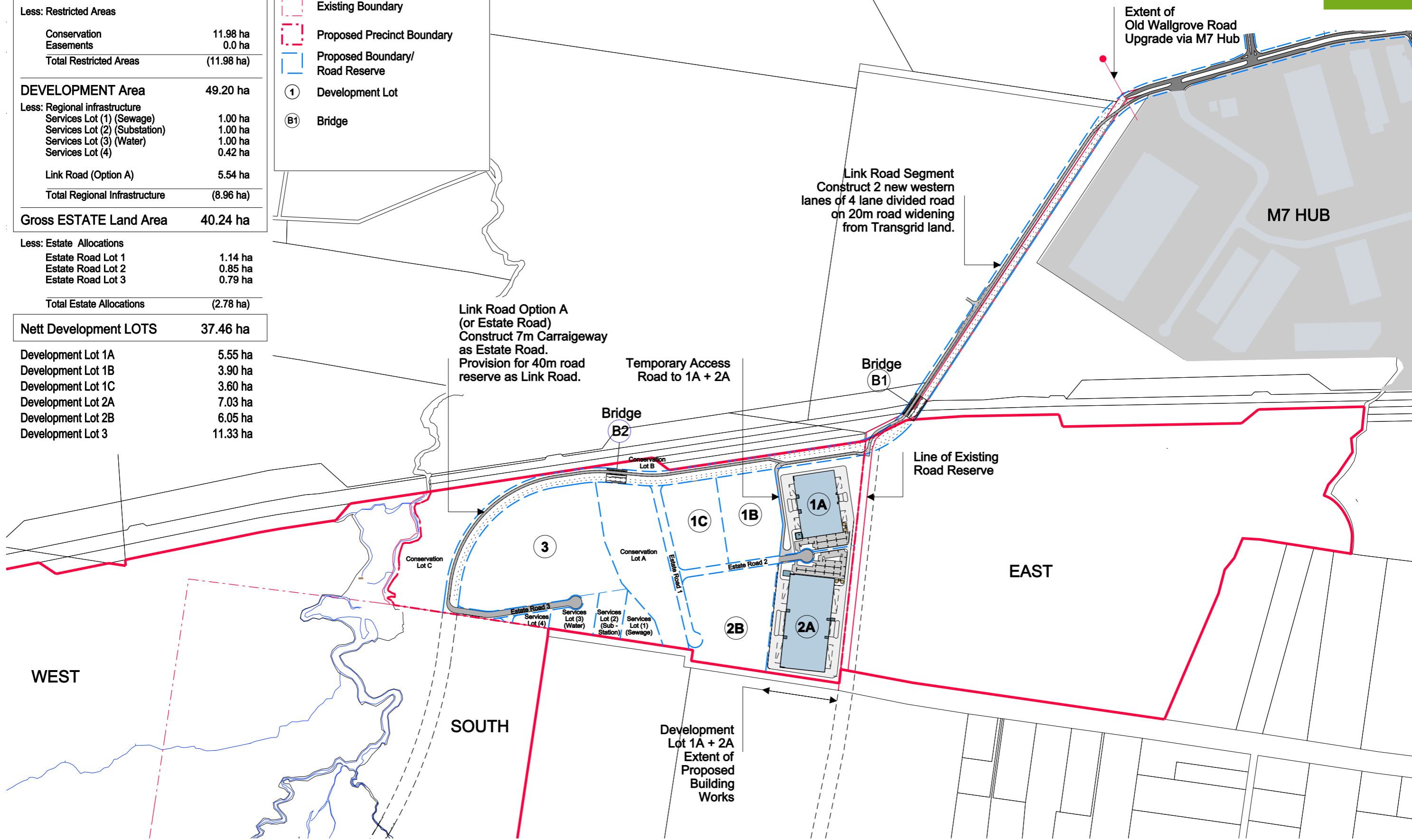


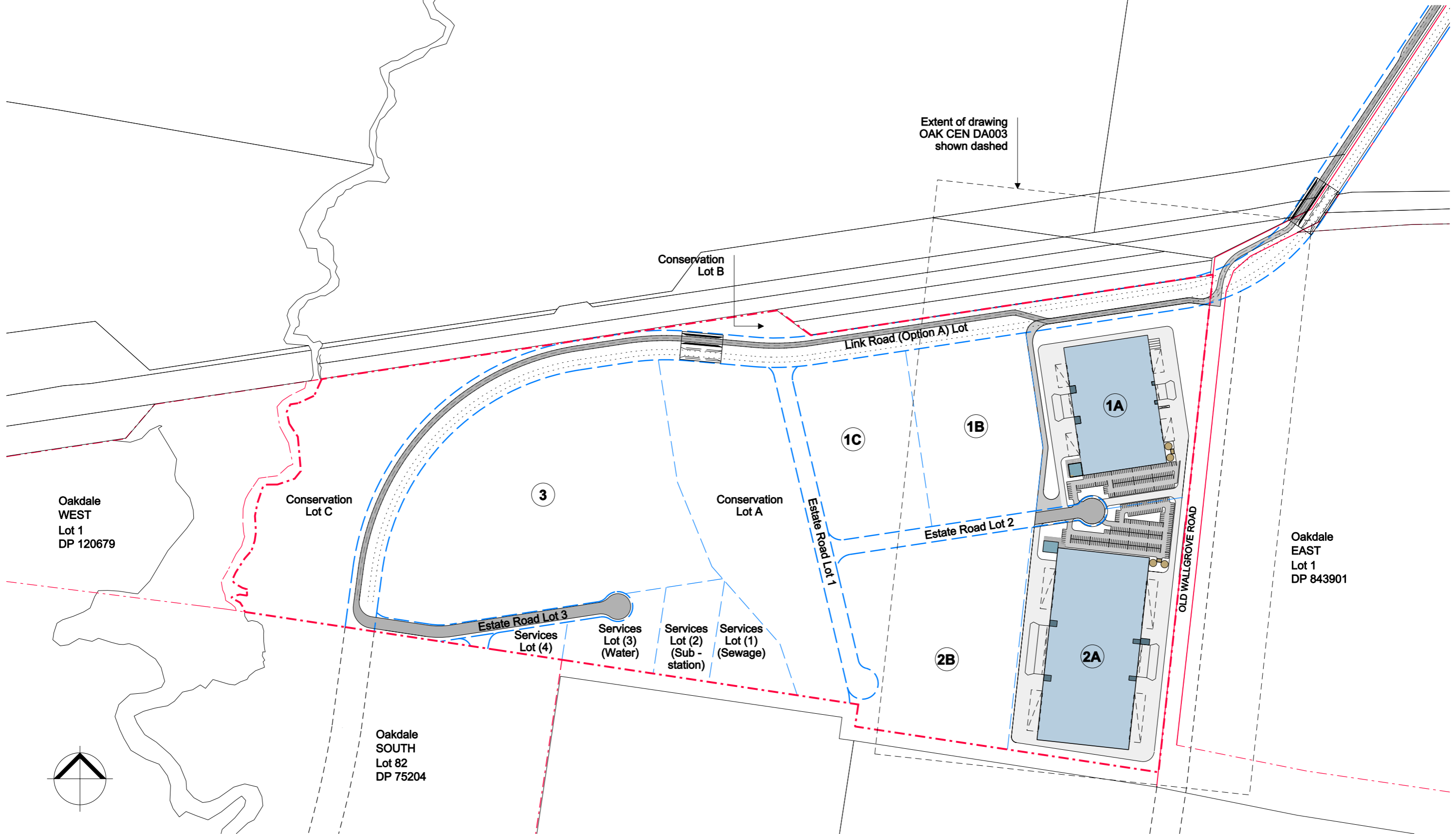
## ***APPENDIX A:***

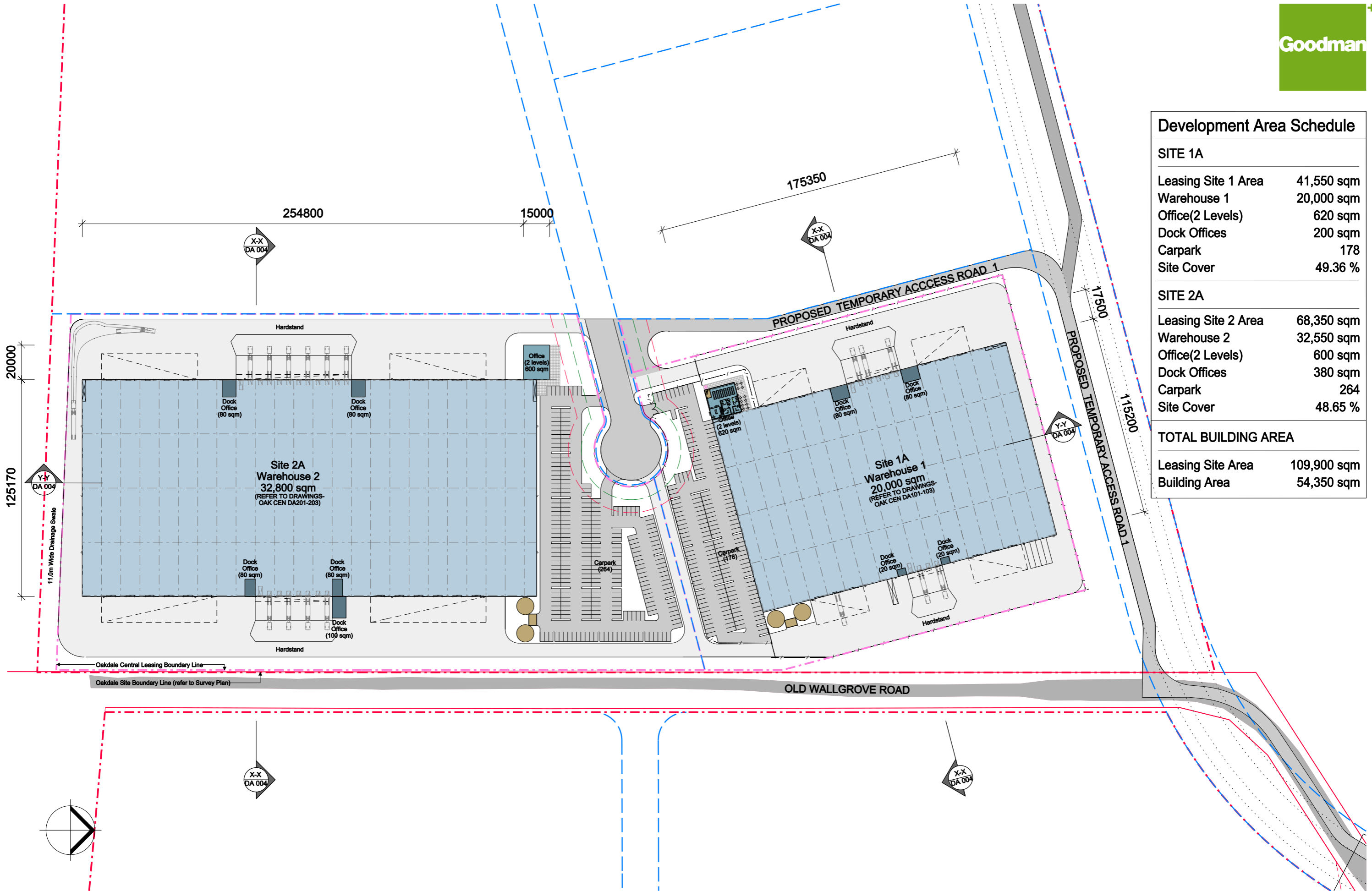
*Reduced Plans*

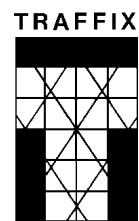
Site Area Schedule	
	Central
Total Land Area	61.18 ha
Less: Restricted Areas	
Conservation Easements	11.98 ha
	0.0 ha
Total Restricted Areas	(11.98 ha)
DEVELOPMENT Area	49.20 ha
Less: Regional Infrastructure	
Services Lot (1) (Sewage)	1.00 ha
Services Lot (2) (Substation)	1.00 ha
Services Lot (3) (Water)	1.00 ha
Services Lot (4)	0.42 ha
Link Road (Option A)	5.54 ha
Total Regional Infrastructure	(8.96 ha)
Gross ESTATE Land Area	40.24 ha
Less: Estate Allocations	
Estate Road Lot 1	1.14 ha
Estate Road Lot 2	0.85 ha
Estate Road Lot 3	0.79 ha
Total Estate Allocations	(2.78 ha)
Nett Development LOTS	37.46 ha
Development Lot 1A	5.55 ha
Development Lot 1B	3.90 ha
Development Lot 1C	3.60 ha
Development Lot 2A	7.03 ha
Development Lot 2B	6.05 ha
Development Lot 3	11.33 ha

Legend	
<span style="border: 2px solid red; display: inline-block; width: 15px; height: 10px;"></span>	Oakdale Concept Plan (CP #2)
<span style="border: 2px dashed red; display: inline-block; width: 15px; height: 10px;"></span>	Existing Boundary
<span style="border: 2px dashed red; display: inline-block; width: 15px; height: 10px;"></span>	Proposed Precinct Boundary
<span style="border: 2px solid blue; display: inline-block; width: 15px; height: 10px;"></span>	Proposed Boundary/ Road Reserve
①	Development Lot
Ⓑ1	Bridge









## ***APPENDIX B:***

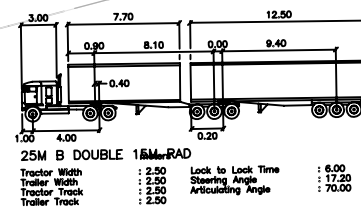
### *B Double Swept Path Analysis*



Dock Office  
(20 sqm)

stand

ROAD 1



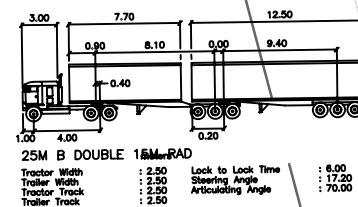
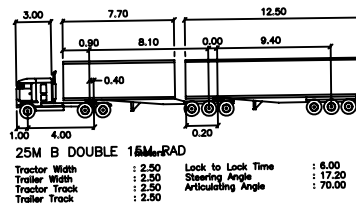
25m B Double Access  
1 in 500 at A3  
TRAFFIX 18 March 2008

TEMPORARY ACCESS ROAD 1

Hardstand

Dock Office  
(80 sqm)

Dock Office  
(80 sqm)

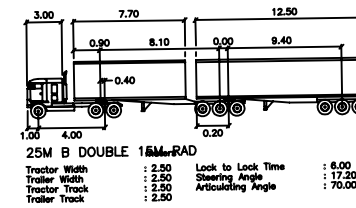
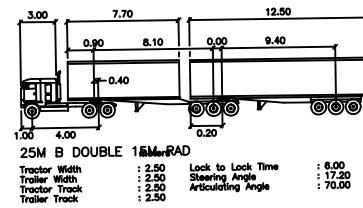


PROPOSED TEMPORARY ACCESS ROAD 2

17500

11520

25m B Double Access  
1 in 500 at A3  
TRAFFIX 18 March 2008

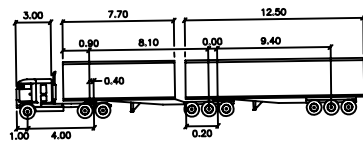


PROPOSED

Office  
(2 levels)  
600 sqm

Office  
(2 levels)  
620 sqm

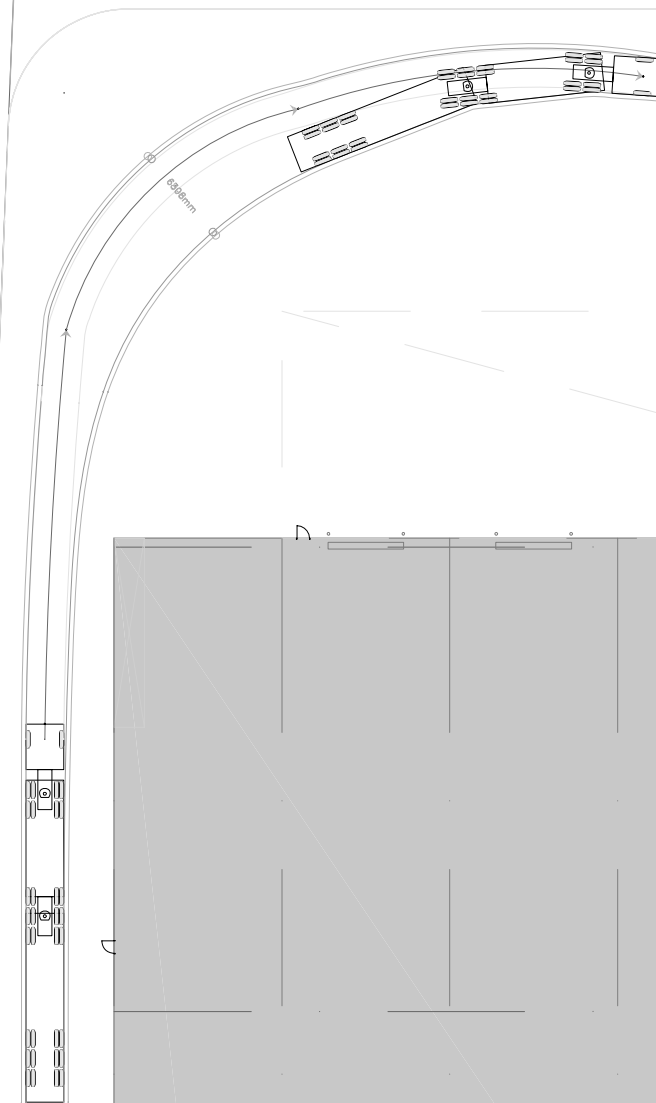
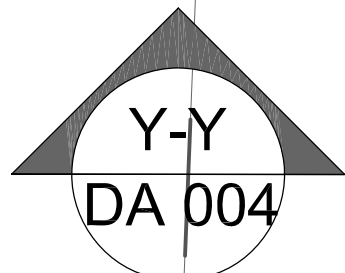
25m B Double Access  
1 in 500 at A3  
TRAFFIX 18 March 2008



25M B DOUBLE 16M RAD  
Tractor Width : 2.50 Lock to Lock Time : 6.00  
Trailer Width : 2.50 Steering Angle : 17.20  
Tractor Track : 2.50 Articulating Angle : 70.00  
Trailer Track : 2.50

20000

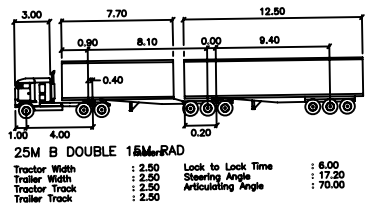
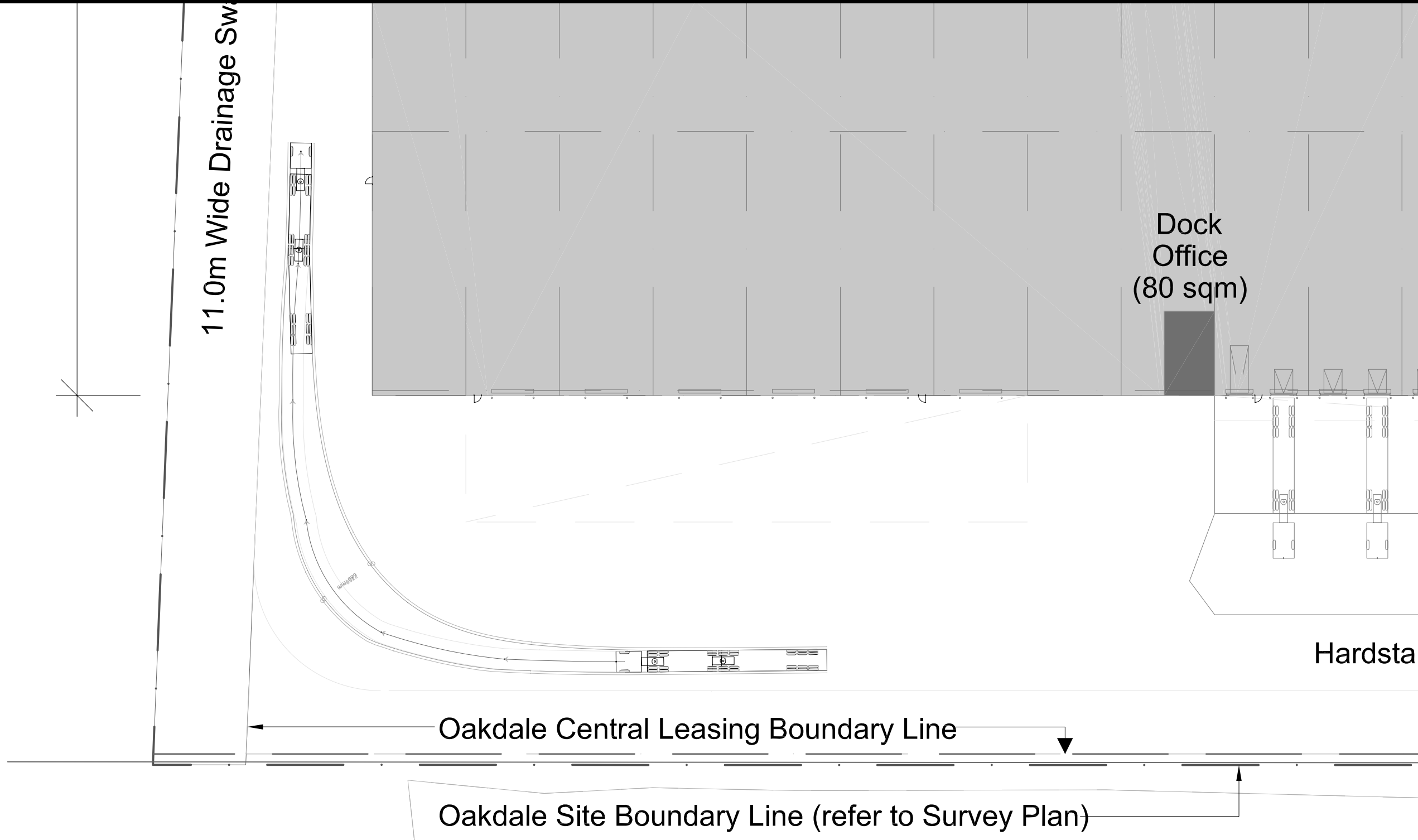
5170



Dock Office  
(80 sqm)

25m B Double Access  
1 in 500 at A3  
TRAFFIX 18 March 2008

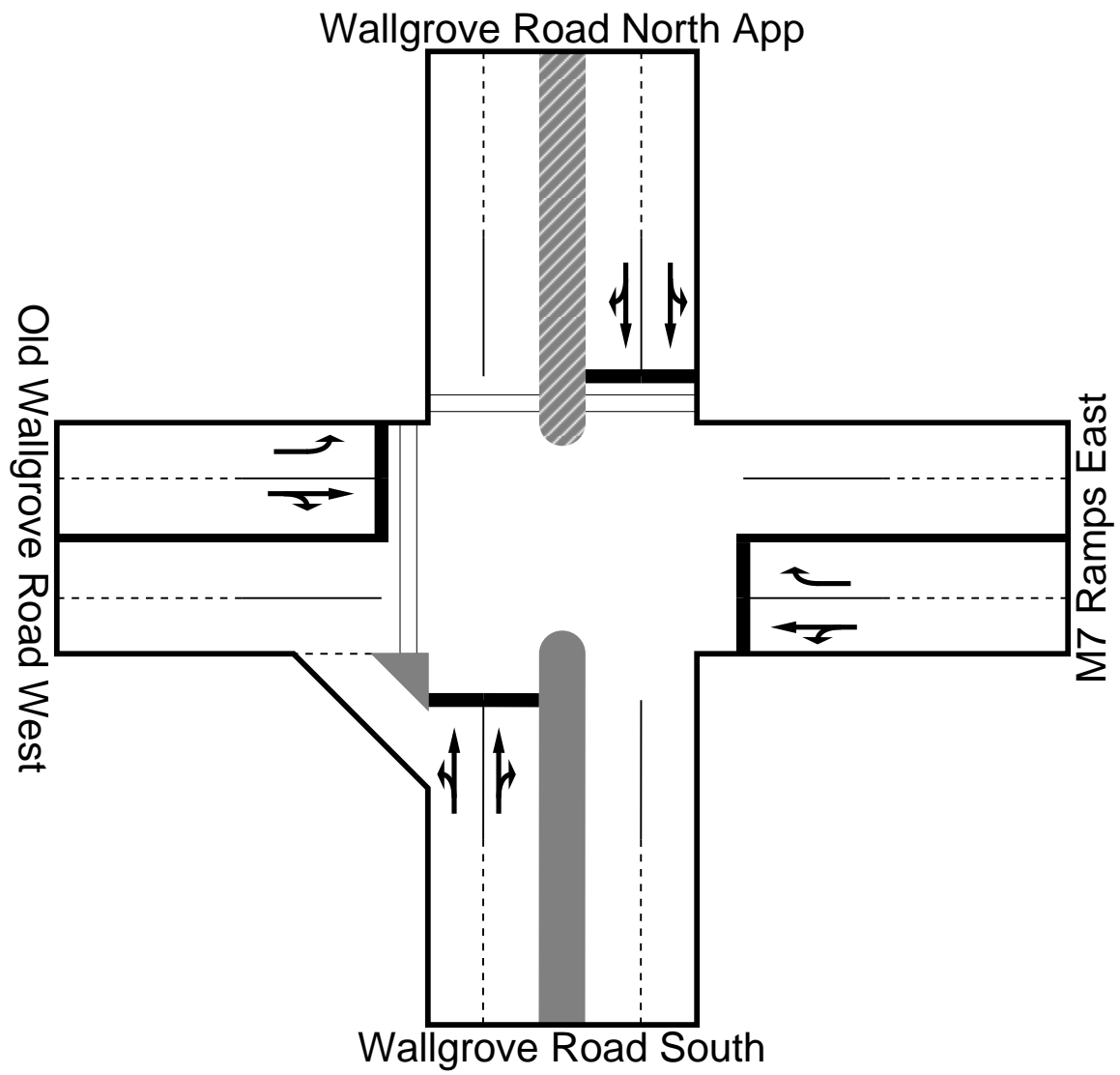
Sit  
Ware  
32,5



25m B Double Access  
1 in 500 at A3  
TRAFFIX 18 March 2008

## ***APPENDIX C:***

*SIDRA Modelling Results*



**EXISTING GEOMETRY**



# Output Tables

## Wallgoove/Old Wallgrove Existing

### PA1 Future AM

#### Run Information

Cycle Time = 70 (Practical Cycle Time)

\* Basic Parameters:

- Intersection Type: Signalised - Fixed Time
- Driving on the left-hand side of the road
- Input data specified in Metric units
- Model Defaults: Standard Left
- Peak Flow Period (for performance): 60 minutes
- Unit time (for volumes): 60 minutes.
- Delay definition: Control delay
  - Geometric delay included
- SIDRA Standard Delay model used
- SIDRA Standard Queue model used
- Level of Service based on: Delay (RTA NSW)
- Queue definition: Back of queue, 95th Percentile

\* Iteration Data:

- No. of Main (Timing-Capacity) Iterations = 3
- Comparison of last two iterations:
  - Difference in intersection degree of satn = 0.0 %
  - Largest difference in eff. green times = 0 secs
  - (max. value for stopping = 0 secs)

#### Table B.1 - Movement Definitions and Flow Rates (Origin-Destination)

Wallgoove/Old Wallgrove Existing  
 PA1 Future AM  
 Intersection ID: C0  
 Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

From Approach	To Approach	Mov ID	Turn	Flow Rate		Flow Scale	Peak Flow Factor
				LV	HV		
-----							
South: Wallgrove Road South							
	East	3	Right	5	0	1.00	0.95*
	North	2	Thru	723	63	1.00	0.95*
	West	1	Left	116	10	1.00	0.95*
-----							
East: M7 Ramps East							
	South	4	Left	4	0	1.00	0.95*
	North	6	Right	100	9	1.00	0.95*
	West	5	Thru	37	3	1.00	0.95*
-----							
North: Wallgrove Road North App							
	South	8	Thru	985	86	1.00	0.95*
	East	7	Left	10	1	1.00	0.95*
	West	9	Right	182	16	1.00	0.95*

```

-----
West: Old Wallgrove Road West
      South      12   Right    56      5      1.00    0.95*
      East       11   Thru     33      3      1.00    0.95*
      North      10   Left    126     11      1.00    0.95*
-----

```

Unit Time for Volumes = 60 minutes

Peak Flow Period = 60 minutes

Flow Rates include effects of Flow Scale and Peak Flow Factor

- \* The Unit Time for Volumes is less than the implied Minimum Total Flow Period for peaking purposes. Check the specified values of Unit Time for Volumes, Peak Flow Period and Peak Flow Factor to ensure that they are as intended. Refer to the User Guide section on the Volumes input dialog for further information.

**Table B.2A - Flow Rates (Separate Light and Heavy Vehicles)**

```

Wallgoove/Old Wallgrove Existing
PA1 Future AM
Intersection ID: C0
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

```

```

-----
Mov      Left      Through      Right
ID      LV      HV      LV      HV      LV      HV
-----
Demand flows in veh/hour as used by the program
South: Wallgrove Road South
  1 L      116      10         0         0         0         0
  2 T         0         0      723      63         0         0
  3 R         0         0         0         0         5         1
-----
East: M7 Ramps East
  4 L         4         1         0         0         0         0
  5 T         0         0      37         3         0         0
  6 R         0         0         0         0      100         9
-----
North: Wallgrove Road North App
  7 L         10         1         0         0         0         0
  8 T         0         0      985      86         0         0
  9 R         0         0         0         0      182        16
-----
West: Old Wallgrove Road West
 10 L      126      11         0         0         0         0
 11 T         0         0      33         3         0         0
 12 R         0         0         0         0        56         5
-----

```

Unit Time for Volumes = 60 minutes

Peak Flow Period = 60 minutes

Flow Rates include effects of Flow Scale and Peak Flow Factor

**Table B.2B - Flow Rates (Total Vehicles and Percent Heavy)**

```

Wallgoove/Old Wallgrove Existing
PA1 Future AM
Intersection ID: C0
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

```

```

-----
Mov      Left      Through      Right
ID      LV      HV      LV      HV      LV      HV
-----

```

	Total	%HV	Total	%HV	Total	%HV
Demand flows in veh/hour as used by the program						
South: Wallgrove Road South						
1 L	126	7.9	0	0.0	0	0.0
2 T	0	0.0	786	8.0	0	0.0
3 R	0	0.0	0	0.0	6	16.7
East: M7 Ramps East						
4 L	5	20.0	0	0.0	0	0.0
5 T	0	0.0	40	7.5	0	0.0
6 R	0	0.0	0	0.0	109	8.3
North: Wallgrove Road North App						
7 L	11	9.1	0	0.0	0	0.0
8 T	0	0.0	1071	8.0	0	0.0
9 R	0	0.0	0	0.0	198	8.1
West: Old Wallgrove Road West						
10 L	137	8.0	0	0.0	0	0.0
11 T	0	0.0	36	8.3	0	0.0
12 R	0	0.0	0	0.0	61	8.2

Unit Time for Volumes = 60 minutes  
Peak Flow Period = 60 minutes  
Flow Rates include effects of Flow Scale and Peak Flow Factor

Table B.3 - Pedestrian Flow Rates

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Flow Rate (ped/h)	Flow Scale	Peak Flow Factor
Across North Approach			
P5	53	1.00	0.95*
Across West Approach			
P7	53	1.00	0.95*

Unit Time for Volumes = 60 minutes  
Peak Flow Period = 60 minutes  
Flow Rates include effects of Flow Scale and Peak Flow Factor

\* The Unit Time for Volumes is less than the implied Minimum Total Flow Period for peaking purposes. Check the specified values of Unit Time for Volumes, Peak Flow Period and Peak Flow Factor to ensure that they are as intended. Refer to the User Guide section on the Volumes input dialog for further information.

Table S.1 - Movement Phase and Timing Parameters

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov	Mov	PHASE	MATRIX	Lost Tim	Req.Mov.Time	Eff. Grn
-----	-----	-------	--------	----------	--------------	----------

ID	Typ	First Green				Second Green				-----		-----		-----	
		Fr	To	Op	Pr	Fr	To	Op	Pr	1st Grn	2nd Grn	1st Grn	2nd Grn	1st Grn	2nd Grn
South: Wallgrove Road South															
1	L (Slp)	A	B			*B	A			6	6	24.5	20.0Min	44	14
2	T	A	B							6		26.0		44	
3	R	A	B		Y					32		40.2		18	
East: M7 Ramps East															
4	L	B	A							6		20.0Min		14	
5	T	B	A							6		20.0Min		14	
6	R	B	A		Y					10		20.0Min		10	
North: Wallgrove Road North App															
7	L	A	B							6		44.9		44	
8	T	A	B							6		45.4		44	
9	R	*A	B		Y					16		46.4		34	
West: Old Wallgrove Road West															
10	L	B	A							6		20.0Min		14	
11	T	B	A							6		20.0Min		14	
12	R	B	A		Y					7		20.0Min		13	
Pedestrian Movements															
P5	(Ped)	B	A							14		20.0Min		6	
P7	(Ped)	A	B							14		20.0Min		36	
Current Phase Sequence: Two-phase															
Input phase sequence: A B															
Output phase sequence: A B															
* Critical Movement/Green Period															
Movement Types:															
Slp	Slip Lane Movement				Under heading 'Op':										
Ped	Pedestrian				Y If opposed turn										
Dum	Dummy														

Under heading 'Op':  
Y If opposed turn

Table S.2 - Movement Capacity Parameters

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Dem		Satn Flow		Flow Ratio		Total	Prac.	Prac.	Lane	Deg.
	Flow		1st	2nd	1st	2nd	Cap.	Deg.	Spare	Util	Satn
	(veh /h)	HV (%)	1st Grn	2nd Grn	1st Grn	2nd Grn	(veh /h)	Satn xp	Cap. (%)	(%)	x
South: Wallgrove Road South											
1 L	126	7.9	462	88	0.238	0.180	308	0.90	120	100	0.409
2 T	786	8.0	3059		0.257		1923	0.90	120	100	0.409
3 R	6	16.7	57		0.105		15	0.90	120	100	0.409
East: M7 Ramps East											
4 L	5	20.0	203		0.025		41	0.90	631	100	0.123
5 T	40	7.5	1627		0.025		325	0.90	632	100	0.123
6 R	109	8.3	1473		0.074		210	0.90	74	100	0.518
North: Wallgrove Road North App											
7 L	11	9.1	22		0.500		14	0.90	13	100	0.795
8 T	1071	8.0	2115		0.506		1329	0.90	12	100	0.806*
9 R	198	8.1	506		0.391		246	0.90	12	100	0.806*
West: Old Wallgrove Road West											
10 L	137	8.0	1756		0.078		351	0.90	131	100	0.390

11 T	36	8.3	563	0.064	113	0.90	181	100	0.320
12 R	61	8.2	1027	0.059	191	0.90	181	100	0.320
-----									
Pedestrian Movements									
P5	53		12000	0.004	1029	0.90		0	0.052
P7	53		12000	0.004	6171	0.90		0	0.009
-----									

**Table S.3 - Intersection Parameters**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Crit Mov ID	App. and Turn	Green Period	Phases ----- Fr To	Adjusted Lost Time	Adjusted Flow Ratio	Required Grn Time Ratio	Required Movement Time
9	N_R		A B	16	0.391	0.435	46.4
1	S_L	2nd	B A	20	-	-	20.0Min
Total:				36	0.391	0.435	66.4

- Flow ratio not used for cycle time calculations and the adjusted lost time equals the required movement time (=Min or Max as shown in Table S.1)

Cycle Time:

Minimum	Maximum	Practical	Chosen
40	150	64	70

Intersection Level of Service	=	B
Worst movement Level of Service	=	C
Average intersection delay (s/pers)	=	17.0
Largest average movement delay (s)	=	40.5
Largest back of queue, 95% (m)	=	198
Performance Index	=	91.11
Degree of saturation (highest)	=	0.806
Practical Spare Capacity (lowest)	=	12 %
Effective intersection capacity, (veh/h)	=	3210
Total vehicle flow (veh/h)	=	2586
Total pedestrian flow (ped/h)	=	106
Total person flow (pers/h)	=	3985
Total vehicle delay (veh-h/h)	=	12.18
Total pedestrian delay (ped-h/h)	=	0.55
Total person delay (pers-h/h)	=	18.83
Total effective vehicle stops (veh/h)	=	1867
Total effective pedestrian stops (ped/h)	=	74
Total effective person stops (pers/h)	=	2875
Total vehicle travel (veh-km/h)	=	1567.6
Total cost (\$/h)	=	1390.74
Total fuel (L/h)	=	215.7
Total CO2 (kg/h)	=	540.96

**Table S.4 - Phase Information**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Phase	Change Time	Starting Intgrn	Green Start	Displayed Green	Green End	Terminating Intgrn	Phase Time	Phase Split
A	0	6	6	44	50	6	50	71%
B	50	6	56	14	70	6	20	29%

Current Phase Sequence: Two-phase  
Input phase sequence: A B  
Output phase sequence: A B

**Table S.5 - Movement Performance**

Mov ID	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Prop. Queued	Eff. Stop Rate	Longest Queue 95% Back (vehs)	Queue (m)	Perf. Index	Aver. Speed (km/h)
South: Wallgrove Road South									
1 L	0.49	0.74	14.1	0.52	0.77	9.0	67	3.50	43.5
2 T	1.55	2.33	7.1	0.55	0.49	9.7	72	19.74	50.2
3 R	0.03	0.04	16.4	0.57	0.78	9.7	72	0.18	41.6
East: M7 Ramps East									
4 L	0.05	0.07	33.8	0.85	0.74	1.9	15	0.21	31.2
5 T	0.28	0.42	25.3	0.85	0.64	1.9	15	1.53	35.4
6 R	1.23	1.84	40.5	0.97	0.79	5.1	38	5.06	28.5
North: Wallgrove Road North App									
7 L	0.07	0.10	21.7	0.82	0.92	26.5	198	0.42	37.7
8 T	4.42	6.62	14.8	0.83	0.82	26.5	198	39.35	42.6
9 R	1.85	2.77	33.6	0.89	1.02	14.0	105	9.41	31.3
West: Old Wallgrove Road West									
10 L	1.36	2.04	35.7	0.91	0.79	5.7	42	5.92	30.4
11 T	0.27	0.40	26.9	0.90	0.71	4.2	31	1.45	34.5
12 R	0.60	0.90	35.3	0.90	0.78	4.2	31	2.62	30.5
Pedestrian Movements									
P5	0.43	0.43	29.3	0.91	0.91	0.1	0	1.08	2.2
P7	0.12	0.12	8.3	0.49	0.49	0.1	0	0.64	3.5

**Table S.6 - Intersection Performance**

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Total Flow (veh/h)	Deg. Satn x	Total Delay (veh-h/h)	Total Delay (pers-h/h)	Aver. Delay (sec)	Prop. Queued	Eff. Stop Rate	Longest Queue (m)	Perf. Index	Aver. Speed (km/h)
South: Wallgrove Road South									
918	0.409	2.07	3.11	8.1	0.55	0.53	72	23.42	49.1
East: M7 Ramps East									
154	0.518	1.55	2.33	36.3	0.94	0.75	38	6.80	30.1
North: Wallgrove Road North App									
1280	0.806	6.33	9.49	17.8	0.84	0.85	198	49.18	40.3

West: Old Wallgrove Road West									
234	0.390	2.22	3.34	34.2	0.91	0.77	42	9.99	31.0
-----									
Pedestrians:									
53	0.009	0.12	0.12	8.3	0.49	0.49	0	0.64	3.5
-----									
ALL VEHICLES:									
2586	0.806	12.18	18.27	17.0	0.74	0.72	198	89.39	41.0
-----									
INTERSECTION (persons):									
3985	0.806		18.83	17.0	0.74	0.72		91.11	40.1
-----									
Queue values in this table are 95% back of queue (metres).									

Table S.7 - Lane Performance

Wallgoove/Old Wallgrove Existing												
PA1 Future AM												
Intersection ID: C0												
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)												
-----												
	Effective Red and Green Times (sec)				Dem Flow	Cap	Deg.	Aver.	Eff.	Q u e u e		
Lane	-----				(veh	(veh	Satn	Delay	Stop	95% Back		Lane
No.	R1	G1	R2	G2	/h)	/h)	x	(sec)	Rate	(vehs)	(m)	Length
-----												
South: Wallgrove Road South												
1 LT	19	44	6	1	477	1167	0.409	8.2	0.55	9.0	67.4	500.0
2 TR	28	42	0	0	441	1079	0.409	8.1	0.50	9.7	72.3	500.0
-----												
East: M7 Ramps East												
1 LT	56	14	0	0	45	366	0.123	26.3	0.65	1.9	14.6	500.0
2 R	60	10	0	0	109	210	0.518	40.5	0.79	5.1	37.9	500.0
-----												
North: Wallgrove Road North App												
1 LT	26	44	0	0	938	1164	0.806	13.3	0.80	26.5	198.5	500.0
2 TR	34	36	0	0	342	425	0.806	30.0	0.98	14.0	104.6	500.0
-----												
West: Old Wallgrove Road West												
1 L	56	14	0	0	137	351	0.390	35.7	0.79	5.7	42.5	220.0
2 TR	56	14	0	0	97	303	0.320	32.2	0.75	4.2	31.2	500.0
-----												

Table S.8 - Lane Flow and Capacity Information

Wallgoove/Old Wallgrove Existing												
PA1 Future AM												
Intersection ID: C0												
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)												
-----												
Lane	Dem Flow (veh/h)				Lane	Saturation Flow			End	Tot		
No.	-----				Width	Adj.	Aver	Aver	Cap	Cap	Deg.	Lane
	Lef	Thru	Rig	Tot	(m)	Basic	1st	2nd	(veh	(veh	Satn	Util
						(tcu)	(veh)	(veh)	/h)	/h)	x	%
-----												
South: Wallgrove Road South												
1 LT	126	351	0	477	3.30	1950	1827	1292	0	1167	0.409	100
2 TR	0	435	6	441	3.30	1950	1798	0	47	1079	0.409	100
-----												
East: M7 Ramps East												
1 LT	5	40	0	45	3.30	1950	1830	0	0	366	0.123	100
2 R	0	0	109	109	3.30	1950	1473	0	110	210	0.518	100

Basic Saturation Flow in this table is adjusted for lane width, approach grade, parking manoeuvres and number of buses stopping. Saturation flow scale applies if specified.

### Table S.9 - Signal Timing Diagram

11/05/2008



5 T	40	325	100	0.123	14	25.3	0.64	1.9	1.53
6 R	109	210	100	0.518	10	40.5	0.79	5.1	5.06
-----									
North: Wallgrove Road North App									
7 L	11	14	100	0.795	44	21.7	0.92	26.5	0.42
8 T	1071	1329	100	0.806*	44	14.8	0.82	26.5	39.35
9 R	198	246	100	0.806*	34*	33.6	1.02	14.0	9.41
-----									
West: Old Wallgrove Road West									
10 L	137	351	100	0.390	14	35.7	0.79	5.7	5.92
11 T	36	113	100	0.320	14	26.9	0.71	4.2	1.45
12 R	61	191	100	0.320	13	35.3	0.78	4.2	2.62
-----									
Pedestrian Movements									
P5 (Ped)	53	1029	100	0.052	6	29.3	0.91	0.1	1.08
P7 (Ped)	53	6171	100	0.009	36	8.3	0.49	0.1	0.64
-----									
* Maximum degree of saturation, or critical green periods									

Table S.12A - Fuel Consumption, Emissions and Cost (TOTAL)

Wallgoove/Old Wallgrove Existing						
PA1 Future AM						
Intersection ID: C0						
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)						
-----						
Mov	Fuel	Cost	HC	CO	NOX	CO2
ID	Total	Total	Total	Total	Total	Total
	L/h	\$/h	kg/h	kg/h	kg/h	kg/h
-----						
South: Wallgrove Road South						
1 L	10.8	64.51	0.043	2.22	0.067	27.0
2 T	56.2	344.35	0.207	9.29	0.309	141.0
3 R	0.5	3.17	0.002	0.11	0.003	1.3
-----						
	67.5	412.04	0.251	11.62	0.379	169.3
-----						
East: M7 Ramps East						
4 L	0.5	3.32	0.002	0.09	0.003	1.2
5 T	3.5	23.90	0.014	0.67	0.020	8.9
6 R	10.7	78.16	0.045	2.09	0.062	26.8
-----						
	14.7	105.38	0.061	2.85	0.085	36.8
-----						
North: Wallgrove Road North App						
7 L	1.0	6.33	0.004	0.21	0.006	2.5
8 T	91.0	555.87	0.360	18.12	0.547	228.3
9 R	19.2	132.76	0.080	3.95	0.116	48.3
-----						
	111.3	694.95	0.445	22.29	0.669	279.1
-----						
West: Old Wallgrove Road West						
10 L	13.1	93.09	0.055	2.60	0.077	32.9
11 T	3.3	22.04	0.013	0.63	0.019	8.2
12 R	5.8	41.31	0.024	1.15	0.034	14.6
-----						
	22.2	156.44	0.092	4.38	0.130	55.7
-----						
Pedestrian Movements						
P5		13.55				
P7		8.36				
-----						
		21.91				
-----						
ALL VEHICLES:	215.7	1368.82	0.850	41.15	1.264	541.0
-----						
INTERSECTION:	215.7	1390.74	0.850	41.15	1.264	541.0
-----						

PARAMETERS USED IN COST CALCULATIONS

Pump price of fuel (\$/L)	=	1.200
Fuel resource cost factor	=	0.50
Ratio of running cost to fuel cost	=	3.0
Average income (\$/h)	=	28.00
Time value factor	=	0.60
Light vehicle mass (1000 kg)	=	1.4
Heavy vehicle mass (1000 kg)	=	11.0
Light vehicle idle fuel rate (L/h)	=	1.350
Heavy vehicle idle fuel rate (L/h)	=	2.000

**Table S.12B - Fuel Consumption, Emissions and Cost (RATE)**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Fuel Rate L/100km	Cost Rate \$/km	HC Rate g/km	CO Rate g/km	NOX Rate g/km	CO2 Rate g/km
-----						
South: Wallgrove Road South						
1 L	14.0	0.84	0.555	28.85	0.866	351.3
2 T	11.8	0.72	0.434	19.50	0.649	295.8
3 R	14.4	0.87	0.576	29.87	0.888	361.6
-----						
	12.1	0.74	0.451	20.85	0.681	303.9
-----						
East: M7 Ramps East						
4 L	15.7	1.10	0.652	31.21	0.925	395.5
5 T	14.6	0.99	0.588	27.75	0.844	365.9
6 R	16.2	1.19	0.681	31.70	0.939	406.4
-----						
	15.8	1.13	0.656	30.65	0.914	395.5
-----						
North: Wallgrove Road North App						
7 L	15.1	0.95	0.619	31.88	0.935	379.6
8 T	14.0	0.86	0.555	27.90	0.843	351.5
9 R	16.1	1.11	0.673	33.05	0.966	403.5
-----						
	14.3	0.90	0.573	28.73	0.862	359.8
-----						
West: Old Wallgrove Road West						
10 L	15.9	1.12	0.661	31.42	0.931	397.7
11 T	14.9	1.01	0.606	28.88	0.870	374.3
12 R	15.8	1.12	0.657	31.22	0.927	396.0
-----						
	15.7	1.11	0.651	30.98	0.920	393.6
-----						
Pedestrian Movements						
P5		7.70				
P7		4.75				
-----						
		6.23				
-----						
ALL VEHICLES:	13.8	0.87	0.542	26.25	0.806	345.1
-----						
INTERSECTION:	13.8	0.89	0.542	26.25	0.806	345.1
-----						

**Table S.14 - Summary of Input and Output Data**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Lane No.	Demand Flow (veh/h)				%HV	Adj. Basic	Eff Grn (secs)		Deg Sat	Aver. Delay	Longest Queue	Shrt Lane
	L	T	R	Tot		Satf.	1st	2nd	x	(sec)	(m)	(m)
South: Wallgrove Road South												
1 LT	126	351		477	8	1950	44	1	0.409	8.2	67	500
2 TR		435	6	441	8	1950	42		0.409	8.1	72	500
	126	786	6	918	8				0.409	8.1	72	
East: M7 Ramps East												
1 LT	5	40		45	9	1950	14		0.123	26.3	15	500
2 R			109	109	8	1949	10		0.518	40.5	38	500
	5	40	109	154	8				0.518	36.3	38	
North: Wallgrove Road North App												
1 LT	11	927		938	8	1950	44		0.806	13.3	198	500
2 TR		144	198	342	8	1950	36		0.806	30.0	105	500
	11	1071	198	1280	8				0.806	17.8	198	
West: Old Wallgrove Road West												
1 L	137			137	8	1949	14		0.390	35.7	42	220
2 TR		36	61	97	8	1949	14		0.320	32.2	31	500
	137	36	61	234	8				0.390	34.2	42	
Pedestrians												
Across N approach				53			6		0.052	29.3	0.1	
Across W approach				53			36		0.009	8.3	0.1	
=====												
ALL VEHICLES				Total Flow	% HV		Cycle Time		Max X	Aver. Delay	Max Queue	
				2586	8		70		0.806	17.0	198	
=====												
Peak flow period = 60 minutes.												

Peak flow period = 60 minutes.

Queue values in this table are 95% back of queue (metres).

Note: Basic Saturation Flows (in through car units) have been adjusted for grade, lane widths, parking manoeuvres and bus stops.

Table S.15 - Capacity and Level of Service

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Mov Typ	Green Time Ratio (g/C)		Total Flow (veh/h)	Total Cap. (veh/h)	Deg. of Satn (v/c)	Aver. Delay (sec)	LOS	Longest Queue 95% Back (vehs)	Queue (m)
		1st grn	2nd grn							
South: Wallgrove Road South										
1 L	(Slp)	0.629	0.200*	126	308	0.409	14.1	A	9.0	67
2 T		0.629		786	1923	0.409	7.1	A	9.7	72
3 R		0.257		6	15	0.409	16.4	B	9.7	72
East: M7 Ramps East										
4 L		0.200		5	41	0.123	33.8	C	1.9	15
5 T		0.200		40	325	0.123	25.3	B	1.9	15
6 R		0.143		109	210	0.518	40.5	C	5.1	38

-----								
North: Wallgrove Road North App								
7 L	0.629	11	14	0.795	21.7	B	26.5	198
8 T	0.629	1071	1329	0.806*	14.8	B	26.5	198
9 R	0.486*	198	246	0.806*	33.6	C	14.0	105
-----								
West: Old Wallgrove Road West								
10 L	0.200	137	351	0.390	35.7	C	5.7	42
11 T	0.200	36	113	0.320	26.9	B	4.2	31
12 R	0.186	61	191	0.320	35.3	C	4.2	31
-----								
Pedestrian Movements								
P5 (Ped)	0.086	53	1029	0.052	29.3	C	0.1	0
P7 (Ped)	0.514	53	6171	0.009	8.3	A	0.1	0
-----								
ALL VEHICLES:		2586		0.806	17.0	B	26.5	198
-----								
INTERSECTION (persons):		3985			17.0		26.5	198
-----								
Level of Service calculations are based on average control delay including geometric delay (RTA NSW criteria), independent of the current delay definition used.								
For the criteria, refer to the "Level of Service" topic in the SIDRA Output Guide or the Output section of the on-line help.								
Intersection capacity is calculated considering vehicle movements only.								
* Maximum v/c ratio, or critical green periods								
" Movement Level of service has been determined using adjacent lane v/c ratio rather than short lane v/c ratio (v/c=1.0)								

Table S.16 - SCATS MF Parameter

Wallgrove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

-----						
Lane No.	Stopline Flow (veh/h)	Capacity (veh/h)	SCATS Satn Flow	SCATS MF	Deg. Satn x	Lane Util. %
-----						
South: Wallgrove Road South						
1 LT	477	1167	1925	NA	0.409	100
2 TR	441	1079	1949	1637	0.409	100
-----						
East: M7 Ramps East						
1 LT	45	366	1939	1357	0.123	100
2 R	109	210	1857	929	0.518	100
-----						
North: Wallgrove Road North App						
1 LT	938	1164	1949	1715	0.806	100
2 TR	342	425	1895	1365	0.806	100
-----						
West: Old Wallgrove Road West						
1 L	137	351	1857	1300	0.390	100
2 TR	97	303	1891	1323	0.320	100
-----						

NA Not Applicable - SCATS MF was not calculated for this lane due to one of the following reasons:  
- the lane is not controlled by signals (slip or continuous lane)  
- two movements share this lane and do not run in the same phases

STOPLINE FLOW: Departure flow rate in veh/h as measured at the stop line. This cannot exceed capacity.

SCATS SATURATION FLOW: This allows for lane width, approach grade and turning vehicles. Saturation flow scale applies if specified. The effects of heavy vehicles, parking manoeuvres, number of buses stopping and conflicting pedestrian volume are not included.

SCATS MF: This emulates the MF (Maximum Flow) parameter used in the SCATS control system. It is calculated from the SCATS SATURATION FLOW parameter.

DEG. SATN: The Demand (Arrival) Flow Rate may exceed the Stopline Flow Rate, therefore  $x > 1$  is possible.

**Table D.0 - Geometric Delay Data**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

From Approach	To Approach	Turn	Negn Radius (m)	Negn Speed (km/h)	Negn Dist. (m)	Appr. Dist. (m)	Downstream (m)	Distance User Spec?
South: Wallgrove Road South								
	East	Right	10.9	20.9	17.2	500	112	No
	North	Thru	S	60.0	16.5	500	113	No
	West	Left	15.0	23.5	23.6	500	116	No
East: M7 Ramps East								
	South	Left	10.0	20.2	15.7	500	111	No
	North	Right	9.9	20.1	15.6	500	111	No
	West	Thru	S	60.0	18.5	500	113	No
North: Wallgrove Road North App								
	South	Thru	S	60.0	16.5	500	113	No
	East	Left	10.0	20.2	15.7	500	111	No
	West	Right	9.9	20.1	15.6	500	111	No
West: Old Wallgrove Road West								
	South	Right	10.9	20.9	17.2	500	112	No
	East	Thru	S	60.0	18.5	500	113	No
	North	Left	10.0	20.2	15.7	500	111	No

Downstream distance is distance travelled from the stopline until exit cruise speed is reached (includes negotiation distance). Acceleration distance is weighted for light and heavy vehicles. The same distance applies for both stopped and unstopped vehicles.

**Table D.1 - Lane Delays**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Delay (seconds/veh)										
Lane No.	Deg. Satn x	Stop-line 1st d1	Stop-line 2nd d2	Stop-line Total dSL	Acc. Dec. dn	Queuing Total dq	MvUp dqm	Stopd (Idle) di	Geom dig	Control dic
South: Wallgrove Road South										
1 LT	0.409	6.0	0.0	6.0	3.9	2.1	0.2	1.9	2.1	8.2
2 TR	0.409	8.0	0.0	8.0	4.8	3.2	0.0	3.2	0.1	8.1
East: M7 Ramps East										
1 LT	0.123	25.3	0.0	25.3	6.9	18.5	0.0	18.5	0.9	26.3
2 R	0.518	32.0	0.0	32.0	3.9	28.1	0.0	28.1	8.5	40.5
North: Wallgrove Road North App										

1 LT	0.806	10.5	2.7	13.2	6.9	6.3	0.4	5.9	0.1	13.3
2 TR	0.806	15.9	9.2	25.1	5.3	19.8	2.1	17.7	4.9	30.0
-----										
West: Old Wallgrove Road West										
1 L	0.390	27.2	0.0	27.2	3.7	23.5	0.0	23.5	8.5	35.7
2 TR	0.320	26.9	0.0	26.9	5.2	21.7	0.0	21.7	5.3	32.2
-----										
dn is average stop-start delay for all vehicles queued and unqueued										

Table D.2 - Lane Stops

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

		Effective Stop Rate				Queue	
Lane	Deg.	Geom.		Overall	Prop.	Move-up	
No.	Satn	he1	he2	hig	h	Queued	Rate
	x					pq	hqm
-----							
South: Wallgrove Road South							
1 LT	0.409	0.46	0.01	0.08	0.55	0.524	0.06
2 TR	0.409	0.50	0.00	0.00	0.50	0.568	0.00
-----							
East: M7 Ramps East							
1 LT	0.123	0.64	0.00	0.01	0.65	0.853	0.00
2 R	0.518	0.77	0.00	0.02	0.79	0.970	0.00
-----							
North: Wallgrove Road North App							
1 LT	0.806	0.75	0.05	0.00	0.80	0.815	0.06
2 TR	0.806	0.79	0.15	0.04	0.98	0.889	0.26
-----							
West: Old Wallgrove Road West							
1 L	0.390	0.73	0.00	0.06	0.79	0.912	0.00
2 TR	0.320	0.71	0.00	0.04	0.75	0.896	0.00
-----							
hig is the average value for all movements in a shared lane							
hqm is average queue move-up rate for all vehicles queued and unqueued							

Table D.3A - Lane Queues (veh)

Wallgoove/Old Wallgrove Existing  
PA1 Future AM  
Intersection ID: C0  
Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Lane No.	Deg. Satn	Ovrfl. Queue	Average (veh)			Percentile (veh)					Queue Stor.
	x	No	Nb1	Nb2	Nb	70%	85%	90%	95%	98%	Ratio
South: Wallgrove Road South											
1 LT	0.409	0.0	4.5	0.0	4.5	5.6	6.8	7.6	9.0	10.4	0.13
2 TR	0.409	0.0	4.9	0.0	4.9	6.1	7.4	8.3	9.7	11.1	0.14
East: M7 Ramps East											
1 LT	0.123	0.0	0.8	0.0	0.8	1.0	1.3	1.5	1.9	2.4	0.03
2 R	0.518	0.0	2.3	0.0	2.3	2.9	3.6	4.1	5.1	6.0	0.08
North: Wallgrove Road North App											
1 LT	0.806	0.8	14.8	1.4	16.2	19.5	22.8	24.6	26.5	28.5	0.40

2 TR	0.806	1.0	6.2	1.5	7.7	9.4	11.3	12.4	14.0	15.6	0.21
-----											
West: Old Wallgrove Road West											
1 L	0.390	0.0	2.6	0.0	2.6	3.3	4.1	4.6	5.7	6.7	0.19
2 TR	0.320	0.0	1.8	0.0	1.8	2.3	2.9	3.4	4.2	5.0	0.06
-----											

Values printed in this table are back of queue (vehicles).

**Table D.3B - Lane Queues (metres)**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Lane No.	Deg. Satn	Ovrfl. Queue	Average (metres)			Percentile (metres)					Queue Stor.
	x	No	Nb1	Nb2	Nb	70%	85%	90%	95%	98%	Ratio
South: Wallgrove Road South											
1 LT	0.409	0.0	33.6	0.0	33.6	41.6	51.1	57.2	67.4	77.6	0.13
2 TR	0.409	0.0	36.6	0.0	36.6	45.3	55.4	61.8	72.3	82.9	0.14
East: M7 Ramps East											
1 LT	0.123	0.0	6.0	0.0	6.0	7.7	9.9	11.5	14.6	17.8	0.03
2 R	0.518	0.0	16.9	0.0	16.9	21.4	27.0	30.8	37.9	45.0	0.08
North: Wallgrove Road North App											
1 LT	0.806	5.9	110.7	10.4	121.1	145.8	171.0	184.0	198.5	213.0	0.40
2 TR	0.806	7.4	46.6	11.1	57.6	70.4	84.4	92.6	104.6	116.5	0.21
West: Old Wallgrove Road West											
1 L	0.390	0.0	19.3	0.0	19.3	24.4	30.5	34.8	42.5	50.2	0.19
2 TR	0.320	0.0	13.6	0.0	13.6	17.3	21.9	25.1	31.2	37.3	0.06

Values printed in this table are back of queue (metres).

**Table D.4 - Movement Speeds (km/h) and Geometric Delay**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	App. Speeds		Exit Speeds		Queue Move-up		Av. Section Spd		Geom. Delay (sec)
	-----		-----		1st	2nd	-----		
	Cruise	Negn	Negn	Cruise	Grn	Grn	Running	Overall	
-----									
South: Wallgrove Road South									
1 L	60.0	23.5	23.5	60.0		6.4	46.7	43.5	8.1
2 T	60.0	60.0	60.0	60.0		0.0	53.0	50.2	0.0
3 R	60.0	20.9	20.9	60.0			46.5	41.6	8.4
-----									
East: M7 Ramps East									
4 L	60.0	20.2	20.2	60.0			45.4	31.2	8.5
5 T	60.0	60.0	60.0	60.0			50.0	35.4	0.0
6 R	60.0	20.1	20.1	60.0			45.0	28.5	8.5
-----									
North: Wallgrove Road North App									

7 L	60.0	20.2	20.2	60.0	50.5	45.1	37.7	8.5
8 T	60.0	60.0	60.0	60.0	47.8	49.5	42.6	0.0
9 R	60.0	20.1	20.1	60.0	30.5	43.4	31.3	8.5
-----								
West: Old Wallgrove Road West								
10 L	60.0	20.2	20.2	60.0		45.2	30.4	8.5
11 T	60.0	60.0	60.0	60.0		49.6	34.5	0.0
12 R	60.0	20.9	20.9	60.0		45.2	30.5	8.4
-----								

"Running Speed" is the average speed excluding stopped periods.

**Table D.5 - Progression Factors and Actuated Signal Parameters**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Control	Coord.	Arrival Type	Delay Prog. Factor	Queue Prog. Factor	Disp. Gmin	Grn. Gmax	Settings 1st Grn Gmin	2nd Grn Gmax
-----									
South: Wallgrove Road South									
1 L	FT	No	3	1.000	1.000	6	NA	6	NA
2 T	FT	No	3	1.000	1.000	6	NA		
3 R	FT	No	3	1.000	1.000	6	NA		
-----									
East: M7 Ramps East									
4 L	FT	No	3	1.000	1.000	6	NA		
5 T	FT	No	3	1.000	1.000	6	NA		
6 R	FT	No	3	1.000	1.000	6	NA		
-----									
North: Wallgrove Road North App									
7 L	FT	No	3	1.000	1.000	6	NA		
8 T	FT	No	3	1.000	1.000	6	NA		
9 R	FT	No	3	1.000	1.000	6	NA		
-----									
West: Old Wallgrove Road West									
10 L	FT	No	3	1.000	1.000	6	NA		
11 T	FT	No	3	1.000	1.000	6	NA		
12 R	FT	No	3	1.000	1.000	6	NA		
-----									
Pedestrians									
P5	FT	No	3	1.000	1.000				
P7	FT	No	3	1.000	1.000				
-----									

**Table D.6 - Gap Acceptance Parameters**

Wallgoove/Old Wallgrove Existing

PA1 Future AM

Intersection ID: C0

Fixed-Time Signals, Cycle Time = 70 (Practical Cycle Time)

Mov ID	Mov Type	Opng Flow (pcu/h)	Critical Gap		Foll-up Headway (s)	Entry HV Equiv
			Hdwy (s)	Dist (m)		
			-----			
South: Wallgrove Road South						
3 R	Normal	1116	5.03	83.2	2.90	2.00

-----						
East: M7 Ramps East						
4 L	Normal	0	4.60	76.7	2.76	2.00
6 R	Normal	178	4.65	36.7	2.69	2.00
-----						
North: Wallgrove Road North App						
9 R	Normal	940	4.64	70.9	2.68	2.00
-----						
West: Old Wallgrove Road West						
10 L	Normal	0	4.12	68.7	2.47	2.00
12 R	Normal	47	4.65	72.5	2.68	2.00
-----						

Values in this table are adjusted for heavy vehicles in the entry stream.



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