

UTS Bon Marche and Science Precinct Projects

Traffic and Transport Impact Assessment

Prepared for:

University of Technology, Sydney

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The Transport Planning Partnership

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Table of Contents

1	Exe	cutive Summary
2	Intro	oduction4
	2.1	Overview of Proposed Modification4
	2.2	Background
		2.2.1 Evolution of UTS
		2.2.2 Evolution of Concept Plan
	2.3	SEARs
	2.4	The Site
	2.5	Relevant Transport Studies
		2.5.1 UTS Broadway Traffic Report
		2.5.2 Transport Management and Accessibility Plan (TMAP) Report
		2.5.3 UTS Sustainable Transport Plan 2016-2017
		2.5.4 UTS City Campus Masterplan Cyclists Facility Strategy
3	Exis	ting Conditions
	3.1	Road Network
		3.1.1 Thomas Street
		3.1.2 Jones Street
		3.1.3 Harris Street
		3.1.4 Wattle Street
		3.1.5 Broadway
	3.2	Public Transport
		3.2.1 Bus Network
		3.2.2 Rail Network
		3.2.3 Future Public Transport Network
	3.3	Pedestrian Infrastructure
	3.4	Cycle Infrastructure
	3.5	Car Sharing Pods
	3.6	Traffic Surveys
		3.6.1 Vehicle Volumes
		3.6.2 Pedestrian Volumes
	3.7	On-street Parking Survey
		3.7.1 UTS Building 10 Car Park Boom Gate Data
		3.7.2 Loading Dock Surveys



	3.8	Exis	ting Intersection Operation	
	3.9	Foc	otpath Capacity Assessment	
		3.9.1	Assessment Methodology	
	3.10	Trav	vel Mode Share	
4	Dev	elopr	nent Proposal	
	4.1	Со	ncept Development	
	4.2	Ca	mpus Population	
	4.3	Pro	posed Vehicle Access and Parking	
	4.4	Loc	ading Arrangements	
5	Traff	fic an	d Transport Assessment	40
	5.1	Traf	fic Generation	40
	5.2	Traf	fic Impact	40
		5.2.1	Post Development	
		5.2.2	Post Development + Harris Street Narrowing	
		5.2.3	Development + Midblock Pedestrian Crossing	43
		5.2.4	Development + Widened Crossing at Harris Street	43
		5.2.5	Development + All Pedestrian Amenity Improvements	45
	5.3	Ser	vice Vehicles Generation	45
	5.4	Pub	olic Transport Capacity	
		5.4.1	Non-Car Trip Generation	
		5.4.2	Future Public Transport Capacity	
	5.5	Pec	destrian Traffic Implications	
		5.5.1	Pedestrian Crash History	
		5.5.2	Effects of Additional Pedestrian Trips	
	5.6	Gre	en Travel Plan	50
	5.7	Со	nstruction Traffic Management Plan	50
6	Park	king A	ssessment	
	6.1	Ca	r Parking Requirement	
	6.2	Pro	posed Set-down/Pick up Parking	
	6.3	Bicy	ycle Parking Requirement	
7	Sum	mary	and Conclusions	



Tables

Table 2.1: SEARs Requirements	
Table 3.1: Bus Frequency Comparison of Bus Stops on Broadway	16
Table 3.2: Frequency of Rail Services	17
Table 3.3: Intersection Level of Service Criteria	
Table 3.4: Existing Intersection Operation	
Table 3.5: Footpath Capacity Criteria	
Table 3.6: Existing Footpath Capacity	
Table 3.7: Journey to Work Comparison	34
Table 3.8: UTS Student and Staff Mode Share	35
Table 4.1: Campus Population Projections (under development)	
Table 5.1: Post Development Intersection Operation	41
Table 5.2: Post Development + Road Narrowing	
Table 5.3: Post Development + Pedestrian Crossing	
Table 5.4: Post Development + Widened Crossing at Harris St	
Table 5.5: Post Development + All Amenity Improvements	45
Table 5.6: Non-Car Trip Generation	
Table 5.7: Harris Street Crash Data	
Table 5.8: Broadway/ George Street/ Regent Street Crash Data	

Figures

Figure 2.1: Key UTS Projects Approved/delivered Under the Concept Plan	6
Figure 2.2: 3D Model of Originally Approved Concept Plan (Source: BVN, DCM AND JBA)	8
Figure 2.3: Site Context	11
Figure 2.4: Aerial Image of Bon Marche and Science Precinct outlined in red) - May 2018	12
Figure 2.5: 3D Perspective of the Existing Bon Marche and Science Project	12
Figure 3.1: Sydney Metro Network Map	19
Figure 3.2: UTS Campus Hubs and Pedestrian Network	20
Figure 3.3: Cycle Network	21
Figure 3.4: Building 10 Bicycle Parking	22
Figure 3.5: Car Sharing Pods	23
Figure 3.6: Existing Traffic Volumes	24
Figure 3.7: Pedestrian Survey Location	25



Figure 3.8: Existing Pedestrian Flows	. 26
Figure 3.9: On-street Car Parking Capacity (Excluding Regent Street)	. 27
Figure 3.10: On-Street Parking Capacity (Regent Street Only)	27
Figure 3.11: UTS Building 10 Car Park Access Volumes	. 28
Figure 3.12: B1 Loading Dock Demand	29
Figure 3.13: Footpath Capacity Criteria	. 32
Figure 3.14: Destination Zone 113341158	34
Figure 4.1: Car Park Access	. 38
Figure 5.1: Harris Street Approach Crossing	. 44
Figure 5.2: Existing Pedestrian Connections	. 50

APPENDICES

- A. TRAFFIC SURVEYS
- B. SIDRA OUTPUTS



1 Executive Summary

The Transport Planning Partnership (TTPP) was commissioned by University of Technology, Sydney to prepare a traffic and transport assessment for the redevelopment of the UTS Bon Marche and Science Precinct (subject site). The assessment is part of a Section 75W modification application relating to the University of Technology Sydney (UTS) City Campus Broadway Concept Plan, which was approved in December 2009 (MP08_0116).

The S75W modification seeks to accommodate a rise in student and staff population that has exceeded the forecasts of the original UTS City Campus Broadway Concept Plan. Latest student and staff loading estimations indicate that the City Campus population is approximately 60 per cent greater than expected for the original concept plan.

Proposed Development Opportunities

As part of the assessment detailed in this report, TTPP has investigated potential opportunities to improve the pedestrian connectivity and accessibility of the UTS City Campus and address issues relating to parking, traffic and pedestrian movements. On this basis, the following opportunities have been assessed:

- Formal set-down/pick up parking restrictions on Thomas Street to address existing issues relating to drivers stopping on the carriageway to drop-off passengers to UTS
- New basement staff car park accommodating potentially up to 150 spaces, with access via the existing driveway located on Thomas Street.
- Reaccommodate Turner Lane loading activities to the Building 1 loading dock and remove Turner Lane.
- Widening of the pedestrian crossing at the Harris Street approach of the Harris Street, Broadway, George Street and Regent Street intersection to address existing concerns relating to pedestrian overflowing off the crossing.
- A new mid-block pedestrian crossing along Harris Street between Thomas Street and Broadway to accommodate existing informal pedestrian crossing activity and anticipated future growth in pedestrian volumes. As discussed in Section 3.6.2, up to 792 pedestrians per hour are crossing informally midblock along Harris Street.
- Widening of the footpath on Harris Street along the frontage of the UTS site to improve the pedestrian accessibility and amenity of the area. Implementation of this opportunity would however involve narrowing Harris Street and thereby losing one lane of traffic. Harris Street on approach to Broadway currently includes five lanes of traffic including one shared left turn and through lane, two through only lanes, one shared right turn and through lane and one right only lane.

TTPP have assessed the above opportunities and their impact to the road network and on-site and on-street parking in Section 5 and Section 6.



Assessment of Development Opportunities

The key findings of TTPP's assessment are as follows:

- A formal set-down/ pick-up area along Thomas Street would require the conversion of two to four existing ticketed kerbside parking spaces to 5-min parking restrictions.
- The new car park would generate up to 41 vehicle movements per hour and 36 vehicle movements per peak hour in the morning and evening peak periods respectively. This would have a negligible impact on the surrounding road network which sees over 2,000 vehicles per hour during the peak periods.
- Turner Lane generates a loading demand of four vehicles per day. The existing Building 1 loading dock is at 50 per cent occupancy and would be capable of accommodating the displaced Turner Lane loading demand. In addition, the Building 2 redevelopment (currently under construction) will include an expansion to the existing loading dock of around five loading bays, enabling the additional loading demand generated by the Building 2 development to be accommodated by these spaces.
- SIDRA modelling indicates:
 - Widening of the existing signalised crossing at the Harris Street approach to Broadway could be implemented with minor impacts to the signal operation of Harris Street, Broadway, Regent Street and George Street.
 - Widening of the Harris Street footpath and associated narrowing of the carriageway would notably impact queue lengths and delays along Harris Street and Ultimo Road. However, acceptable operation of the road network could be maintained with dynamic reallocation of green time to the Harris Street approach from the Broadway and George Street approaches. This is would be achieved within the minimum and maximum variable times provided at the existing intersection.
 - Provision of a signalised mid-block crossing could be provided with minimal impacts noting that the pedestrian crossing phase would run during the red signal on the Harris Street approach to Broadway.

Of particular note is the existing high volumes of pedestrian traffic in the vicinity of the site, in particular along Broadway. Fruin Analysis of the footpath along Broadway indicates theoretically that there is available capacity along Broadway, with a LoS B (recommended level of service for a footpath) along the Harris Street footpath and LoS C (acceptable but with movement becoming "increasingly uncomfortable") along the Broadway footpath. Visual inspection of the site indicates that these footpaths are congested and this will be due to pedestrians stopping at intersections where they have to congregate therefore affecting pedestrian flow. In particular, at the corner of the Broadway and Harris Street, where several conflicting pedestrian movements converge in one area creating difficulty in movement. Notably, pedestrians commonly overflow outside of the extents of the footpath and crossings. With a proposed increase to UTS

¹ See Figure 3.13



and general growth in the CBD, these volumes are anticipated to increase, opportunities to improve pedestrian footpath and crossing capacity are key in securing an acceptable level of pedestrian amenity and safety around and within the UTS City Campus.



2 Introduction

This report supports a Section 75W modification application submitted to the Minister for Planning pursuant to the Environmental Planning and Assessment Act 1979 (EP&A Act) and more specifically, Schedule 2 of the Environmental Planning and Assessment (Savings, Transitional and Other Provisions) Regulation 2017.

The Application relates to the Concept Plan Approval for the University of Technology Sydney (UTS) City Campus Broadway Precinct, which was approved in December 2009 (MP08_0116).

More specifically the modification application relates to the Bon Marche and Science Precinct (Buildings, 3, 4, 9 and 18) and includes establishing new building envelopes with corresponding height and Gross Floor Area (GFA).

The Transport Planning Partnership (TTPP) has prepared this report on behalf of UTS Sydney to accompany the Section 75W modification application to assess the traffic and transport implications of the modification.

2.1 Overview of Proposed Modification

The s75W Application seeks the following key modifications to the approved Concept Plan:

- Conceptual demolition of existing Building 4, and rear section of Building 3,
- Conceptual modification to heritage items, Building 3, Building 9, and Building 18;
- Creation of a new building envelope for Building 4, Building 3 (part) and Building 9 (cantilevering over only), resulting in a maximum height of RL 86.55, an increase of approximately 45m above existing Building 4 and approximately 50m above existing Building 3;
- Corresponding increase in GFA for Building 4 and Building 3, comprising an additional increase of up to 36,500m²;
- Consequential amendments to the Urban Design Quality Controls/Principles to guide the future development of the Bon Marche and Science Precinct; and
- Indicative landscape and public domain concept for the precinct.

The proposed new envelope for the Bon Marche and Science Precinct will accommodate a future building that will have an effective maximum height of 16/17 storeys above Harris Street and six (6) storeys above Thomas Street (i.e. excluding basement levels and plant). The resulting total GFA for the Bon Marche and Science Precinct (new building envelope and existing buildings) is some 65,000m².

No physical works are proposed as part of this s75W modification application, with detailed application(s) to follow any approval granted.



2.2 Background

2.2.1 Evolution of UTS

UTS was formed in 1988 from the former NSW Institute of Technology, and was restructured in 1990 with the merger of the Kuring-gai College of Advanced Education, the School of Design, and the Institute of Technical and Adult Teacher Education to form the current UTS. This change in profile, combined with the University's predominantly CBD location in Sydney, created a new identity. During its early evolution, student numbers increased at UTS without any significant increase in student facilities.

UTS recognised the need to upgrade the City Campus back in 2000, and undertook a number of visioning and master planning projects culminating in the City Campus Masterplan 2020 (BVN, 2008) which provided a framework for refurbishments and new building works across the campus (comprising the Broadway Precinct and other sites in the Sydney CBD) in order to provide improved facilities and to accommodate future expected student and staff growth.

On 23 December 2009 a critical step in realising UTS's vision and identity for the Broadway Precinct was realised, with approval of the UTS City Campus Broadway Precinct Concept Plan (BPCP).

Since approval of the Concept Plan in 2009 UTS has secured the necessary detailed planning approvals and delivered a number of state of the art and iconic learning, research and social facilities across the Broadway Precinct, including (refer to Figure 2.1):

- Faculty of Engineering and IT Building, designed by Denton Corker Marshall Architects.
- Multi-Purpose Sports Hall, designed by PTW Architects.
- Alumni Green, designed by ASPECT Studios Landscape Architects.
- Faculty of Science and Graduate School of Health Building, designed by Durbach Block Jaggers in association with BVN Architecture.
- Library Retrieval System, designed by Hassell Architects.
- Great Hall and Balcony Room Upgrade, Designed by DRAW Architects in association with Kann Finch Architects.
- Student Housing Building, designed by nettletontribe architects.

The UTS Central Project (designed by fijmt in collaboration with Lacoste + Stevenson in association with Darryl Jackson Robin Dyke Architects) represents the latest project being delivered by UTS to meet the needs of staff and students. The first phase of the UTS Central Project, which required a modification to the Concept Plan (MOD 5), is expected to be completed in 2019. The second phase of this project will include an extension to the podium of Building 1 addressing Broadway.



UTS currently has less than 2% of space across campus unallocated which is insufficient to accommodate forecast continued growth in student and staff numbers in the future. The educational facilities within the existing Bon Marche Building 3 are outdated and inadequate to meet the needs of contemporary teaching and learning environments.

The existing Science buildings (Building 4) are nearing the end of their lifecycle, which together with the continued growing demands from students locally and abroad and growth in both Science and Design, Architecture and Building (DAB) faculties presents an opportunity for UTS to progress with plans to support additional and much needed teaching and research space.

UTS plays an important role in the success of Sydney and NSW, with the Greater Sydney Commission's recently released Sydney Regional and District plans acknowledging this importance and identifying the need to protect and support the growth of education activity within the Harbour CBD Innovation Corridor.



Figure 2.1: Key UTS Projects Approved/delivered Under the Concept Plan

Source: BVN



2.2.2 Evolution of Concept Plan

The UTS City Campus Broadway Precinct Concept Plan (BPCP, as illustrated in Figure 2.2) was approved by the then Minister for Planning on 23 December 2009 (MP08_0116). The Concept Plan initially included:

- New Broadway Building and Thomas Street Building with a combined gross floor area (GFA) of 44,650m2;
- Expansion of Buildings 1 and 2 with a combined additional GFA of 10,800m2;
- Expansion of Building 6 for the provisions of student housing with an additional 25,250m2 GFA;
- Modifications to Buildings 3, 4 and 10;
- Modifications to Alumni Green with a new Multi-Purpose Sports Hall and book vault beneath; and
- Public domain improvements to Broadway and Thomas, Harris, Wattle and Jones Streets.

The Minister also granted Project Approval for the following works:

- Construction of a new underground Multi-Purpose Sports Hall; and
- Demolition of Buildings 11, 12 and 13.

The Concept Plan did not set new maximum heights and GFA for the Bon Marche and Science Precinct as demand for growth or redevelopment of these buildings was not identified at the time. The Concept Plan (2009) was informed by UTS's Growth Plan at the time to 2020, which had not foreseen that additional floor area and significant modifications and upgrades to existing buildings was required in the Bon Marche and Science Precinct. The 2009 Concept Plan also did not take into account the lifecycle status of Building 4, which was recently investigated and reported to be nearing end of life in 2026.





Figure 2.2: 3D Model of Originally Approved Concept Plan (Source: BVN, DCM AND JBA)

Since the Concept Plan was approved, five (5) subsequent modifications have been approved.

Modification No 1

Modification No 1 (MP 08_0116 Mod 1), approved in March 2011, sought to include bulk excavation works for the Broadway Building as part of the Project Approval works granted under the Concept Plan approval (enabling these works to be undertaken ahead of the Project Application for the building).

Modification No 2

Modification No 2 (MP 08_0116 Mod 2), approved in March 2011, related to an administration amendment to Concept Plan condition B2.



Modification No 3

Modification No 3 (MP 08_0116 Mod 3), approved in July 2011, sought to include the excavation, construction and operation of the Library Retrieval System (LRS) and Storage Building together with bulk excavation works for the Thomas Street Building as part of the Project Approval works granted under the Concept Plan approval (enabling these works to be undertaken without any further environmental assessment).

The modification also included a revised breakdown of GFA across the UTS Broadway site, with the Environmental Assessment submitted in support of the S75W identifying an increased GFA for the Thomas Street building of 12,150 square metres (corresponding with a decreased GFA for the Broadway Building of 34,650 square metres).

Modification No 4

Modification No 4 (MP 08_0116 Mod 4), approved in March 2012, related to an administration amendment to Concept Plan condition E3 (approved truck route plan for excavation of Thomas Street building and the library retrieval system).

Modification No 5

Modification No 5 (MP 08_0116 MOD 5) was approved by the then Minister for Planning in March 2016 and facilitated an expanded Building 2 envelope (maximum RL of 79.5) and corresponding increase in GFA for a new Building 2 and the Building 1 podium extension (resulting in a total maximum of 60,357sqm). [Alexis/Chris to check this is correct]

The modification provided the planning framework for the UTS Central project currently under construction.

Modification No 6

This report has been prepared in support of proposed Modification No 6 (MP 08_0116 Mod 6) to the Concept Plan.

2.3 SEARs

Secretary's Environmental Assessment Requirements (SEARs) were issued by the Department of Planning and Environment (DP&E) on 1 February 2018. Specifically, this report responds to the SEARs requirements listed in Table 2.1.



Table 2.1: SEARs Requirements

Requirement	Report Section	Comment
Modelling of the traffic impacts associated with the proposed modifications to the concept plan, including an estimate of the total daily and peak hour vehicle trips generated by the proposal	Section 5	
An assessment of the current and future performance of key intersections providing access to the site under the approved and proposed scenarios, and identify any additional upgrades required as a consequence of the proposal	Section 5.2	
An assessment of the impacts of all modifications to the approved road network and infrastructure, and use of the Austroads Guidelines to identify appropriate mitigation measures	Section 5	
Detailed plans of the proposed layout of the internal road network and on-site parking in accordance with the relevant Australian Standards	Section 4.3 Section 4.4 Section 6	The proposed layout and parking arrangements are in concept level at this stage. Assessment of Australian Standard compliance would be addressed in the Stage 2 DA.
An assessment of traffic and transport impacts during construction and demonstration of mitigation of impacts	Section 5.7	
An assessment of the adequacy of public transport services to meet the likely future demand of the proposed development	Section 5.4	

2.4 The Site

The Broadway Precinct of the UTS City Campus is located on the southern edge of the Sydney Central Business District (CBD). The UTS City Campus is located entirely within the Sydney Local Government Area.

The Campus has frontages to Broadway, Thomas, Wattle and Harris Streets, and the Goods Line [change text in diagram as no longer known as the UPN] and is less than 700 metres from Central Railway Station. Jones Street runs through the Precinct. The area covered by the Concept Plan (MP 08_0116) is shown in Figure 2.3.



ULTIMO RD ANNS MARY JONES STREET THE GOODS LINE WATTLESTREE HOMAS STREET GEORGEST LEEST BROADWAY BALFOUR ST KENTRD CHIPPEN LANE TON ST CARL FRASERS SITE SHOWN INDICATIVELY IRVING ST O'CONNOR ST TS ON

Figure 2.3: Site Context

Source: BVN

More specifically, the Bon Marche and Science Precinct is located within the eastern part of the Broadway campus between Thomas Street and Broadway with frontage to Harris Street. It incorporates Buildings 3, 4, 9 and 18. Buildings 3, 9 and 18 are identified as heritage items under the Sydney Local Environmental Plan 2012 (SLEP 2012). Refer to Figure 2.4 and Figure 2.5 for the location of the Bon Marche and Science Precinct.





Figure 2.4: Aerial Image of Bon Marche and Science Precinct outlined in red) - May 2018

The Site

NOT TO SCALE

Source: Nearmap

Figure 2.5: 3D Perspective of the Existing Bon Marche and Science Project





2.5 Relevant Transport Studies

2.5.1 UTS Broadway Traffic Report

The UTS Broadway Precinct – UTS Central Transport Impact Assessment was undertaken by GTA Consultants in April 2016. This addressed the transport and traffic concerns issued in the Secretary's Environmental Assessment Requirements (SEARs) for the project which was deemed as a Stage Significant Development under the *State Environmental Planning Policy* (*Stage and Regional Development*) 2011. The works involved the redevelopment of Building 2 and extension of the Building 1 podium to accommodate the increase in the projected student population in 2020.

The report indicated that the proposed land uses in Building 2 would contribute low trip generation rates. In addition to this, no parking was to be provided in the proposed redevelopment of Building 2 which was consistent with the approved UTS City Campus Plan which supports limited to no parking provision. Furthermore, 2011 Journey to Work data showed a high percentage of public transport usage in the travel zone containing UTS in which there was an increase of 6% in the mode share for public transport since the 2006 census, and a corresponding reduction in car usage.

Ultimately, the proposed development was found to be satisfactory from a traffic, transport and parking perspective.

A Green Travel Plan (GTP) was prepared by GTA Consultants in July 2016 to encourage sustainable transport in relation to the targets and objects set out in *Sustainable Sydney 2030* through the reduction in private vehicle travel.

2.5.2 Transport Management and Accessibility Plan (TMAP) Report

The Transport Management and Accessibility Plan report was completed by Halcrow MWT in October 2009 to address the Department of Planning's Director General's Key Assessment Requirement in relation to the proposed UTS City Campus Broadway Concept Plan. The report developed a TMAP for the site based on UTS's operation and transport situation at the time. In addition to this, analysis of the strategic context found that there was a very high transit mode share by existing students and staff due to the site's close proximity to Central Station and substantial number of bus services with limited level of on-site parking provision. The TMAP also noted that the patronage is "modest in the context of the current system" and well below the population and expected future growth of the surrounding area.

2.5.3 UTS Sustainable Transport Plan 2016-2017

The UTS Sustainable Transport Plan 2016-2017 supports the strategic context of UTS's Sustainability Policy and Sustainability Strategy 2016-2017 with the aim of reducing car usage as a transport mode for journeys to/from UTS.



The plan indicated a commitment to improving cycling facilities to attract students and staff to cycle to the site and pedestrianisation of Jones Street to encourage people to walk to UTS. These features are in-line with City of Sydney's Broadway Link which aims to increase pedestrian amenity and cyclist safety in the Darlington, Chippendale and Ultimo areas through the provision of parks, green spaces and car-free boulevards.

2.5.4 UTS City Campus Masterplan Cyclists Facility Strategy

As part of the UTS City Campus Masterplan 2020, UTS developed a bicycle parking strategy for staff and students. The 'Cyclist Facility Strategy' was prepared by Halcrow in September 2011.

The strategy determined the future bicycle parking requirements of the Campus based on Council parking rates as stipulated in the Development Control Plan and the future on-site population of the Campus.

Surveys of the existing Campus population were carried out and indicated that approximately 40 per cent of the Campus EFTSL was on-site at any one time. It was assumed that staff population on site would also be equivalent to 40% and would be consistent for students and staff in future years.

The results of study determined that 1,008 bicycle parking spaces would be required for the whole City Campus which includes 890 spaces for students and 118 spaces for staff.



3 Existing Conditions

3.1 Road Network

3.1.1 Thomas Street

Thomas Street is a local road and in the vicinity of the site is aligned in an east-west direction. It is a 12.8m wide, two-way road configured with one lane in each direction, set within a 20m wide road reserve (approx.). Kerbside parking is permitted on both sides of Thomas Street subject to time restrictions. The speed limit of Thomas Street is 40km/h.

3.1.2 Jones Street

Jones Street is a local road and in the vicinity of the site aligned in a north-south direction. It is a no through road with its connection to Broadway closed. It is a 12.8m wide, two-way road configured with one lane in each direction, set within a 21m wide road reserve (approx.). Typically, kerbside parking is permitted on both sides of Jones Street subject to time restrictions, however, currently Jones Street is closed to the public and is being used as a Works Zone for the construction of UTS Building 2. As part of the construction works, Jones Street has been temporarily given restricted vehicular access from Broadway for construction vehicles outside of the road network peak periods.

3.1.3 Harris Street

Harris Street is a classified State Road (MR170) and in the vicinity of the site is aligned in a north-south direction. It is configured as two-way north of Thomas Street and one-way southbound south of Thomas Street with five lanes of traffic. Near the site, kerbside parking is not permitted with No Stopping restrictions and clearway restrictions from 6:00am-10:00 and 3:00pm-7:00pm Monday to Friday. The speed limit is posted as 50km/h.

3.1.4 Wattle Street

Wattle Street is a classified State Road (MR594) and in the vicinity of the site is aligned in a north-south direction. It is a one-way northbound road configured with a four-lane, 14m wide carriageway, set within a 24m wide road reserve (approx.). North of Thomas Street, kerbside parking is permitted on both sides of Wattle Street, subject to clearway and time restrictions. Wattle Street carries approximately 24,100 vehicles per day². The speed limit is posted as 50km/h.

² Based on Roads and Maritime Services (RMS) 2009 Annual Average Daily Traffic Data (AADT)



3.1.5 Broadway

Broadway is a State Road (HW5) in the vicinity of the site and is aligned in an east-west direction. It is a two-way road configured with four lanes in each direction (including one bus lane in each direction) and functions as one of the main routes for traffic into and out of the Sydney CBD. Broadway carries approximately 36,000 vehicles per day³.

3.2 Public Transport

The site is well serviced by high frequency public transport with Central Station Transport Interchange, a key transport hub in Sydney located 500m east of the site.

3.2.1 Bus Network

The subject site is located in close proximity to several key bus corridors including Broadway along the southern boundary of Building 1 and 2. Central Transport Interchange features five main bus hubs at Railway Square (200m east of the site), George Street, Eddy Avenue and Chalmers Street which serve destinations across the Sydney Metropolitan Area including Sydney's south, eastern suburbs, inner-west, northern beaches and north-west.

The nearest bus stop to the site is located immediately in front of Building 1 on Broadway. This stop is served by 18 services within the Sydney Buses network and is a major inbound stop in the area. Corresponding outbound bus services from UTS is available from Railway Square, which is approximately 200m to the east of Building 1.

The Sydney bus network continues to grow with the number of available bus services and routes gradually increasing as the demand for public transport grows. Since the approved UTS Concept Plan in 2009, a number of new bus routes and services have been introduced and existing bus routes have been amended. This includes the introduction of several Metrobus services including the M30 between Mosman and Sydenham which serves the UTS site.

A comparison on bus frequencies reported in the 2009 Concept Plan and current bus frequencies of bus stop on Broadway, nearest to the UTS site, is summarised in Table 3.1.

Period	Inbound	Outbound	Total		
AM Peak (8:00am-9:00am)					
2008 Broadway	116	71	187		
2018 Broadway	130	78	208		

Table 3.1: Bus Frequency Comparison of Bus Stops on Broadway

³ TCS 416 Traffic Count Data 02 August 2018



PM Peak (5:00pm-6:00pm)					
2008 Broadway 64 99 163					
2018 Broadway	71	101	172		

Source: 2008 information from Halcrow (2009), 2018 information from <u>www.transportnsw.info</u>, accessed August 2018

3.2.2 Rail Network

Central Railway Station Central serves as the key rail hub in Sydney for CityRail services to destinations across the Sydney Metropolitan Area, the Illawarra, Blue Mountains and Central Coast. Central Station is also the hub for interstate rail services in Sydney. Central Station is also the main terminus for the Central to Lilyfield Light Rail Network.

Since the approved Concept Plan in 2009, the rail network has expanded with the South West rail link recently opened for service, and the North West Rail Link is currently under construction. The expansions provide a wider reach for public transport availability in wider Sydney.

Table 3.2 summarises the frequency of rail services to and from Central Station.

	AM Peak		PM P	eak
Rail Line	To City	From City	To City	From City
T1 North Shore Line	19	20	20	16
T1 Northern Line	9	6	6	7
T1 Western Line	25	21	8	23
T2 Inner West and Leppington Line	18	16	10	16
T3 Bankstown Line	14	10	6	12
T4 Eastern Suburbs & Illawarra Line	18	18	16	18
T8 Airport & South Line	14	8	9	14
Blue Mountains Line	4	1	2	4
Central Coast & Newcastle Line	8	2	2	7
South Coast Line	4	1	3	4
Southern Highlands Line	4	1	1	2

Table 3.2: Frequency of Rail Services

Source: <u>www.transportnsw.info</u>, accessed August 2018



3.2.3 Future Public Transport Network

Access to the Sydney CBD will be enhanced through future public transport systems including the Sydney Metro and Sydney Light Rail. These are expected to increase capacity for patrons accessing the Sydney CBD as well as improve travel times as a result of increased services during peak periods. Central Station which is currently highly-utilised by people accessing UTS, will enjoy access to such services.

3.2.3.1 Sydney Metro

The New South Wales (NSW) Government is implementing Sydney's Rail Future, a plan to transform and modernise Sydney's rail network so that it can grow with the city's population and meet the needs of customers in the future (Transport for NSW, 2012). Sydney Metro is a new standalone rail network identified in Sydney's Rail Future.

Sydney Metro is Australia's biggest public transport project, consisting of Sydney Metro Northwest (Stage 1), which is scheduled for completion in 2019 and Sydney Metro City & Southwest (Stage 2), which is scheduled for completion in 2024.

Sydney Metro West is expected to be operational in the late 2020s.

Stage 2 of Sydney Metro includes the construction and operation of a new metro rail line from Chatswood, under Sydney Harbour through Sydney's CBD to Sydenham and on to Bankstown through the conversion of the existing line to metro standards.

The proposed Sydney Metro network map is shown in Figure 3.1.





Figure 3.1: Sydney Metro Network Map

The project also involves the delivery of eight new metro stations, including at Central. Once completed, Sydney Metro will have the ultimate capacity for 30 trains an hour (one every two minutes) through the CBD in each direction - a level of service never seen before in Sydney.

3.2.3.2 CBD and South East Light Rail

The CBD and South East Light Rail will function as a new light rail network featuring 19 stops between Circular Quay, Kingsford and Randwick via Central Station on a 12km route.

Key features of the new light rail route include reliable and high-capacity services available every four minutes during peak periods and additional services between Central and the Moore Park and Alison Road stops during special events.

3.3 Pedestrian Infrastructure

The pedestrian network surrounding the site is well established with pedestrian paths located on both sides of the surrounding roads.

Source: Sydneymetro.info, accessed August 2018



Safe crossing points in vicinity of the site include the following pedestrian crossings:

- two pedestrian crossings at the frontage of Building 2 of the Chippendale Way/ Broadway intersection,
- all legs of the George Street/ Harris Street/ Regent Street intersection.
- Within the Campus, UTS has proposed and implemented several pedestrian links as part of the original City Campus Masterplan to enable the connection of key hubs across its City Campus. The Masterplan included the plan to close Jones Street to vehicular traffic from Broadway, which has been implemented. The pedestrian network that was proposed as part of the original City Campus Masterplan is shown in Figure 3.2.

Figure 3.2: UTS Campus Hubs and Pedestrian Network



Source: UTS City Campus Masterplan, 2008

3.4 Cycle Infrastructure

The site is located within close proximity to both on and off-road cycling facilities as indicated in an extract from the City of Sydney's cycle network map shown in Figure 2.4. The nearest dedicated cycle facility surrounding the site is the 4m wide shared path along Jones Street between Thomas Street and Mary Ann Street, which is identified as a part of a 'Regional



Cycle Route' as defined in Transport for NSW's *Sydney's Cycling Future*. Jones Street is also part of the newly completed CoS initiative, the Broadway Link, which provides walkers and cyclists a safe corridor between Darlington and Ultimo.



Figure 3.3: Cycle Network

Source: Sydney Cycleways, accessed August 2018

Bicycle parking facilities are provided throughout the UTS City Campus with the nearest facilities to the site located at:

- Building 10 car park (entrance located at the corner of Thomas Street and Jones Street)
- Multi-Purpose Sports Hall.

The Building 10 car park currently accommodates 288 bicycle parking spaces, 260 lockers, 14 toilets and 28 male and female showers. These bicycle parking spaces are an initiative of the original Concept Plan to provide staged increases in bicycle parking provision at UTS.



Figure 3.4: Building 10 Bicycle Parking



Source: GTA Consultants, 2017

3.5 Car Sharing Pods

Car sharing is a flexible, cost effective alternative to car ownership and is a convenient and reliable way for residents to use a car when they need one. GoGet and Flexicar are car share companies operated in Australia with a number of vehicles positioned within the area.

Car share is a concept by which members join a car ownership club, choose a rate plan and pay an annual fee. The fees cover fuel, insurance, maintenance, and cleaning. The vehicles are mostly sedans, but also include SUVs, station wagons and vans. Each vehicle has a home location, referred to as a "pod", either in a parking lot or on a street, typically in a highly-populated urban neighbourhood. Members reserve a car by web, telephone and use a key card to access the vehicle.

The locations of car sharing pods in the vicinity of the site are shown in Figure 3.5.





Figure 3.5: Car Sharing Pods

Source: City of Sydney, accessed August 2018

3.6 Traffic Surveys

3.6.1 Vehicle Volumes

Intersections are typically the critical locations in the road network, due to the need for opposing movements to occupy the same space. To quantify existing intersection conditions, a program of peak period intersection turning movement surveys were commissioned by TTPP at the intersections of:

- Harris Street-Ultimo Road,
- Harris Street-Thomas Street,
- George Street-Broadway-Harris Street-Regent Street,
- Wattle Street-Broadway-Abercrombie Street, and
- Wattle Street-Thomas Street.

The surveys were conducted between 7:00am and 10:00am, and between 3:30pm and 6:30pm on Thursday 2 August 2018. The network peak hours have been revealed to be 7:45am to 8:45am and 5:00pm to 6:00pm.



The turning movement volumes for the AM and PM peak periods are shown in Figure 3.6 with detailed results presented in Appendix A.

Figure 3.6: Existing Traffic Volumes (127) 134----(2297) 2149----2 - 71 (155) - 237 (516) ULTIMO ROAD 21 (36) EXISTING PEAK VOLUMES (TOTAL) 10 (10) = AM (PM) AM PEAK = 7:45 - 8:45 PM PEAK = 17:00 - 18:00 (140) 177 (36) 22 -(109) 62 -BIVE - 118 (116) THOMAS STREET 176 (120) 176 (120) 2053 (2008) (233) 139 (1517) 1461 (868) 468 (926) 1295 (587) 814-(1033) 1366 -2 Ī -4 (16) -989 (1834) - 481 (1024) - 29 (22) e BROADWAY GEORGE STREET 48 (56) 1326 (1410) 67 (123) ABERCROMBIE STREET REGENT STREET

3.6.2 Pedestrian Volumes

Pedestrian movement counts were conducted between 7:00am and 5:00pm on Thursday 2 August 2018 at the following key locations to identify existing pedestrian volumes in the vicinity of the subject site:



- Informal mid-block crossing movements across Harris Street, between Thomas Street and Broadway.
- Signalised crossing at the northern leg of the Harris Street/ Broadway/ George Street/ Regent Street intersection.
- Signalised crossing at the western leg of the Harris Street/ Broadway/ George Street/ Regent Street intersection signalised crossing.
- Pedestrian movements around the north-western corner of Broadway and Harris Street (not crossing at the adjoining roads).

The movements included in the pedestrian survey is shown in Figure 3.7.



Figure 3.7: Pedestrian Survey Location

Basemap source: Google Maps Australia

The daily profile of surveyed pedestrian flows is summarised in Figure 3.8 with detailed results presented in Appendix A.





Figure 3.8: Existing Pedestrian Flows

The peak pedestrian volumes (pedestrians per hour) for each movement are as follows:

- Midblock Harris St AM: 643, PM: 792
 North Leg Crossing AM: 2,334, PM: 2,624
 West Leg Crossing AM: 551, PM: 783
- Broadway-Harris St corner
 AM: 206, PM: 332

Figure 3.8 indicates the pedestrian volumes are very high along Broadway at Harris Street. This suggests that the majority of people accessing UTS do so from the east, with the Central Station and Railway Parade interchanges being the major attraction.

3.7 On-street Parking Survey

A parking survey was conducted between 7:00am and 4:00pm Thursday 2 August 2018 to identify the inventory and capacity of on-street car parking spaces in the vicinity of the site. The survey was undertaken at the following locations:

- Mary Ann Street between Wattle Street and Harris Street,
- Harris Street between Mary Ann Street and Thomas Street,
- Thomas Street,
- Wattle Street between Mary Ann Street and Broadway, and
- Regent Street.

The on-street parking capacity during the survey period is shown in Figure 3.9.





Figure 3.9: On-street Car Parking Capacity (Excluding Regent Street)

Figure 3.9 indicates 137 car parking spaces are available at the surveyed locations. This is reduced to 120 spaces during clearways between 6:00am to 10:00am and 3:00pm to 7:00pm Monday to Friday on the west side of Wattle Street.

The peak parking demand occupied between 11:00am and 1:00pm with 128 spaces of the spaces occupied. During this period, nine car parking spaces remained vacant.

The on-street parking capacity for Regent Street during the survey period is shown in Figure 3.6.



Figure 3.10: On-Street Parking Capacity (Regent Street Only)



Figure 3.10 indicates 12 car parking spaces are available at the surveyed location. This parking area includes 'no parking coaches excepted' between 3:00pm-7:00pm Monday-Friday and loading zones between 7:00am-3:00pm Monday-Friday and 7:00am-10:00am Saturday.

3.7.1 UTS Building 10 Car Park Boom Gate Data

Boom gate data from Wednesday 1 August 2018 was obtained to determine the number of entry and exit movements to the existing Building 10 car park to give a profile of cars arriving at the campus.

The boom gate data is summarised in Figure 3.11.



Figure 3.11: UTS Building 10 Car Park Access Volumes

The data indicates a peak volume of 89 vehicles entering and exiting the car park between 8:00am and 9:00am and 79 vehicles per hour between 5:00pm and 6:00pm. The peak volumes occur during the road network peak period.

3.7.2 Loading Dock Surveys

To review the existing loading demand of the existing site, loading dock surveys were undertaken at two locations between the hours of 7:00am and 5:00pm:

The Building 1 (B1), Level 2 basement loading dock accessed from Thomas Street.
 The B1 loading dock contains some 23 marked bays and two informal loading areas. The B1 loading is a general loading area used by a number of services within the main campus.



 Turner Lane loading area accessed from Harris Street. Turner Lane includes three formal loading bays and room for one informal parking space. It generally services the food court located in Level 3 of B1.

A summary of the cumulative loading demand of B1 loading dock is shown in Figure 3.12.



Figure 3.12: B1 Loading Dock Demand

Figure 3.12 indicates the B1 loading dock has a peak demand of 16 vehicles at any one time however, only for a five-minute period at 10:10am. Otherwise, there is a peak demand of around 15 vehicles and an average demand of around 10 vehicles.

Approximately 20 per cent of the above parking activity is generated by construction contractors undertaking temporary works in Building 1.

Based on the above, during the peak occupancy the loading dock is approximately 50 per cent. However, it should be noted that typically, the B1 loading dock is also used by Building 2 which has been demolished for redevelopment and is currently under construction.

Turner Lane is recorded to have relatively low usage with five vehicles accessing the laneway over the span of the survey period and containing a peak demand of two vehicles which occurred over a five-minute period. However, it is noted that one retail tenancy in Building 3, fronting Harris Street (previously a café) is currently vacant and was anticipated to have undertaken loading from Turner Lane.

3.8 Existing Intersection Operation

The operating characteristics of the surveyed intersections have been assessed using SIDRA INTERSECTION 8, an analysis program which determines characteristics of intersection

operating conditions including the degree of saturation, average delays, and levels of service. The degree of saturation, or x-value, is the ratio of the arrival rate of vehicles to the capacity. The average delay, expressed in seconds per vehicle, is measured over all movements at signalised intersections, and over the movement with the highest average delay at roundabout and priority intersections. Average vehicle delay is the commonly used measure of intersection performance defined by RMS. Table 2.3 shows the criteria adopted by RMS for assessing the level of service.

Level of Service (LoS)	Average Delay per vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Sign
A	Less than 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Near capacity	Near capacity, accident study required
E	57 to 70	At capacity, at signals incidents will cause excessive delays	At capacity, requires other control mode
F	Greater than 70	Extra capacity required	Extreme delay, major treatment required

Table 3.3: Intersection Level of Service Criteria

Table 3.4 presents a summary of the existing peak hour operating characteristics of the surveyed intersections with detailed SIDRA outputs contained in Appendix B.

		AM Peak			PM Peak		
Intersection	Approach	Average Delay	LoS	95 th Percentile Queue	Average Delay	LoS	95 th Percentile Queue
	North	1	А	34	8	А	158
Harris St-	East	53	D	48	49	D	114
Ultimo Rd	South	11	А	8	19	В	16
	Overall	7	А	48	17	В	158
	North	1	А	17	3	А	87
Harris St- Thomas St	West	52	D	36	50	D	58
	Overall	3	А	36	5	А	87
	North	32	С	183	28	В	212
Broadway- George St-	East	20	В	39	25	В	75
Harris St- Regent St	West	21	В	90	28	В	82
Kegeni si	Overall	26	В	183	28	В	212
Broadway- Wattle St- Abercrombie St	East	13	А	68	19	В	222
	West	31	С	424	31	С	222
	South	41	С	146	36	С	149

Table 3.4: Existing Intersection Operation


	Overall	30	С	424	28	В	222
Wattle St-	East	52	D	31	37	С	23
Thomas St	South	5	А	0	5	А	0

Based on Table 3.4, it is evident that the intersections currently operate satisfactorily with acceptable delays, notwithstanding notable queueing on some approaches in particular during the afternoon peak period. A review of the Harris Street intersections indicate that queues are long in the afternoon peak period along Harris Street, but minimal delays are experienced with Harris Street traffic given priority in the road network. Ultimately the side streets to Harris Street experience notably delays with Thomas Street and Ultimo Road both nearing capacity with a LoS D.

3.9 Footpath Capacity Assessment

An assessment of footpath capacity and performance has been undertaken along Harris Street and Broadway fronting the site.

3.9.1 Assessment Methodology

In order to determine whether capacity is available for existing pedestrian demands while maintaining safety and convenience for pedestrians in the vicinity of the proposed development, TPPP has undertaken the footpath capacity assessment through Fruin Theory⁴ to obtain the level of service (LOS) of the area and evaluation of the pedestrian capacity. This approach is utilised in the '*Transit Capacity and Quality of Service Manual – 2nd Edition – part 7⁵*'.

Assessment of pedestrian LOS involves determining the pedestrian flow rate which is measured in pedestrians per metre per minute and represents the number of pedestrians that pass a point in a specific timeframe. Although there is no guideline for the specific minimum criteria to which footpaths are designed against, LOS C is generally satisfactory for footpaths in popular pedestrian areas.

The criteria of assessment in Fruin Theory however has been updated based on the latest guidance as determined by 'Transport for London'. Figure 3.13 and Table 3.5 provide a quantitative representation for assessing the footpath capacity level of service and the associated level of service criteria respectively.

⁴ Fruin, John J. 1987 Pedestrian Planning and Design – Revised Edition

⁵ Transportation Research Board 2003 Transit Capacity and Quality of Service Manual – 2nd Edition – Part 7



Figure 3.13: Footpath Capacity Criteria



Source: TFL, 2010 as reproduced in Pantzar. M, 2012, Pedestrian Level of Service and Trip Generation, University of Melbourne, Australia.



Table 3.5: Footpath Capacity Criteria

	Flow rate (ped/min/m)		
Level of Service (LoS)	From	То	
A	0	8	
В	9	17	
С	18	26	
D	27	35	
E	36	36+	

Source: Transport for London

Based on the footpath capacity criteria portrayed in Table 3.5, the Harris Street and Broadway footpaths at the north-west corner of the intersection of Harris Street, Broadway and George Street has been undertaken. The results are summarised in Table 3.6.

Table 3.6: Existing Footpath Capacity

Site	Harris Street	Broadway path
Peak pedestrians/ 15min	376	767
Peak 15min period	13:00-13:15	14:45-15:00
Pedestrians /min	25	51
Footpath Width*	2	3
Effective Footpath Width*	1.5	2.5
Pedestrians/minute/metre	17	20
LOS	В	С

*Footpath widths are an approximation determined using aerial photography. Effective width is the available space between physical obstructions on the footway

Table 3.6 indicates that the footpaths at the north-west corner of Harris Street and Broadway are currently operating between a LOS B and LOS C during peak pedestrian volume activity.

This indicates that the footpaths have available capacity though pedestrians would experience some conflict from opposing movements. In addition, visual inspection of the site indicates that these footpaths are congested due to pedestrians stopping at intersections where they have to congregate therefore affecting pedestrian flow. Noting that the Harris Street-Broadway corner includes a number of conflicting movements due to pedestrians travelling in various directions from the intersection crossings, the actual capacity of the footpath is anticipated to be lower than is calculated above. Indeed, site observations indicate pedestrian congestion at the corner of Broadway and Harris Street.



3.10 Travel Mode Share

2016 census data shows the existing Journey to Work patterns of people working in the areas in and around the UTS Campus. Destination Zone 113341158 contains the Broadway precinct of the UTS Campus and also the TAFE located adjacent to UTS as shown in Figure 2.7. The travel mode split of the Destination Zone 113341158 is summarised in Table 3.7.



Figure 3.14: Destination Zone 113341158

Source: Australian Bureau of Statistics, accessed August 2018

Table 3.7: Journey to Work Comparison

Mode	2011 Mode Share	2016 Mode Share
Vehicle Driver	23%	15%
Vehicle Passenger	3%	2%
Train	42%	50%
Bus	18%	18%
Walked	8%	9%



Other ⁶	4%	6%
Mode not stated	2%	1%
Total	100%	100%

Data Source: 2011 data from GTA Consultants 'UTS Broadway Precinct-UTS Central Transport Impact Assessment, 2016 data from Australian Bureau of Statistics, accessed August 2018

The data presented in Table 3.7 relates to employees working in the Destination Zone 113341158. Due to the nature of the data (i.e. journey to work data from the census), it is unlikely to include the travel behaviour of students.

UTS had previously conducted travel behaviour surveys of their staff and students. This data was presented in the UTS' Sustainable Transport Plan 2016-2017.

The data presented in the travel plan provides separated modal split for staff and students at UTS and is summarised in Table 3.8.

Mode	Staff	Students
Car	19%	7%
Train	43%	52%
Bus	2%	23.5%
Walk	10%	11%
Cycle	6%	6%
Other	1%	0.5%
Total	100%	100%

Table 3.8: UTS Student and Staff Mode Share

Source: UTS Sustainable Transport Plan 2016-2017

The modal share split for staff as presented in the travel plan is comparable with the Census data in Table 2.3, where 17% of people drive or are vehicle passengers, 50% catch a train, 18% catch a bus, compared to 19% of drivers, 43% of train and 22% of bus users.

The modal share for students suggests that there is a far greater percentage of students travelling by non-car modes of transport with only 7% driving to the university, 52% catching a train, and 24% catching a bus.

⁶ Includes ferry, taxi, truck, motorbike/scooter/ bicycle, did not go to work modes



4 Development Proposal

4.1 Concept Development

The proposed redevelopment of the Bon Marche and Science Precinct aims to improve the pedestrian connectivity and accessibility of the UTS City Campus and address issues relating to parking, traffic and pedestrian movements.

On this basis, the following opportunities are being considered for the redevelopment:

- Formal set-down/pick up parking restrictions on Thomas Street to address existing issues relating to drivers stopping on the carriageway to drop-off passengers to UTS
- New basement staff car park with access via the existing driveway located on Thomas Street.
- Reaccommodate Turner Lane loading activities to the Building 1 loading dock and remove Turner Lane.
- Widening of the pedestrian crossing at the Harris Street approach of the Harris Street, Broadway, George Street and Regent Street intersection to address existing concerns relating to pedestrian overflowing off the crossing.
- A new mid-block pedestrian crossing along Harris Street between Thomas Street and Broadway to accommodate existing informal pedestrian crossing activity and anticipated future growth in pedestrian volumes. As discussed in Section 3.6.1, up to 323 pedestrians per hour are crossing informally along Harris Street.
- Widening of the footpath on Harris Street along the frontage of the UTS site to improve the pedestrian accessibility and amenity of the area. Implementation of this opportunity would however involve narrowing Harris Street and thereby losing one lane of traffic. Harris Street on approach to Broadway currently includes five lanes of traffic including one shared left turn and through lane, two through only lanes, one shared right turn and through lane and one right only lane.

TTPP have assessed the above opportunities and their impact to the road network and on-site and on-street parking in Section 5 and Section 6.

4.2 Campus Population

The original Concept Plan had planned facilities and services to accommodate an expectant student load of 15,000 EFTSL (Equivalent Full Time Student Load) at the UTS City Campus by 2015.

However, student growth has well exceeded this figure and modifications to the concept plan to date have looked to accommodate this growth. As of 2017, the student load at the City Campus has been noted as 36,422 EFTSL including 25,467 EFTSL at the Broadway



Campus, which is approximately 60 per cent greater than expected for the original concept plan.

On this basis, UTS are currently developing projections on population growth and have estimated a student load of 32,358 EFTSL at Broadway by the 2028. The indicative student loading projects are summarised in Table 4.1.

	-		
Year	Broadway	Haymarket	Total
2017	25,467	10,955	36,422
2020	30,505	9,648	40,153
2028	32,358	10,033	42,391

Table 4.1: Campus Population Projections (under development)

The anticipated growth in student population and subsequent increase to staff numbers necessitates and justifies the proposed increase to the site's floor area, provide more parking for staff and improving pedestrian connections and amenity around and within the site.

4.3 Proposed Vehicle Access and Parking

The development proposes to provide a new basement car park containing up to 150 car spaces. Access to the car park would be via the existing driveway on Thomas Street which currently provides access to the B1 loading dock and existing 50 space staff car park in the basement. A new vehicular link will be created from the existing basement loading dock to the proposed car park under Building 4 (the development site). An indicative layout of the proposal is shown in Figure 4.1.







4.4 Loading Arrangements

As part of the proposal a portion of the B1 loading dock will be reconfigured. While the proposed layout is indicative, it is anticipated that eight small loading bays would be converted to four larger bays and would seek to accommodate existing loading activities from B1, Building 2 which is currently under construction and Turner Lane as well as loading activity from the development site i.e. redeveloped Bon Marche and Science precinct.

It is understood that currently waste collection and deliveries to the Building 1 Student Bar, Level 3 food court, the Loft bar and Building 3 are made via Turner Lane. Survey data indicates that Turner Lane generates around four vehicles per day.

With an occupancy of 50 per cent of the existing loading dock, Turner Lane loading demand is anticipated to be easily accommodated within the B1 loading dock. However, it should be noted that the typical loading demand generated by Building 2 before its demolition or the loading demand is not accounted for in the survey data for the Building 1 loading dock. In addition, the loading requirements of the currently vacant retail tenancy on Harris Street is not accounted for on Turner Lane.

Notwithstanding this, the small café is not anticipated to generate a substantial demand for loading through the week. Building 2, while it has been demolished, the student and staff population for the UTS Campus has not reduced, therefore it is gathered that the loading



requirements for the UTS Campus have not been likely to have reduced to any great extent as a result of the redevelopment of Building 2.

In addition, the future Building 2 redevelopment includes a small expansion of the B1 loading dock to the west which will add an additional five loading bays. This would likely accommodate the loading requirements of the new Building 2 food court. However, this is understood to replace the existing Level 3 food court in Building 1 which is serviced from Turner Lane and therefore a portion of the existing Turner Lane loading demand is already approved to be accommodated within the Building 1 loading dock.

The remaining servicing demand as generated by the Loft Bar, Student Bar, Building 3 and the currently vacant retail tenancy is anticipated to increase the demand marginally.

Based on the above, the Building 1 loading dock is believed to be appropriate to accommodate the loading requirements of the future site.



5 Traffic and Transport Assessment

5.1 Traffic Generation

The proposed development includes the provision of a new basement car park containing up to 150 car spaces for use by staff members of UTS. To determine the vehicle generating potential of the proposed car park, boom gate data from the existing car park located at UTS Building 10 has been obtained.

The existing Building 10 car park contains 328 car parking spaces. Boom gate data indicates the car park generates a peak generation of 89 trips per hour (86 in/ 3 out) and 79 trips per hour (20 in/ 59 out) in the morning and afternoon peak periods respectively. On this basis, the car park generates the following trip generation rates:

- Morning peak: 0.27 trips per space, and
- Afternoon peak: 0.24 trips per space.

The proposed development car park is anticipated to generate a similar level of traffic to Building 10. Based on the above rates, it is anticipated that up to a 150-space car park would generate up to 41 trips per hour in the morning peak and 36 trips per hour in the afternoon peak.

The estimated traffic to be generated by the proposed development is minor resulting in approximately one to two vehicles per minute on adjacent roads which currently carry over 2,000 vehicles per hour. On this basis, the proposed development traffic is anticipated to have a negligible impact on the road network. Notwithstanding this, SIDRA modelling has been undertaken with consideration for the additional 36 to 41 trips per hour, which has been distributed into the road network based on existing traffic flow distributions.

5.2 Traffic Impact

SIDRA modelling has been undertaken to not only assess the impact of the proposed development traffic but also to assess the proposed changes to the road network to improve pedestrian amenity as discussed in Section 4. The modelled scenarios involve an assessment of each proposed pedestrian amenity improvement and an assessment of the combined impact of all proposed changes. The assessment focuses on the critical afternoon peak period. The modelled scenarios are as follows.

Post Development

Existing conditions plus development traffic.

Post Development plus Harris Street Lane Reduction to accommodate wider footpath

Post development conditions plus the proposed narrowing of Harris Street to accommodate a wider pedestrian footpath along the UTS frontage.



Post Development plus Midblock Pedestrian Crossing

Post development conditions plus a new mid-block signalised pedestrian crossing along Harris Street between Thomas Street and Broadway.

Post Development plus Widened Intersection Crossing

Post development conditions plus widening of the pedestrian crossing on Harris Street at its intersection with Broadway which would subsequently result in setting back the stop line

Post Development plus All Pedestrian Amenity Improvements

Post development conditions plus all of the above alterations to the road network including narrowing of Harris Street, midblock pedestrian crossing on Harris Street, and widening of the intersection crossing.

The SIDRA modelling results are presented in the following with detailed SIDRA outputs contained in Appendix B.

5.2.1 Post Development

Table 5.1 presents a summary of post development operating conditions of the studied intersections with the addition of 36 vehicle trips per hour.

Intersection	PM Peak			
mersection	Average Delay	LoS	95 th Percentile Queue	
Harris St-Ultimo Rd	22	В	204	
Harris St-Thomas St	6	А	93	
Broadway-George St-Harris St- Regent St	28	В	215	
Broadway-Wattle St-Abercrombie St	28	В	228	
Wattle St-Thomas St	43	D	29	

Table 5.1: Post Development Intersection Operation

Based on the above, the addition of development traffic on to the road network would have a negligible impact on the road network with delays and queues generally consistent with those of the existing scenario as presented in Table 3.4. The notable exception includes the Thomas Street approach to Wattle Street which would increase in delay by six seconds, reducing the approach from a LoS C to a LoS D. However, the remaining road network is noted to continue as per existing.

5.2.2 Post Development + Harris Street Narrowing

The proposal to widen the Harris Street footpath along UTS would require the narrowing of Harris Street and subsequent reduction in the number of traffic lanes from five to four lanes between Thomas Street and Broadway. The proposal results in a reduction in the capacity of Harris Street, with the afternoon peak period being notably impacted which currently experiences long queues under existing conditions.

Inevitably, the loss of a lane would increase delays and queuing along Harris Street.

However, SIDRA modelling indicates that acceptable levels of intersection performance could be maintained by the dynamic reallocation of green time to the Harris Street approach from the Broadway and George Street approaches, which would be achieved within the minimum and maximum variable times provided at the existing intersection.

Table 5.2 presents results of the post development scenario with Harris Street narrowed to four lanes by the removal of one through-only lane.

Intersection	Proposed PM Peak			
intersection	Average Delay	LoS	95 th Percentile Queue	
Harris St-Ultimo Rd	20	В	207	
Harris St-Thomas St	4	А	69	
Broadway-George St-Harris St- Regent St	31	С	256	
Broadway-Wattle St-Abercrombie St	28	В	227	
Wattle St-Thomas St	43	D	29	

Table 5.2: Post Development + Road Narrowing

A comparison with Table 3.4 (Existing Conditions) and Table 5.1 (Post Development) indicates that the proposed road configuration would continue to operate acceptably with minor increases to delay and queueing.

A sensitivity analysis has been undertaken of the model to assess the impact of removing kerbside parking along Regent Street, south of Broadway. Notably in the existing scenario, Harris Street includes five lanes (including four lanes permitting the through movement) however the shared left turn and through lane is very underutilised by through traffic due the presence of kerbside parking downstream. Therefore, in the existing scenario, there are effectively three lanes of through traffic. A sensitivity test indicates that the removal of parking along Regent Street would act to increase capacity on the left-turn/through lane on the Harris Street approach, with the impact of removing a through lane being relatively minor. The results indicate that the intersection and road network would continue to operate similar to existing conditions with the removal of a through traffic lane, if the capacity of Regent Street is increases through the removal of parking. However, noting that the kerbside parking on Regent Street includes a combination of special parking restrictions including coach parking, loading zones and mail zones with no clear ways, the removal of this parking may be a concern to the adjoining properties that utilise it.

5.2.3 Development + Midblock Pedestrian Crossing

The network model has been updated with a signalised midblock pedestrian crossing along Harris Street located half way between Thomas Street and Broadway. The pedestrian phase has been allowed to run once every cycle. It is envisaged that the pedestrian crossing phase would be able to run during the red signal period of Harris Street at Broadway and it would therefore have minimal impact to the timing of Harris Street intersections.

The results of the model are presented in Table 5.3.

Intersection	PM Peak			
intersection	Average Delay	LoS	95 th Percentile Queue	
Harris St-Ultimo Rd	15	В	128	
Harris St-Thomas St	5	А	64	
Midblock Crossing	1	А	23	
Broadway-George St-Harris St- Regent St	28	В	139	
Broadway-Wattle St-Abercrombie St	28	В	227	
Wattle St-Thomas St	43	D	29	

Table 5.3: Post Development + Pedestrian Crossing

A comparison with Table 3.4 (Existing Conditions) and Table 5.1 (Post Development) indicates that the proposed road configuration would continue to operate acceptably. It is noted that on some approaches, queueing appears to have improved in this post development scenario. The new road network layout with the proposed crossing alters the SIDRA network calculation which in turn improves the queues along Harris Street and Ultimo Road.

5.2.4 Development + Widened Crossing at Harris Street

For the purposes of this assessment, it has been assumed that the existing intersection crossing would be widened to be 10-metre in width. Currently it is seven metres at the eastern end and four metres at its western end as shown in Figure 5.1.



Figure 5.1: Harris Street Approach Crossing

A proposed widening of the crossing on the Harris Street approach of the Harris Street and Broadway intersection would result in vehicles needing to travel an additional three to six metres to cross the intersection. It is estimated that this would equate to an approximate vehicle delay of some 0.5-1 second per signal cycle based on a vehicle travel speed of 25km/h or 7m/s.

SIDRA modelling of this delay indicates that the resulting intersection would require one second signal timing to be shifted from the Broadway and George Street approaches to the Harris Street approach. This is anticipated to have a negligible impact to the road network as indicated by the SIDRA results in Table 5.4.

Intersection	PM Peak			
mersection	Average Delay	LoS	95 th Percentile Queue	
Harris St-Ultimo Rd	22	В	204	
Harris St-Thomas St	6	А	93	
Broadway-George St-Harris St- Regent St	28	В	215	
Broadway-Wattle St-Abercrombie St	28	В	227	
Wattle St-Thomas St	43	D	29	

Table 5.4: Post Development + Widened Crossing at Harris St

A comparison with Table 3.4 (Existing Conditions) and Table 5.1 (Post Development) indicates that the proposed road configuration would continue to operate acceptably with minor increases to delay and queueing.



5.2.5 Development + All Pedestrian Amenity Improvements

A combination of all the above changes indicate that there would be a significant increase to queueing and delays. However, SIDRA modelling indicates that acceptable levels of intersection performance could be maintained by the dynamic reallocation of green time to the Harris Street approach from the Broadway and George Street approaches, which would be achieved within the minimum and maximum variable times provided at the existing intersection. The change in phase times would be the same as the Post Development plus Road Narrowing scenario which includes a shift of some 13 seconds.

The results of the SIDRA model are summarised in Table 5.5.

had a second second	PM Peak			
Intersection	Average Delay	LoS	95 th Percentile Queue	
Harris St-Ultimo Rd	15	В	119	
Harris St-Thomas St	56	D	69	
Midblock Crossing	5	А	45	
Broadway-George St-Harris St- Regent St	30	С	139	
Broadway-Wattle St-Abercrombie St	28	В	227	
Wattle St-Thomas St	43	D	29	

Table 5.5: Post Development + All Amenity Improvements

5.3 Service Vehicles Generation

The proposed development is expected to generate relatively low volume of service vehicle movements. While the precinct will include a significantly larger floor area than the existing building on the development site, loading and delivery activities are not anticipated to increase significantly from the existing situation as it is to be maintained as a primarily educational facility.

The B1 loading dock generates a peak of some 30 two-way trips per hour in the morning peak hour and services many uses including mail room deliveries, construction contractor parking, general staff parking and shuttle buses.

The proposed development site would comparatively generate a far lower increase in service vehicle traffic.

However, it is proposed that a Loading Dock Management Plan be provided to manage the loading dock. This is common at a number of city centre sites to ensure that loading operation can be managed throughout the day so as not to result in concurrent loading which in turn could lead to operational issues.



5.4 Public Transport Capacity

5.4.1 Non-Car Trip Generation

An estimate of the number of non-car trips has been calculated based on available mode share data of students.

As per Table 4.1, the Broadway campus population is expected increase by some 6,891 EFTSL between 2017 and 2028. The portion of this volume to be accommodated within the development site (Bon Marche and Science precinct) cannot be accurately established at this stage.

As such, the entire Broadway campus trip generation characteristics have been calculated based on the student travel mode share survey results summarised in Table 3.8. Noting that no future parking is proposed for students in the Campus concept plan, and the limited availability of public parking in the surrounding area, no additional vehicle generation is anticipated from student travel.

On this basis the existing travel mode share has been modified to account for a zero per cent of car travel, and the future non-car mode trip generation has been estimated as is detailed in Table 5.6.

Mode	Current Student Mode Share	Modified Student Mode Share	Future Additional Trips
Car	7%	0%	0
Train	52%	56%	3,859
Bus	23.5%	25%	1,723
Cycle	6%	7%	482
Walk	11%	12%	827
Other	0.5%	0%	0
Total	100%	100%	6,891

Table 5.6: Non-Car Trip Generation

Table 5.6 indicates that the development would generate around 3,859 train, 1,723 bus and 1,309 cycling or walking trips.

The proposed Broadway Campus trip generation would however be distributed over the week with student timetables varying considerably. However, for the purposes of the following assessment it is assumed that the additional trip generation would be generated over a day therefore 3,859 train and 1,723 bus trips per day.



5.4.2 Future Public Transport Capacity

Sydney is undergoing significant upgrades to its public transport capacity with the CBD and South East Light (CSELR) and Sydney Metro both under construction with CSELR expected to be complete by 2019 and Sydney Metro by 2024.

It is understood that the Sydney metro would increase rail capacity for an additional 100,000 customers per hour or more⁷ across the Sydney CBD rail lines. The CSELR is anticipated to carry a capacity of up to 13,500 passengers per hour.

The proposed improvement to light rail and heavy rail capacity would additionally relieve capacity of bus services with some customer demand anticipated to displace on to the future rail services.

On this basis, the future trip generation of the Broadway Campus with a total of 5,582 train and bus trips per day would equate to less than five per cent of the CSELR capacity and less than one per cent of the Sydney Metro capacity.

5.5 Pedestrian Traffic Implications

5.5.1 Pedestrian Crash History

Historical crash data has been sourced from Roads and Maritime for the five-year period to 31 December 2017. The crash data indicates that in the five-year period, there were a total of nine crashes on Harris Street and a total of 23 crashes occurring within 50m of the intersection on Broadway, George Street and Regent Street.

5.5.1.1 Harris Street

The crash data indicates that in the five-year period, there were a total of nine crashes on Harris Street, between Thomas Street and Broadway. Of these nine crashes, four crashes involved pedestrian with no fatalities recorded.

⁷ Sydney Metro Chatswood to Sydenham EIS



	Non	Pedestrian Cra	ishes	Pe	edestrian Crash	es	
Year	Fatality	Injury	Non- casualty	Fatality	Injury	Non- casualty	All Crashes
2013	0	1	1	0	0	0	2
2014	0	0	0	0	1	0	1
2015	0	1	0	0	0	0	1
2016	0	0	0	0	3	0	3
2017	0	2	0	0	0	0	2
Total	0	4	1	0	4	0	9

Table 5.7: Harris Street Crash Data

Analysis of the crash data indicates that approximately two of the crashes involving pedestrians is attributed to a pedestrian being hit by a vehicle travelling on the far side lane and one crash is attributed to a pedestrian being hit by a vehicle on the near side lane. The other pedestrian crash is related to a vehicle colliding with a pedestrian walking along the footpath while the vehicle entered the driveway.

While no fatalities have resulted, the above data presents a risk especially as pedestrian volumes are anticipated to grow in the surrounding areas from the development of UTS and also from general growth and development in the surrounding areas. As noted in Section 3.6.2, Harris Street currently includes up to 792 pedestrians per hour crossing midblock informally along Harris Street. In the road network peak hours, there is an informal crossing volume of up to 233 and 620 pedestrians per hour in the morning and afternoon peak periods respectively.

The pedestrian volume data indicates a heavy demand for crossing midblock on Harris Street, and justifies investigation into the feasibility of providing a midblock crossing at this location. Notably, the Roads and Maritime Services warrants for a 'signalised midblock marked foot crossing' is met, which requires over 250 persons per hour crossing the road for four separate one-hour periods of an average day.

5.5.1.2 Broadway, George Street and Regent Street

The crash data indicates that in the five-year period, there were a total of 23 crashes occurring within 50m of the intersection on Broadway, George Street and Regent Street. Of these 23 crashes, 12 crashes involved pedestrian with no fatalities recorded.



	No	n-Pedestria	n Crashes	Peo	hes		
Year	Fatality	Injury	Non-casualty	Fatality	Injury	Non-casualty	All Crashes
2013	0	4	0	0	1	0	5
2014	0	1	1	0	4	0	6
2015	0	2	1	0	2	0	5
2016	0	2	0	0	2	0	4
2017	0	0	0	0	3	0	3
Total	0	9	2	0	12	0	23

Table 5.8. Broadway/	' George Street/ Regent Street Crash Data ⁸
Table 5.0. Diodaway/	Ocorge street Regent street of ash bata

Analysis of the crash data indicates that six of the crashes involving pedestrians is attributed to a pedestrian being hit by a vehicle travelling on the far side lane and three crashes are related to a pedestrian being hit by a vehicle travelling on the nearest lane. The data indicates that there is notable number of crashes at this intersection, but it does not appear to be concentrated at one location but is spread across all corners of the intersection.

5.5.2 Effects of Additional Pedestrian Trips

As indicated in Section 5.4.1, the city campus population is anticipated to grow by an additional 6,891 EFTSL. It anticipated that a majority of these trips would be pedestrian trips to and from transport modes, in particular from the Central Station interchange from the east.

Noting that up to 2,600 pedestrians per hour walk to and from the east via the Harris Street intersection, the additional trips would likely increase congestion on this approach.

It is noted that there are alternative routes to travel to the UTS city Campus without using the Harris Street intersection including the pedestrian bridge over Harris Street via the Ultimo Pedestrian walkway. However, the route via the Harris Street intersection is clearly the most popular route. On this basis, opportunities to improve the capacity of this crossing and to potentially encourage pedestrians to use other routes should be considered.

The detailed design of the Bon Marche and Science precinct will therefore look to improving the pedestrian connectivity and accessibility to and around the UTS City Campus.

The existing pedestrian connections to the UTS City Campus is shown in Figure 5.2.

⁸ Does not include crashes along Harris Street





Figure 5.2: Existing Pedestrian Connections

Source: <u>https://www.uts.edu.au/partners-and-community/initiatives/city-campus-master-plan/campus-development-news/2018-news/get</u>, accessed 28/08/2018

5.6 Green Travel Plan

As required by City of Sydney's general requirements for development consent, a green travel plan is to be prepared to promote sustainable travel. A green travel plan applicable to students, staff and visitors travelling to site would be prepared for the detailed DA and implemented upon occupation of the precinct. The key objective of this green travel plan would be to:

- Identify the existing travel behaviour and mode share of the Campus
- Identify initiatives to encourage sustainable transport modes
- Identify a methodology to monitor the implementation of the green travel following occupation of the precinct
- Set targets to measure the success of initiatives implemented in the green travel plan.

5.7 Construction Traffic Management Plan

A Construction Traffic Management Plan (CTMP) is to be prepared and submitted to City of Sydney for approval. The CTMP will provide further details on the construction activities and



their impacts. At this preliminary stage the construction details are unknown. However, the CTMP should address the following key items:

- Works Zone location notably for the other UTS building sites, a Works Zone has been required on-road. Consideration would need to be given if a Works Zone would be required on Thomas Street and/or Harris Street. Similar to other UTS development sites, if access is required to an arterial road i.e. Harris Street, it is envisaged that time restrictions would be applied to enable operation of any Works Zones outside of traffic peak periods only.
- Construction vehicle size, access and swept path analysis.
- A pedestrian and cyclist access management plan noting the heavy volumes of pedestrians in the area.
- Cumulative impacts from other Construction activities occurring in the surrounding areas.



6 Parking Assessment

6.1 Car Parking Requirement

The parking requirement for an educational facility is stipulated in the City of Sydney Council's Sydney Local Environmental Plan 2012 (LEP). The LEP requires parking for education facilities to be provided at a maximum parking provision rate of one space per 200m².

Therefore, the proposed development with a total maximum floor area of some 65,000m² has a maximum permissible parking requirement of 325 spaces.

The proposed parking provision of 150 spaces for staff is therefore compliant with Council's maximum permissible requirements.

The proposal does not include any parking for students. This consistent with the principles of the approved Concept Plan, which focuses on sustainable transport modes for students. The site is able to take advantage of its location with its proximity to existing transport hubs including Central Railway Station and the Railway Parade bus interchange and to future public transport infrastructure promotes a sustainable travel culture. On this basis, the aim to provide zero parking for students is appropriate.

6.2 Proposed Set-down/Pick up Parking

Currently there is a notable demand for set-down and pick-up activity along Thomas Street. There is a high demand for on-street parking along Thomas Street and therefore limited kerbside parking space is available, drivers tend to stop in the traffic lanes thereby blocking through traffic along Thomas Street.

The behaviour identifies a need to accommodate this activity more formally. On this basis, as part of the proposed development, UTS is considering a proposal to convert around two to four on-street car parking spaces to 5-min parking restrictions for set-down/pick-up activity. The proposal would require displacement of two on-street parking spaces with two-hour ticketed time restrictions along Thomas Street fronting the development site.

Parking demand surveys of the local nearby on-street parking indicates a high parking demand of 93 per cent occupancy but it does reveal nine spaces are vacant even during the busiest times even during the peak parking demand period. On this basis, the displacement of two parking spaces could be accommodated within the existing on-street parking supply.



6.3 Bicycle Parking Requirement

The UTS City Campus Bicycle Parking Strategy submitted to the Department of Planning and Environment in 2014, has indicated a proposed bicycle parking requirement of 1,008 spaces over the City Campus, based on Council's DCP rate of 1 space per 10 students and staff.

UTS has been gradually increasing the bicycle parking supply on Campus based on regular monitoring of bike parking demand on Campus. With a proposed increase to the Campus population, UTS is revising its Bike Parking Strategy for the entire Campus. Historical monitoring of the Campus bike parking will enable accurate understanding of the bicycle parking demand of students and staff on Campus based on population.

On this basis, the bicycle parking requirement for the Bon Marche and Science precinct will not be addressed in this report with a Campus wide strategy to be prepared and submitted as part of the detailed DA.



7 Summary and Conclusions

The development proposal seeks approval to for the construction of the Bon Marche and Science Precinct within the existing UTS Broadway Precinct. The main works involved with the proposal include the full and partial demolition of existing buildings (Building 4 and rear section of Building 3) and the creation of a new building envelope for Building 4 and Building 3. The key elements of the proposal are as follows:

- The proposed development includes establishing new building envelopes with corresponding height and Gross Floor Area (GFA) to the Bon Marche and Science Precinct (Buildings, 3, 4, 9 and 18).
- All other buildings within the campus shall be maintained.
- It is proposed to provide for up to 150 car parking spaces for staff, within a future potential new basement carpark to serve the proposed development. This is in accordance with the maximum permissible parking requirement stipulated by City of Sydney Council's Sydney Local Environmental Plan 2012 (LEP).
- Parking for students is not proposed and is consistent with the principles of the approved Concept Plan, which focuses on sustainable transport modes for students.
- Future transport capacity is anticipated to cater for the increase in public transport demand associated with the increase in student intake. Notably, the Sydney Metro and future improvements to light rail and heavy rail facilities shall provide additional rail capacity and relieve demands on existing bus services with some customer demand anticipated to displace on to the future rail services.
- Vehicle access to the proposed basement car park shall be via the existing access from Thomas Street. A new vehicular link will be provided from the existing B1 loading dock access to the car park located in the basement of the new Building 4.
- The proposed development is expected to generate an additional 41 vehicle movements per hour and 36 vehicle movements per peak hour in the morning and evening peak periods respectively. SIDRA modelling analysis of post development conditions indicates the additional trips associated with the development would have negligible impact on the road network. The exception is the Thomas Street approach to Wattle Street which would increase in delay by six seconds, reducing the approach from a LoS C to a LoS D. However, Wattle Street and the remaining road network is not anticipated to be impacted.
- Several opportunities are being investigated with an aim to improve the pedestrian amenity and connectivity of the local area. These opportunities include
 - widening of the Harris Street footpath and associated narrowing of Harris Street carriageway (loss of one traffic lane)
 - provision of a midblock crossing along Harris Street between Thomas Street and Broadway



- widening of the pedestrian crossing at the Harris Street approach to Broadway.
- SIDRA modelling of the above opportunities indicate that the above modifications are feasible noting that some dynamic reallocation of green time to the Harris Street approach from the Broadway and George Street approaches would be required. However this is would be achieved within the minimum and maximum variable times provided at the existing intersection.



Appendix A

Traffic surveys

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N4366 1119 1115 Survey 131 Harris Survey 1311 January 1311 January 1311 August 2018 1311 1311 Canter Section Count 1310 Urb Surmeny	Harris St	8 x x x x x x x x x x x x x x x x x x x	Direction 7 Direction 8 (Through) (Left Turn)	ensure ensure constr constr<
bob No. : M4396 Client : TTPP Suburb : UTSsurvey Location : 3. Harris St. Uttino Rd Location : 3. Harris St. Uttino Rd Location : 3. Harris St. Uttino Rd Deyr/Date : Thu, 2nd August 2018 Weather : Fine Description : Classified Intersection C	Approach		Direction Direction (Le	Time Period Lun Picol

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			Crossing Pedestrians	A B C D C F F 9 1 3 2 0 3 7 7 9 1 3 3 3 3 3 7 7 9 1 3 3 3 3 3 7 7 7 1 3 4 3 3 3 3 7 7 1 3 4 3
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:M396 :TTPP :UT55unwey :4. Watte Sr, Thomas St :Thu, 2nd August 2018 :Fhre :Classified Intersection Count :Hourd Summary	Wa	Interdint Interdint 1	Direction 7 Direction 8	IutoT o
Job No. 11 Clent 11 Suburb 12 Location 4 Day/Date 17 Weather 16 Description 10	Approach	Direction 7:00 10 7:10 10 7:10 10 7:10 10 7:10 10 7:10 10 7:10 10 7:10 10 7:10 10 8:10 10 8:10 10 8:10 10 9:10 10 9:11 10 9:12 10 9:13 10 9:14 10 9:15 10 9:12 10 9:13 10 9:14 10 9:15 10 9:10 10 9:10 10 9:10 10 9:10 10 9:10 10 9:10 10 9:10 10 9:10 10 9:10 10	Direction	

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		Direction 4 (Left Turn)	sipru			o o o		•		• •		• •	0 0	•		Direction 10 (Left Turn)	syonuj	48	8	31	33 33		11	6	7 8	2 00	5	5 7	52
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Broadway			leto			• • •	• •	• •		• •		• •		•		F	letol	• •	•	• •	0 0 0		+	•	。。	, .	• •	• •	
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		Direction 3U (U Turn)	sipru			o o o	• •	•	• •	• •	• •	• •		۰		Direction 9U (U Turn)	Lucks		0	• •	• • •		• •	•	• •		• •	• •	•
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			leto	22 28 28	48 52	8 8 5	65	139	52	8 9	4 4	42	: * *	150			letol	• •	•	• •	• • •	• • •	• •	•	• •	, .	• •	• •	•
		lon 3 Turn)	səsn			0 "1 "	1	-	• •	• •	• •			1		ion 9 Turn)	səsng	• •	•	• •	• • •		• •	•	• •	0	• •	• •	•
		Direction 3 (Right Turn)	sypru			5 e e	3	12		3		• •		3		Direction 9 (Right Turn)	syonuj		0	• •	o o c	• • •	• •	• •	• •		• •	• •	۰
	mbie St		jars	17 23 23	41	12	61	126	46	35	39	41 SS	8 8 8	146	tle St		sueg	0 0	0	• •	• • •	• • •	• •	•	• •	• •	• •	• •	۰
	Abercrombie St		leto	1,348	1,326	1,300	1,182	3,823	1,323	1,343	1,435	1,449	1,413	4,071	Wattle St		letol	• •	0	• •	• • •	• • •	• •	0	• •		• •	• •	۰
		Direction 2 (Through)	səsn	1 1 1	3 2	4 00 Ş	п	28		s v	n s	3 2	3	01		Direction 8 (Through)	səsng	0 0	•	• •	• • •		• •	•	• •	• •	• •	• •	۰
bie St		Diree (Thr	sipru	1 8 7 9		36 47			8	R R		18	1 I I	12		Direc (Thr	syonuj	• •	•	• •	• • •	• • •	• •	•	• •	• •	• •	• •	۰
eccom			jars	1,235	1,269	1,260	1,095	3,615	1,283	1,309	1,406	1,429	1,397	3,988			sueg	• •	•	• •			• •	•	• •	0	• •	• •	۰
: M436 : TTP P : UTS Survey : S. Wattle S./ Broadway / Abercrombe St Martie S./ Broadway / Abercrombe St Im. 2nd August 2018 : Classified Intersection Count : Hine : Classified Intersection Count : Hourdy Sammary			leto	1 4 8 2	67	72	+	206	74	82	+	111	141	308			letol	• •	•	• •	• • •	• • •	• •	•	• •	•	• •	• •	۰
/ Broad		Direction 1 (Left Turn)	səsn	• • •	• • •	• • •	•	•	•	• •	•	• •	0 0	۰		Direction 7 (Left Turn)	səsng	0 0	•	• •	• • •	• • •	• •	•	• •	• •	• •	• •	۰
: N4396 : TTPP : UTS Survey : UTS Survey : S. Wattle St / Broadwa : S. Wattle St / Broadwa : Sture : Thu 2nd August 2018 : Fine : Classifie of Intersection : Hourity Summary		Dire (Lef	sypnu			- 0 -		-	5	2		~ ~		9		Dire (Lef	Lrucks	• •	•	• •	• • •		• •	•	• •	0	• •	• •	۰
: N4396 : TTPP : UTS Sur : 5. Watt : 5. Watt : 5. Watt : 1. Au : Thu, 2n : Thu, 2n : Thu : 1. Au : Hourly (sue;			5 72 0 72 5 50		194 7		00 80 81		45 108 00 120		302			sueg	0			0 0 0				00 0		0 0	15 0 30 0	
Job No. Client Suburb Location Day/Date Weather Description	Approach	Direction	Time Period	7:00 to 8:00 7:15 to 8:15 7:20 to 0:12	7:45 to 8:45 8:00 to 9:00	8:15 to 9:15 8:30 to 9:30	9:00 to 10:00	AM Totals	15.45 to 16.45	16:00 to 17:00		16.45 to 17.45 17.00 to 18.00		PM T otals	Approach	Direction	Time Period	7:00 to 8:00	8		8:15 to 9:15 8:30 to 9:30 8:45 to 9:45		AIVI IOUNIS 15:30 to 16:30	15.45 to 16.45	16:00 to 17:00 16:15 to 17:15	16:30 10 17:30	16.45 to 17.45 17.00 to 18.00	17:15 to 18:15 17:30 to 18:30	PM Totals

Client	TTPP
Date	Thu, 2nd August 2018
Description	UTS Parking Survey





Location	1. Mary ann St												1
Date	Thu, 2nd August 2018										4 4 7	CD I	1
	· •									IV	(A)	ſRJ	Х
Description	UTS Parking Survey										Traf	fic and fran	sport Data
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Mary ann St_North Side													
Wattle St													
	No Stopping												
	Loading Zone	7am-6pm (Mon-Fri) & 7am-10am (Sat)											
	1P Ticket	6pm-9pm (Mon-Fri) & 10am-9pm (Sat-Sun) Public holidays permit holders excepted area 20	3	0	0	0	1	1	2	2	2	1	0
	No Stopping												
Wattle Ln													
	No Stopping												
	1P Ticket	10am-9pm Permit holders excepted area 20	2	1	2	2	2	2	2	2	2	2	2
	No Stopping												
Mckee St													
	No Stopping												
	1P Ticket	10am-9pm Permit holders excepted area 20	5	4	5	5	3	4	5	5	5	5	5
	No Parking							2 Bikes	2 Bikes				
	P	Motorbikes only	11	11	11	11	11	11	11	11	11	11	11
	1P Ticket	10am-9pm Permit holders excepted area 20	11	9	10	11	11	11	10	10	10	9	9
	No Parking	Authorised car share vehicles excepted zone H24/7	1	1	1	1	1	1	1	1	1	1	1
Duluan Dal	No Stopping												
Bulwara Rd	No Changing												
	No Stopping	40-m Oran Dennik helden som sted over 20	5	4	4	4	4	5	5	5	5	5	5
	1P Ticket No Stopping	10am-9pm Permit holders excepted area 20	5	4	4	4	4	5	5	5	5	5	5
Hackett St	No stopping												
Hackett St	No Stopping												
Harris St	No Stopping												
Harris Sc		Total	38	30	33	34	33	35	36	36	36	34	33
		% Capacity		79%	87%	89%	87%	92%	95%	95%	95%	89%	87%
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Mary ann St_South Side													
Harris St													
	No Stopping												
	No Parking												
	2P Ticket	8am-7pm permit holders excepted area 20 60' angle parking rear to kerb vehicles under 6m only	13	8	10	12	13	13	13	12	13	12	12
	No Parking												
	2P Ticket	8am-7pm permit holders excepted area 20 60' angle parking rear to kerb vehicles under 6m only	15	6	12	15	15	15	15	15	15	15	15
	No Parking												
	Р	Disable only	2	0	0	1	2	2	2	2	2	2	2
	No Parking												ļ
	2P Ticket	8am-7pm permit holders excepted area 20 60' angle parking rear to kerb vehicles under 6m only	22	5	17	22	22	22	21	20	20	21	20
	No Parking	Authorised car share vehicles excepted Zone GG	1	1	1	1	1	1	1	1	1	1	1
Wattle St													
		Total	53	20	40	51	53	53	52	50	51	51	50
		% Capacity		38%	75%	96%	100%	100%	98%	94%	96%	96%	94%

Client

TTPP

Client	TTPP												
Location	2. Harris St												1
Date	Thu, 2nd August 2018									N	1 47	FRI	V
Description	UTS Parking Survey									1		fic and Tran	sport Data
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Harris St_East Side													
Mary ann St													
	No Stopping & Clearway	CW - 6am-10am, 3pm-7pm (Mon-Fri)											
Ultimo Rd													
	No Stopping												
	No Parking & Clearway	CW - 6am-10am, 3pm-7pm (Mon-Fri)											
	No Stopping & Clearway	CW - 6am-10am, 3pm-7pm (Mon-Fri)											
Opposite of Harris St													
		Total	0	0	0	0	0	0	0	0	0	0	0
		% Capacity		0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Harris St_West Side													
Thomas St													
	No Stopping												
	Mail Zone		1	0	0	0	0	0	0	0	0	0	0
	No Stopping												
	Bus Zone & Clearway	CW - 3pm-7pm (Mon-Fri)											
	No Stopping												
Mary ann St													
		Total	1	0	0	0	0	0	0	0	0	0	0
		% Capacity		0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Location	3. Thomas St												7
Date	Thu, 2nd August 2018									N	1 17	FRI	V
Description	UTS Parking Survey									1.	Traf	flic and Tran	sport Data
•	<u> </u>												
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Thomas St_North Side													
Wattle St				L									
	No Stopping												
	Bus Zone			L									
	2P Ticket	8am-7pm	3	3	3	3	3	3	3	3	3	3	3
	No Parking	Authorised car share vehicles excepted Zone GG	1	1	1	1	1	1	1	1	1	1	1
	No Parking												
Jones St													
	No Parking												
	2P Ticket	8am-7pm	7	7	7	7	7	7	7	7	7	7	7
	1/4P & 2P Ticket	8am-7pm (Mon-Fri) 8am-7pm (Sun&Public holidays)	2	1	1	2	2	2	2	2	2	2	2
	No Parking												
	No Stopping												
	2P Ticket	8am-7pm	5	2	2	2	3	5	5	5	5	4	4
	No Parking												
	Loading Zone & 2P Ticket	8am-7pm (Mon-Fri) 8am-7pm (Sun&Public holidays)	2	1	1	1	2	2	2	2	2	1	1
	No Stopping												
Harris St													
		Total	20	15	15	16	18	20	20	20	20	18	18
	% Capacity			75%	75%	80%	90%	100%	100%	100%	100%	90%	90%
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Thomas St_ South Side													
Harris St													
	No Stopping												
	Loading Zone	8am-7pm (Mon-Fri)	2	0	1	2	2	1	2	2	1	1	0
	2P Ticket	8am-7pm	5	4	4	5	5	5	5	5	5	5	5
	No Stopping												
	No Parking												
	No Parking	7am-10pm (Mon-Fri) Motorbikes excepted	95	44	65	80	93	95	95	95	91	84	86
Jones St													
	No Stopping												
	2P Ticket		9	5	7	9	7	7	7	7	7	8	8
	No Stopping								1			1	1
Wattle St													
		Total	111	53	77	96	107	108	109	109	104	98	99
		% Capacity	1	48%		86%			98%			88%	89%

Client

TTPP

Client	TTPP												
Location	4. Wattle St												7
Date	Thu, 2nd August 2018									N	1A1	D I	V
Description	UTS Parking Survey									IV	Traf	fic and Tran	sport Data
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Wattle St_East Side													
Mary ann St													
	No Stopping												
	2P Ticket	8am-7pm	4	1	2	4	4	3	3	4	4	4	4
	No Stopping & No Parking	NS - 6am-10am / 3pm-8pm (Mon-Fri)											
	No Stopping												
Thomas St													
	No Stopping												
Broadway		Total											
		4	1	2	4	4	3	3	4	4	4	4	
	% Capacity			25%	50%	100%	100%	75%	75%	100%	100%	100%	100%
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Wattle St_West Side													
Broadway													
	No Stopping												
Wattle Pl													
	No Stopping												
	No Parking & Clearway	CW - 6am-10am / 3pm-7pm (Mon-Fri)											
	1P & Clearway	1P - 10am-3pm (Mon-Fri) CW - 6am-10am / 3pm-7pm (Mon-Fri)	17	0	0	0	14	15	14	13	7	0	0
House No.43			17										
	Total			0	0	0	14	15	14	13	7	0	0
		% Capacity		0%	0%	0%	82%	88%	82%	76%	41%	0%	0%
Client	TTPP												
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Location	5. Regent St												1
Date	Thu, 2nd August 2018									N	1AT	D	V
Description	UTS Parking Survey									1	Traf	fic and Tran	sport Data
	• /												
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Regent St_East Side													
George St													
	No Stopping												
	Mail Zone		1	0	0	0	0	0	0	0	0	0	0
	No Parking & Loading Zone & 4P Ticket	3pm-7pm (Mon-Fri) Coaches excepted 7am-3pm (Mon-Fri) & 7am-10am(Sat) 7pm-10pm (Mon-Fri) & 10am-10pm (Sat) & 8am- 10pm (Sun&Public holiday)	2	1	2	2	2	2	2	2	1	0	0
	No Parking							1					
	No Parking & Loading Zone & 4P Ticket	3pm-7pm (Mon-Fri) Coaches excepted 7am-3pm (Mon-Fri) & 7am-10am(Sat) 7pm-10pm (Mon-Fri) & 10am-10pm (Sat) & 8am- 10pm (Sun&Public holiday)	5	0	3	4	5	2	1	1	1	2	2
	No Parking	3pm-7pm (Mon-Fri) Coaches excepted	5	0	1	0	0	2	1	0	1	2	3
	No Stopping												
Lee St													
		Total	13	1	6	6	7	7	4	3	3	4	5
		% Capacity		8%	46%	46%	54%	54%	31%	23%	23%	31%	38%
Side of the Street	Parking Restriction	Time Restrictions	Available Spaces	7:00	8:00	9:00	10:00	11:00	12:00	13:00	14:00	15:00	16:00
Regent St_West Side													
Regent St													
	No Stopping												
	Work Zone & No Stopping	WZ - 10am-3pm (Mon-Fri) & 8am-1pm (Sat)	3	0	0	0	0	0	0	0	0	0	0
	No Stopping												
Goold St													
	No Stopping												
George St													
		Total	3	0	0	0	0	0	0	0	0	0	0
		% Capacity		0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Traffic and Transpor	Dedation 1. Harris St, between Thomas St and Broadway 2. Harris St / Broadway Pedestrians Direction	
	Additional and a string and a string and a string a service a string a string a service a string a st	ttle R
Thu, 2nd August 2018 07:00-17:00 (10hrs) UTS Ped Survey	The Apprentice Restaurant Sydnay Tronnas St Tronnas St Tronnas St ActivateFin on Hat ActivateFin on Hat Branching ActivateFin on Hat NativateFin on Hat ActivateFin on Hat	
Date Survey Time Description	A HAN	

Real

PIO

Car

sport Data

TTPP Harris St & Broadway

Client Location

Client TTPP

Description

Location1. Harris St, between Thomas St and BroadwayDateThu, 2nd August 2018Survey Time07:00-17:00 (10hrs)

UTS Ped Survey



[15mins interval]			
1. H	arris St, between Th	nomas St and Broad	way
Time Periods	Eastbound	Westbound	Total
7:00 to 7:15	4	7	11
7:15 to 7:30	11	16	27
7:30 to 7:45	11	21	32
7:45 to 8:00	16	43	59
8:00 to 8:15	16	25	41
8:15 to 8:30	22	29	51
8:30 to 8:45	30	52	82
8:45 to 9:00	43	95	138
9:00 to 9:15	32	57	89
9:15 to 9:30	34	43	77
9:30 to 9:45	23	40	63
9:45 to 10:00	53	51	104
10:00 to 10:15	42	36	78
10:15 to 10:30	35	26	61
10:30 to 10:45	69	73	142
10:45 to 11:00	85	98	183
11:00 to 11:15	50	74	124
11:15 to 11:30	74	32	106
11:30 to 11:45	87	67	154
11:45 to 12:00	109	82	191
12:00 to 12:15	111	81	192
12:15 to 12:30	90	73	163
12:30 to 12:45	97	59	156
12:45 to 13:00	143	123	266
13:00 to 13:15	135	72	207
13:15 to 13:30	86	48	134
13:30 to 13:45	115	55	170
13:45 to 14:00	129	82	211
14:00 to 14:15	89	53	142
14:15 to 14:30	80	53	133
14:30 to 14:45	113	77	190
14:45 to 15:00	94	84	178
15:00 to 15:15	110	61	171
15:15 to 15:30	87	34	121
15:30 to 15:45	118	31	149
15:45 to 16:00	168	74	242
16:00 to 16:15	114	58	172
16:15 to 16:30	96	40	136
16:30 to 16:45	80	40	120
16:45 to 17:00	131	61	192
Totals	3,032	2,226	5,258

[Hourly Sur	nmary]			
	1. Ha	rris St, between Th	omas St and Broad	way
Time Per	riods	Eastbound	Westbound	Total
7:00 to	8:00	42	87	129
7:15 to	8:15	54	105	159
7:30 to	8:30	65	118	183
7:45 to	8:45	84	149	233
8:00 to	9:00	111	201	312
8:15 to	9:15	127	233	360
8:30 to	9:30	139	247	<mark>3</mark> 86
8:45 to	9:45	132	2 <mark>35</mark>	367
9:00 to	10:00	142	191	333
9:15 to	10:15	152	170	322
9:30 to	10:30	153	153	306
9:45 to	10:45	199	186	3 <mark>85</mark>
10:00 to	11:00	231	233	464
10:15 to	11:15	2 <mark>3</mark> 9	271	510
10:30 to	11:30	278	277	555
10:45 to	11:45	296	271	567
11:00 to	12:00	320	2\$5	575
11:15 to	12:15	381	262	643
11:30 to	12:30	397	303	700
11:45 to	12:45	407	295	702
12:00 to	13:00	441	336	777
12:15 to	13:15	465	327	792
12:30 to	13:30	461	302	763
12:45 to	13:45	479	298	777
13:00 to	14:00	465	257	722
13:15 to	14:15	419	238	657
13:30 to	14:30	413	243	656
13:45 to	14:45	411	265	676
14:00 to	15:00	376	267	643
14:15 to	15:15	397	275	672
14:30 to	15:30	404	256	660
14:45 to	15:45	409	210	619
15:00 to	16:00	483	200	683
15:15 to	16:15	487	197	684
15:30 to	16:30	496	203	699
15:45 to	16:45	458	212	670
16:00 to	17:00	421	199	620
Tota	ls	3,032	2,226	5,258

[Peak Hour Summary]

	1. H	arris St, between Th	omas St and Broad	way
	Time Periods	Eastbound	Westbound	Total
AM	11:00 to 12:00	320	255	575
PM	12:15 to 13:15	465	327	792

Client Location Date Survey Time Description

TTPP 2. Harris St / Broadway Thu, 2nd August 2018 07:00-17:00 (10hrs) UTS Ped Survey

MATRIX

				2.	2. Harris St / Broadway	oadway											2. Harris St / Broadway	Broadway					
Time Periods	1	2	3	4	5	9	7	8	6	10	Total	Time Periods	÷	2	3	4	5	9	7	8	6	10	Total
7:00 to 7:15	2	0	48	33	8	4	11	0	9	6	121	7:00 to 8:00	20 5	15	414	131	99	21	129	9	45	19	851
7:15 to 7:30	0	0	56	29	12	6	14	0	9	0	126	7:15 to 8:15	15 📘 13	26	550	165	62	23	170	6	59	15	1,109
7:30 to 7:45	0	5	122	32	18	9	45	2	17	e	250	7:30 to 8:30		47	730	239	102	29	224	14	44	17	1,495
7:45 to 8:00	3	10	188	37	28	2	59	4	16	7	354	7:45 to 8:45		62	970	288	145	23	301	16	86	22	1,983
8:00 to 8:15	10	11	184	67	21	9	52	m	20	ъ	379	8:00 to 9:00	31	76	1, 265	336	217	65	467	16	145	23	2,641
8:15 to 8:30	2	21	236	103	35	15	68	2	25	2	512	8:15 to 9:15		91	1,407	355	249	65	551	21	173	34	2,972
8:30 to 8:45	13	20	362	81	61	30	122	4	37		738	8:30 to 9:30		86	1,412	331	270	69	554	24	168	39	2,997
8:45 to 9:00	9	24	483	85	100	14	225	4	63	80	1,012	8:45 to 9:45	45 31	97	1, 323	337	251	52	487	23	163	42	2,806
9:00 to 9:15	ŝ	26	326	86	53	9	136	80	48	16	710	9:00 to 10:00		92	1,106	372	200	67	352	27	124	38	2,413
9:15 to 9:30	∞	28	241	79	56	19	71	~	20	7	537	9:15 to 10:15		81	1,040	384	177	85	251	23	93	28	2,204
9:30 to 9:45	12	19	273	87	42	13	55	m	32	11	547	9:30 to 10:30		99	1,046	397	142	97	199	27	93	22	2,145
9:45 to 10:00	10	19	266	120	49	29	90	~	24	4	619	9:45 to 10:45		51	1,063	460	124	128	180	59	73	33	2,238
10:00 to 10:15	12	15	260	98	30	24	35	4	17	9	501	10:00 to 11:00		48	1,086	488	116	127	123	79	84	48	2,279
10:15 to 10:30	28	7	247	92	21	31	19	12	20	1	478	10:15 to 11:15		43	1,078	547	115	134	111	83	86	60	2,345
10:30 to 10:45	17	10	290	150	24	44	36	35	12	22	640	10:30 to 11:30	30 82	28	1,005	663	127	137	118	87	68	86	2,452
10:45 to 11:00	29	16	289	148	41	28	33	28	29	19	660	10:45 to 11:45		62	696	739	136	145	110	69	96	70	2,475
11:00 to 11:15	14	10	252	157	29	31	23	~~	25	18	567	11:00 to 12:00	00 83	67	202	825	136	193	129	68	85	55	2,548
11:15 to 11:30	22	22	174	208	33	34	26	16	23	27	585	11:15 to 12:15		93	893	917	156	268	137	68	87	40	2,793
11:30 to 11:45	14	14	254	226	33	52	28	17	19	9	663	11:30 to 12:30	30 128	95	944	989	192	326	139	87	84	20	2,998
11:45 to 12:00	33	21	2.27	234	41	76	52	27	18	4	733	11:45 to 12:45		129	1,014	1,015	242	370	154	38	67	16	3,242
12:00 to 12:15	44	36	238	249	49	106	31	29	27	m	812	12:00 to 13:00		177	1,096	1,063	285	380	164	100	83	13	3,503
12:15 to 12:30	37	24	2.25	280	69	92	28	14	14	7	790	12:15 to 13:15	15 128	195	1,125	1,114	315	383	198	110	99	19	3,653
12:30 to 12:45	23	48	324	252	83	96	43	28	8	2	907	12:30 to 13:30		200	1,142	1,107	333	364	215	125	69	17	3,704
12:45 to 13:00	38	69	3 09	282	84	86	62	29	34	1	994	12:45 to 13:45		174	1,119	1,165	306	339	216	124	89	23	3,690
13:00 to 13:15	30	54	267	300	79	109	65	39	10	9	962	13:00 to 14:00		140	1,063	1,162	295	333	183	119	70	32	3,522
13:15 to 13:30	41	29	2.42	273	87	73	45	29	17	5	841	13:15 to 14:15		112	1,016	1,180	274	302	147	113	\$1	26	3,384
13:30 to 13:45	26	22	301	310	56	71	44	27	28	~	893	13:30 to 14:30		109	966	1,179	237	285	115	101	84	23	3,210
13:45 to 14:00	28	35	253	279	73	80	29	24	15	10	826	13:45 to 14:45		411	926	1,167	224	274	116	110	72	17	3,142
14:00 to 14:15	38	26	2.20	318	58	78	29	33	21	3	824	14:00 to 15:00	00 133	111	915	1,318	203	269	118	122	76	12	3,277
14:15 to 14:30	25	26	192	272	50	56	13	17	14	2	667	14:15 to 15:15		110	941	1,287	180	271	89	89	70	14	3,187
14:30 to 14:45	28	30	261	298	43	60	45	36	22	2	825	to		113	924	1,313	167	279	76	72	71	25	3,181
14:45 to 15:00	42	29	2.42	430	52	75	31	36	19	5	961	14:45 to 15:45		112	874	1,299	167	305	31	36	61	27	3,047
15:00 to 15:15	41	25	2.46	287	35	80	26	51	15	5	734	15:00 to 16:00	124	126	838	1,148	168	306	0	0	64	33	2,807
15:15 to 15:30	30	29	175	298	37	64	12	29	15	13	661	15:15 to 16:15		123	790	1,229	167	303	21	35	73	46	2,902
15:30 to 15:45	22	29	211	284	43	86	22	51	12	4	691	15:30 to 16:30	30 121	120	830	1,285	175	324	40	76	74	53	3,098
15:45 to 16:00	31	43	206	279	53	76	33	45	22	11	721	15:45 to 16:45		122	869	1,346	161	325	65	134	74	57	3,290
16:00 to 16:15	32	22	198	368	34	77	21	35	24	18	829	16:00 to 17:00		135	904	1,405	156	358	94	197	69	51	3,512
16:15 to 16:30	36	26	215	354	45	85	19	41	16	20	857	Totals	907	987	9,594	8,248	1,842	2,119	1,759	734	839	324	27,353
16:30 to 16:45	38	31	250	345	29	87	25	58	12		883												
16:45 to 17:00	37	56	241	338	48	109	29	63	17		943												
							110		000		01010												

Z. HarrissY. Broadway Time Periods 1 2 3 4 5 6 7 8:30 10 9:30 32 98 1,412 331 270 69 554 12:30 to 132 200 1,142 1,142 333 364 215		8 9 10 Total	24 168 39 2,997	125 69 17 3,704	
Time Periods 1 2 3 4 8:30 to<9:30		7	554	215	
Time Periods 1 2 3 4 8:30 to<9:30	Broadway	9	69	364	
Time Periods 1 2 3 4 8:30 to<9:30	2. Harris St / I	5	270	333	
Time Periods 1 2 8:30 to 9:30 32 98 1 12:30 to 13:30 132 200 1		4	331	1,107	
Time Periods 1 8:30 to 92 12:30 to 132		3	1,412	1, 142	
Time Periods 8:30 to 9:30 12:30 to 13:30		2	86	200	
Time Peri 8:30 to 12:30 to		÷	32	132	
P A		<u>n</u>	þ	12:30 to	



Appendix B

SIDRA Outputs

Site: 101 [Broadway-George St-Harris St-Regent St Ex AM]

♦ Network: N101 [Ex AM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	t Perform	ance ·	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% B Que	eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
East:	Georg	je St												
4	L2	31	10.3	31	10.3	0.191	24.0	LOS B	3.4	39.3	0.65	0.56	0.65	24.1
5	T1	506	18.3	506	18.3	0.191	19.6	LOS B	4.6	33.4	0.64	0.53	0.64	19.0
Appro	oach	537	17.8	537	17.8	0.191	19.8	LOS B	4.6	39.3	0.64	0.53	0.64	19.4
North	n: Harris	s St												
7	L2	146	12.9	146	12.9	0.330	40.1	LOS C	6.2	48.2	0.83	0.77	0.83	19.3
8	T1	1538	5.3	1538	5.3	0.764	26.1	LOS B	25.0	182.7	0.82	0.75	0.83	24.1
9	R2	493	5.3	493	5.3	0.764	46.4	LOS D	20.1	147.1	0.97	0.88	1.03	11.9
Appro	oach	2177	5.8	2177	5.8	0.764	31.6	LOS C	25.0	182.7	0.86	0.78	0.88	20.7
West	: Broad	dway												
11	T1	1363	14.0	1363	14.0	0.490	20.6	LOS B	12.5	90.2	0.60	0.61	0.60	30.3
Appro	oach	1363	14.0	1363	14.0	0.490	20.6	LOS B	12.5	90.2	0.60	0.61	0.60	30.3
All Ve	ehicles	4077	10.1	4077	10.1	0.764	26.4	LOS B	25.0	182.7	0.74	0.69	0.75	24.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pe	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	673	50.6	LOS E	2.0	2.0	0.97	0.97
P2	East Full Crossing	465	50.1	LOS E	1.4	1.4	0.96	0.96
P3	North Full Crossing	1259	51.9	LOS E	3.9	3.9	1.00	1.00
P4	West Full Crossing	512	50.2	LOS E	1.5	1.5	0.97	0.97
All Pe	destrians	2908	51.0	LOS E			0.98	0.98

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St Ex AM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	Perform	ance ·	- Vehio	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles	Distance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	West:	Harris St -	NW											
28	T1	2318	5.0	2318	5.0	0.612	0.6	LOS A	2.3	16.7	0.05	0.08	0.05	42.7
29	R2	186	2.3	186	2.3	0.612	4.9	LOS A	2.2	15.7	0.05	0.17	0.05	38.1
Appr	oach	2504	4.7	2504	4.7	0.612	0.9	LOS A	2.3	16.7	0.05	0.09	0.05	42.3
Sout	hWest:	Thomas S	St											
30	L2	23	27.3	23	27.3	0.401	52.0	LOS D	4.5	36.3	0.96	0.77	0.96	13.4
32	R2	65	14.5	65	14.5	0.401	52.3	LOS D	4.5	36.3	0.96	0.77	0.96	13.4
Appr	oach	88	17.9	88	17.9	0.401	52.2	LOS D	4.5	36.3	0.96	0.77	0.96	13.4
All Ve	ehicles	2593	5.2	2593	5.2	0.612	2.7	LOS A	4.5	36.3	0.09	0.11	0.09	33.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pedestr	ians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	158	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd Ex AM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Bac Queue		Prop. Queued	Effective Stop	Aver. A No.	e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Di veh	stance m		Rate	Cycles S	speed km/h
South	nEast:	Harris St -			,,,									
22	T1	22	28.6	22	28.6	0.017	10.5	LOS A	0.9	7.7	0.80	0.60	0.80	25.3
Appro	bach	22	28.6	22	28.6	0.017	10.5	LOS A	0.9	7.7	0.80	0.60	0.80	25.3
North	East: I	Ultimo Rd												
24	L2	249	5.5	249	5.5	0.516	53.6	LOS D	6.5	47.5	0.98	0.79	0.98	7.1
26	R2	75	9.9	75	9.9	0.296	51.8	LOS D	3.7	28.1	0.94	0.76	0.94	10.1
Appro	bach	324	6.5	324	6.5	0.516	53.2	LOS D	6.5	47.5	0.97	0.78	0.97	7.8
North	West:	Harris St -	NW											
27	L2	141	3.0	141	3.0	0.571	6.1	LOS A	4.7	33.8	0.12	0.20	0.12	40.5
28	T1	2262	4.5	2262	4.5	0.571	0.8	LOS A	4.7	33.8	0.07	0.09	0.07	42.9
Appro	bach	2403	4.4	2403	4.4	0.571	1.1	LOS A	4.7	33.8	0.07	0.10	0.07	42.6
All Ve	ehicles	2749	4.9	2749	4.9	0.571	7.4	LOS A	6.5	47.5	0.18	0.18	0.18	24.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestri	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St Ex AM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 124 seconds (Site User-Given Phase Times)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Ba Que	ue	Prop. Queued	Effective Stop	Aver. A No.	e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	speed km/h
South	n: Aber	crombie S		VCII/II	70	v/C	300		VCII					KI1//11
1	L2	71	7.5	71	7.5	0.663	44.9	LOS D	20.0	145.9	0.93	0.81	0.93	27.2
2	T1	1396	4.3	1396	4.3	0.663	40.3	LOS C	20.0	145.9	0.93	0.81	0.93	13.0
3	R2	51	14.6	51	14.6	0.663	45.0	LOS D	19.9	146.4	0.93	0.81	0.93	12.9
Appro	bach	1517	4.8	1517	4.8	0.663	40.7	LOS C	20.0	146.4	0.93	0.81	0.93	14.0
East:	Broad	way - E												
5	T1	1041	11.8	1041	11.8	0.295	12.8	LOS A	9.4	68.3	0.52	0.45	0.52	41.2
6	R2	4	100.0	4	100. 0	0.031	28.2	LOS B	0.2	2.1	0.64	0.65	0.64	22.9
Appro	bach	1045	12.2	1045	12.2	0.295	12.8	LOS A	9.4	68.3	0.52	0.45	0.52	41.1
West	: Broad	lway - W												
10	L2	857	3.9	857	3.9	0.946	49.6	LOS D	58.6	424.0	0.99	1.03	1.18	21.8
11	T1	1438	13.7	1438	13.7	0.569	19.2	LOS B	16.1	126.1	0.60	0.54	0.60	33.0
Appro	bach	2295	10.0	2295	10.0	0.946	30.6	LOS C	58.6	424.0	0.75	0.72	0.82	27.6
All Ve	hicles	4857	8.9	4857	8.9	0.946	29.9	LOS C	58.6	424.0	0.75	0.69	0.79	27.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	504	57.4	LOS E	1.7	1.7	0.97	0.97					
P2	East Full Crossing	126	56.4	LOS E	0.4	0.4	0.96	0.96					
P3	North Full Crossing	809	58.1	LOS E	2.8	2.8	0.98	0.98					
P4	West Full Crossing	225	56.7	LOS E	0.8	0.8	0.96	0.96					
P4B	West Slip/Bypass Lane Crossing	737	57.9	LOS E	2.5	2.5	0.98	0.98					
All Pe	destrians	2402	57.7	LOS E			0.98	0.98					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

▼ Site: 102 [Wattle St-Thomas Street Ex AM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ance - '	Vehio	cles									
Mov ID	Turn	Demand F	Flows A	rrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total	HV 1		ΗV					Distance		Rate	Cycles S	
		veh/h	% V6	eh/h	%	v/c	sec		veh	m				km/h
South	h: Wattl	e St - S												
2	T1	1108	5.3 1	108	5.3	0.299	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	49.9
3a	R1	1053	2.0 1	053	2.0	0.539	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	185	8.0	185	8.0	0.135	4.6	LOS A	0.0	0.0	0.00	0.52	0.00	29.1
Appro	bach	2346	4.0 2	2346	4.0	0.539	1.8	NA	0.0	0.0	0.00	0.23	0.00	46.2
East:	Thoma	as St												
6b	R3	124	4.2	124	4.2	0.750	52.1	LOS D	4.2	30.8	0.95	1.32	1.94	19.8
Appro	bach	124	4.2	124	4.2	0.750	52.1	LOS D	4.2	30.8	0.95	1.32	1.94	19.8
All Ve	ehicles	2471	4.0 2	2471	4.0	0.750	4.4	NA	4.2	30.8	0.05	0.28	0.10	41.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay

is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Broadway-George St-Harris St-Regent St Ex PM]

♦ Network: N101 [Ex PM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Ba Que	ue	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
East:	Georg	e St												
4	L2	23	18.2	23	18.2	0.350	27.9	LOS B	5.4	66.4	0.65	0.56	0.65	22.5
5	T1	1078	14.5	1078	14.5	0.437	24.7	LOS B	10.6	75.0	0.67	0.58	0.67	16.8
Appro	bach	1101	14.5	1101	14.5	0.437	24.7	LOS B	10.6	75.0	0.67	0.58	0.67	16.9
North	: Harris	s St												
7	L2	245	4.7	245	4.7	0.421	27.1	LOS B	8.2	59.7	0.66	0.74	0.66	23.7
8	T1	1597	1.9	1597	1.9	0.806	19.5	LOS B	29.8	212.3	0.77	0.72	0.79	27.8
9	R2	914	1.5	914	1.5	0.806	44.4	LOS D	25.2	177.4	0.98	0.91	1.06	12.2
Appro	bach	2756	2.0	2756	2.0	0.806	28.4	LOS B	29.8	212.3	0.83	0.78	0.87	21.3
West	Broad	lway												
11	T1	975	9.0	975	9.0	0.410	27.8	LOS B	11.7	82.3	0.77	0.71	0.77	27.1
Appro	bach	975	9.0	975	9.0	0.410	27.8	LOS B	11.7	82.3	0.77	0.71	0.77	27.1
All Ve	hicles	4832	6.3	4832	6.3	0.806	27.5	LOS B	29.8	212.3	0.78	0.72	0.80	22.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	900	51.1	LOS E	2.8	2.8	0.98	0.98				
P2	East Full Crossing	848	51.0	LOS E	2.6	2.6	0.98	0.98				
P3	North Full Crossing	3368	57.2	LOS E	11.6	11.6	1.10	1.10				
P4	West Full Crossing	760	50.8	LOS E	2.3	2.3	0.98	0.98				
All Pe	All Pedestrians		54.5	LOS E			1.05	1.05				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St Ex PM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles	Distance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	nWest:	Harris St -	NW											
28	T1	2806	1.9	2806	1.9	0.828	2.8	LOS A	12.2	87.1	0.22	0.23	0.23	31.6
29	R2	147	2.9	147	2.9	0.828	7.4	LOS A	11.0	78.7	0.22	0.28	0.24	28.6
Appr	oach	2954	1.9	2954	1.9	0.828	3.0	LOS A	12.2	87.1	0.22	0.24	0.23	31.4
Sout	hWest:	Thomas S	st											
30	L2	38	30.6	38	30.6	0.551	49.7	LOS D	7.7	58.1	0.96	0.80	0.96	13.8
32	R2	115	1.8	115	1.8	0.551	50.0	LOS D	7.7	58.1	0.96	0.80	0.96	13.8
Appr	oach	153	9.0	153	9.0	0.551	49.9	LOS D	7.7	58.1	0.96	0.80	0.96	13.8
All Ve	ehicles	3106	2.3	3106	2.3	0.828	5.3	LOS A	12.2	87.1	0.26	0.27	0.27	25.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95				
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95				
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95				
All Pe	destrians	158	49.3	LOS E			0.95	0.95				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd Ex PM]

New Site Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
South	nEast: I	Harris St -	SE											
22	T1	38	30.6	38	30.6	0.034	18.5	LOS B	1.8	15.6	0.93	0.72	0.93	34.6
Appro	bach	38	30.6	38	30.6	0.034	18.5	LOS B	1.8	15.6	0.93	0.72	0.93	34.6
North	East: l	Jltimo Rd												
24	L2	543	2.3	543	2.3	0.803	51.8	LOS D	16.0	114.1	0.98	0.93	1.14	7.3
26	R2	163	1.3	163	1.3	0.338	41.5	LOS C	7.2	51.2	0.87	0.78	0.87	25.7
Appro	bach	706	2.1	706	2.1	0.803	49.4	LOS D	16.0	114.1	0.95	0.90	1.08	12.8
North	West:	Harris St -	NW											
27	L2	134	0.0	134	0.0	0.812	14.7	LOS B	22.2	157.3	0.56	0.57	0.58	39.6
28	T1	2418	1.8	2418	1.8	0.812	8.0	LOS A	22.2	158.0	0.52	0.50	0.53	41.0
Appro	bach	2552	1.7	2552	1.7	0.812	8.4	LOS A	22.2	158.0	0.52	0.50	0.53	40.9
All Ve	hicles	3296	2.1	3296	2.1	0.812	17.3	LOS B	22.2	158.0	0.62	0.59	0.65	32.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St Ex PM]

New Site Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 123 seconds (Site User-Given Phase Times)

Mov	ement	t Perform	nance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% Ba Quei		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles E veh)istance m		Rate	Cycles S	Speed km/h
South	n: Aber	crombie S												
1	L2	129	2.4	129	2.4	0.628	40.3	LOS C	20.7	147.3	0.89	0.80	0.89	28.4
2	T1	1484	1.3	1484	1.3	0.628	35.7	LOS C	21.0	148.5	0.89	0.79	0.89	14.1
3	R2	59	1.8	59	1.8	0.628	40.3	LOS C	21.0	148.5	0.89	0.79	0.89	14.0
Appro	bach	1673	1.4	1673	1.4	0.628	36.2	LOS C	21.0	148.5	0.89	0.79	0.89	15.9
East:	Broad	way - E												
5	T1	1931	8.4	1931	8.4	0.596	18.8	LOS B	24.4	172.9	0.70	0.63	0.70	38.0
6	R2	14	100.0	14	100. 0	0.080	31.0	LOS C	0.5	7.1	0.69	0.69	0.69	21.7
Appro	bach	1944	9.0	1944	9.0	0.596	18.9	LOS B	24.4	172.9	0.70	0.63	0.70	37.9
West	: Broad	dway - W												
10	L2	618	1.2	618	1.2	0.766	36.8	LOS C	31.4	222.1	0.92	0.86	0.92	25.5
11	T1	1087	8.3	1087	8.3	0.477	27.4	LOS B	15.6	117.1	0.77	0.68	0.77	28.9
Appro	bach	1705	5.7	1705	5.7	0.766	30.8	LOS C	31.4	222.1	0.83	0.74	0.83	27.5
All Ve	ehicles	5322	5.6	5322	5.6	0.766	28.2	LOS B	31.4	222.1	0.80	0.71	0.80	29.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	761	57.5	LOS E	2.6	2.6	0.98	0.98				
P2	East Full Crossing	201	56.1	LOS E	0.7	0.7	0.96	0.96				
P3	North Full Crossing	1649	59.8	LOS E	5.9	5.9	1.02	1.02				
P4	West Full Crossing	375	56.5	LOS E	1.3	1.3	0.97	0.97				
P4B	West Slip/Bypass Lane Crossing	1579	59.6	LOS E	5.6	5.6	1.02	1.02				
All Pe	destrians	4565	58.9	LOS E			1.01	1.01				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

♥ Site: 102 [Wattle St-Thomas Street Ex PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ance - Vehi	cles									
Mov ID	Turn	Demand F	Flows Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total	HV Total	ΗV				Vehicles	Distance		Rate	Cycles S	Speed
		veh/h	% veh/h	%	v/c	sec		veh	m				km/h
South: Wattle St - S													
2	T1	1061	2.3 1061	2.3	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
3a	R1	1053	0.4 1053	0.4	0.507	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	126	8.3 126	8.3	0.127	4.6	LOS A	0.0	0.0	0.00	0.49	0.00	30.3
Appro	bach	2240	1.7 2240	1.7	0.507	1.8	NA	0.0	0.0	0.00	0.22	0.00	46.4
East:	Thoma	as St											
6b	R3	122	3.4 122	3.4	0.629	36.5	LOS C	3.2	23.3	0.92	1.19	1.57	23.7
Appro	bach	122	3.4 122	3.4	0.629	36.5	LOS C	3.2	23.3	0.92	1.19	1.57	23.7
All Ve	ehicles	2362	1.8 2362	1.8	0.629	3.6	NA	3.2	23.3	0.05	0.27	0.08	42.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay

is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Broadway-George St-Harris St-Regent St Post Dev PM]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand Total veh/h	HV	Arrival Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% B Que Vehicles veh	eue	Prop. Queued	Effective Stop Rate	Aver. / No. Cycles S	e
East:	Georg		,,,		70	110			Volt					1111/11
4	L2	23	18.2	23	18.2	0.350	27.9	LOS B	5.4	66.4	0.65	0.56	0.65	22.5
5	T1	1078	14.5	1078	14.5	0.437	24.7	LOS B	10.6	75.0	0.67	0.58	0.67	16.8
Appro	bach	1101	14.5	1101	14.5	0.437	24.7	LOS B	10.6	75.0	0.67	0.58	0.67	16.9
North	: Harri	s St												
7	L2	246	4.7	246	4.7	0.423	27.4	LOS B	8.3	60.4	0.67	0.74	0.67	23.6
8	T1	1604	1.9	1604	1.9	0.810	19.7	LOS B	30.2	215.0	0.78	0.72	0.79	27.6
9	R2	917	1.5	917	1.5	0.810	44.6	LOS D	25.4	179.0	0.98	0.91	1.06	12.1
Appro	bach	2767	2.0	2767	2.0	0.810	28.7	LOS C	30.2	215.0	0.83	0.79	0.87	21.2
West	Broad	lway												
11	T1	975	9.0	975	9.0	0.410	27.8	LOS B	11.7	82.3	0.77	0.71	0.77	27.1
Appro	bach	975	9.0	975	9.0	0.410	27.8	LOS B	11.7	82.3	0.77	0.71	0.77	27.1
All Ve	hicles	4843	6.3	4843	6.3	0.810	27.6	LOS B	30.2	215.0	0.78	0.72	0.81	22.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	900	51.1	LOS E	2.8	2.8	0.98	0.98					
P2	East Full Crossing	848	51.0	LOS E	2.6	2.6	0.98	0.98					
P3	North Full Crossing	3368	57.2	LOS E	11.6	11.6	1.10	1.10					
P4	West Full Crossing	760	50.8	LOS E	2.3	2.3	0.98	0.98					
All Pe	destrians	5877	54.5	LOS E			1.05	1.05					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St Post Dev PM]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		lack of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
North	West:	Harris St -	NW											
28	T1	2806	1.9	2806	1.9	0.839	3.4	LOS A	13.1	93.2	0.24	0.25	0.25	29.2
29	R2	166	2.5	166	2.5	0.839	8.5	LOS A	12.5	88.7	0.24	0.31	0.26	25.5
Appro	oach	2973	1.9	2973	1.9	0.839	3.7	LOS A	13.1	93.2	0.24	0.26	0.25	29.0
Sout	hWest:	Thomas S	St											
30	L2	43	26.8	43	26.8	0.612	50.2	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
32	R2	126	1.7	126	1.7	0.612	50.6	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
Appro	oach	169	8.1	169	8.1	0.612	50.5	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
All Ve	ehicles	3142	2.2	3142	2.2	0.839	6.2	LOS A	13.1	93.2	0.28	0.29	0.29	23.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	destrians	158	49.3	LOS E			0.95	0.95						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd Post Dev PM]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% B Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
South	nEast:	Harris St -	SE											
22	T1	38	30.6	38	30.6	0.034	17.0	LOS B	1.6	14.4	0.86	0.66	0.86	35.5
Appro	bach	38	30.6	38	30.6	0.034	17.0	LOS B	1.6	14.4	0.86	0.66	0.86	35.5
North	NorthEast: Ultimo Rd													
24	L2	543	2.3	543	2.3	0.851	56.9	LOS E	16.3	116.1	0.99	0.99	1.25	19.9
26	R2	163	1.3	163	1.3	0.338	41.5	LOS C	7.2	51.2	0.87	0.78	0.87	31.8
Appro	bach	706	2.1	706	2.1	0.851	53.3	LOS D	16.3	116.1	0.96	0.94	1.16	23.1
North	West:	Harris St -	NW											
27	L2	134	0.0	134	0.0	0.856	19.8	LOS B	27.7	196.6	0.63	0.67	0.70	40.7
28	T1	2437	1.8	2437	1.8	0.856	12.4	LOS A	28.6	203.6	0.59	0.60	0.64	37.4
Appro	bach	2571	1.7	2571	1.7	0.856	12.8	LOS A	28.6	203.6	0.59	0.60	0.64	37.7
All Ve	ehicles	3315	2.1	3315	2.1	0.856	21.5	LOS B	28.6	203.6	0.67	0.67	0.76	32.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	destrians	211	49.3	LOS E			0.95	0.95						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St Post Dev

PM]

New Site

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 123 seconds (Site User-Given Phase Times)

Mov	omont	Perform	22000	Vohi	clos									
						-			050/ 5		_			
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Queu		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles D veh	istance m		Rate	Cycles S	Speed km/h
Sout	n: Aber	crombie S		VOII/II	70	10	000		VOIT					
1	L2	129	2.4	129	2.4	0.631	40.4	LOS C	20.9	148.4	0.89	0.80	0.89	28.4
2	T1	1494	1.3	1494	1.3	0.631	35.8	LOS C	21.1	149.6	0.89	0.79	0.89	14.1
3	R2	59	1.8	59	1.8	0.631	40.3	LOS C	21.1	149.6	0.89	0.79	0.89	14.0
Appr	oach	1682	1.4	1682	1.4	0.631	36.3	LOS C	21.1	149.6	0.89	0.79	0.89	15.9
East:	Broad	way - E												
5	T1	1931	8.4	1931	8.4	0.596	18.8	LOS B	24.4	172.9	0.70	0.63	0.70	38.0
6	R2	14	100.0	14	100. 0	0.080	31.0	LOS C	0.5	7.1	0.69	0.69	0.69	21.7
Appr	oach	1944	9.0	1944	9.0	0.596	18.9	LOS B	24.4	172.9	0.70	0.63	0.70	37.9
West	: Broad	lway - W												
10	L2	627	1.2	627	1.2	0.777	37.0	LOS C	32.2	227.3	0.93	0.87	0.93	25.4
11	T1	1087	8.3	1087	8.3	0.477	27.4	LOS B	15.6	117.1	0.77	0.68	0.77	28.9
Appr	oach	1715	5.7	1715	5.7	0.777	30.9	LOS C	32.2	227.3	0.83	0.75	0.83	27.5
All Ve	ehicles	5341	5.6	5341	5.6	0.777	28.2	LOS B	32.2	227.3	0.80	0.72	0.80	29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective					
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m							
P1	South Full Crossing	761	57.5	LOS E	2.6	2.6	0.98	0.98					
P2	East Full Crossing	201	56.1	LOS E	0.7	0.7	0.96	0.96					
P3	North Full Crossing	1649	59.8	LOS E	5.9	5.9	1.02	1.02					
P4	West Full Crossing	375	56.5	LOS E	1.3	1.3	0.97	0.97					
P4B	West Slip/Bypass Lane	1579	59.6	LOS E	5.6	5.6	1.02	1.02					
	Crossing												
All Pe	destrians	4565	58.9	LOS E			1.01	1.01					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

▽ Site: 102 [Wattle St-Thomas Street Post Dev PM]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance	- Vehi	cles									
Mov ID	Turn	Demand F	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles I	Distance		Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Wattl	e St - S												
2	T1	1061	2.3	1061	2.3	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
3a	R1	1053	0.4	1053	0.4	0.516	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	145	7.2	145	7.2	0.129	4.6	LOS A	0.0	0.0	0.00	0.50	0.00	29.9
Appro	bach	2259	1.7	2259	1.7	0.516	1.8	NA	0.0	0.0	0.00	0.23	0.00	46.3
East:	Thoma	as St												
6b	R3	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.28	1.83	21.8
Appro	bach	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.28	1.83	21.8
All Ve	hicles	2393	1.8	2393	1.8	0.714	4.1	NA	4.0	29.0	0.05	0.29	0.10	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Broadway-George St-Harris St-Regent St Post

Dev PM +Widened Crossing]

♦♦ Network: N101 [Post Dev PM +Widened Crossing]

New Site

Site Category: (None) Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% B Que		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
East:	Georg	e St												
4	L2	23	18.2	23	18.2	0.359	28.9	LOS C	5.5	68.0	0.67	0.57	0.67	22.1
5	T1	1078	14.5	1078	14.5	0.447	25.6	LOS B	10.8	77.0	0.69	0.59	0.69	16.4
Appro	bach	1101	14.5	1101	14.5	0.447	25.7	LOS B	10.8	77.0	0.69	0.59	0.69	16.5
North	: Harris	s St												
7	L2	246	4.7	246	4.7	0.423	27.4	LOS B	8.3	60.4	0.67	0.74	0.67	23.6
8	T1	1604	1.9	1604	1.9	0.810	19.7	LOS B	30.2	215.0	0.78	0.72	0.79	27.6
9	R2	917	1.5	917	1.5	0.810	44.6	LOS D	25.4	179.0	0.98	0.91	1.06	12.1
Appro	bach	2767	2.0	2767	2.0	0.810	28.7	LOS C	30.2	215.0	0.83	0.79	0.87	21.2
West:	Broad	lway												
11	T1	975	9.0	975	9.0	0.420	28.6	LOS C	11.9	83.6	0.79	0.72	0.79	26.7
Appro	bach	975	9.0	975	9.0	0.420	28.6	LOS C	11.9	83.6	0.79	0.72	0.79	26.7
All Ve	hicles	4843	6.3	4843	6.3	0.810	28.0	LOS B	30.2	215.0	0.79	0.73	0.81	21.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	900	51.1	LOS E	2.8	2.8	0.98	0.98					
P2	East Full Crossing	848	51.0	LOS E	2.6	2.6	0.98	0.98					
P3	North Full Crossing	3368	57.2	LOS E	11.6	11.6	1.10	1.10					
P4	West Full Crossing	760	50.8	LOS E	2.3	2.3	0.98	0.98					
All Pe	destrians	5877	54.5	LOS E			1.05	1.05					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St Post Dev PM +Widened Crossing]

♦♦ Network: N101 [Post Dev PM +Widened Crossing]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
North	nWest:	Harris St -	NW											
28	T1	2806	1.9	2806	1.9	0.839	3.4	LOS A	13.1	93.2	0.24	0.25	0.25	29.2
29	R2	166	2.5	166	2.5	0.839	8.5	LOS A	12.5	88.7	0.24	0.31	0.26	25.5
Appro	oach	2973	1.9	2973	1.9	0.839	3.7	LOS A	13.1	93.2	0.24	0.26	0.25	29.0
South	hWest:	Thomas S	st											
30	L2	43	26.8	43	26.8	0.612	50.2	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
32	R2	126	1.7	126	1.7	0.612	50.6	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
Appro	oach	169	8.1	169	8.1	0.612	50.5	LOS D	8.7	64.9	0.98	0.81	0.98	13.7
All Ve	ehicles	3142	2.2	3142	2.2	0.839	6.2	LOS A	13.1	93.2	0.28	0.29	0.29	23.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate							
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
All Pe	destrians	158	49.3	LOS E			0.95	0.95							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd Post Dev PM +Widened

♦♦ Network: N101 [Post Dev PM +Widened Crossing]

Crossing]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network Site User-Given Phase Times)

Move	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
South	nEast:	Harris St -	SE											
22	T1	38	30.6	38	30.6	0.034	17.0	LOS B	1.6	14.4	0.86	0.66	0.86	35.5
Appro	bach	38	30.6	38	30.6	0.034	17.0	LOS B	1.6	14.4	0.86	0.66	0.86	35.5
North	East: l	Jltimo Rd												
24	L2	543	2.3	543	2.3	0.851	56.9	LOS E	16.3	116.1	0.99	0.99	1.25	19.9
26	R2	163	1.3	163	1.3	0.338	41.5	LOS C	7.2	51.2	0.87	0.78	0.87	31.8
Appro	bach	706	2.1	706	2.1	0.851	53.3	LOS D	16.3	116.1	0.96	0.94	1.16	23.1
North	West:	Harris St -	NW											
27	L2	134	0.0	134	0.0	0.856	19.8	LOS B	27.7	196.6	0.63	0.67	0.70	40.7
28	T1	2437	1.8	2437	1.8	0.856	12.4	LOS A	28.6	203.6	0.59	0.60	0.64	37.4
Appro	bach	2571	1.7	2571	1.7	0.856	12.8	LOS A	28.6	203.6	0.59	0.60	0.64	37.7
All Ve	ehicles	3315	2.1	3315	2.1	0.856	21.5	LOS B	28.6	203.6	0.67	0.67	0.76	32.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St Post Dev

PM +Widened Crossing]

♦♦ Network: N101 [Post Dev PM +Widened Crossing]

New Site

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 123 seconds (Site User-Given Phase Times)

Mov	ement	t Perform	nance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quei		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles E veh	Distance m		Rate	Cycles S	Speed km/h
Sout	h: Aber	crombie S	St											
1	L2	129	2.4	129	2.4	0.631	40.4	LOS C	20.9	148.4	0.89	0.80	0.89	28.4
2	T1	1494	1.3	1494	1.3	0.631	35.8	LOS C	21.1	149.6	0.89	0.79	0.89	14.1
3	R2	59	1.8	59	1.8	0.631	40.3	LOS C	21.1	149.6	0.89	0.79	0.89	14.0
Appro	oach	1682	1.4	1682	1.4	0.631	36.3	LOS C	21.1	149.6	0.89	0.79	0.89	15.9
East:	Broad	way - E												
5	T1	1931	8.4	1931	8.4	0.596	18.8	LOS B	24.4	172.9	0.70	0.63	0.70	38.0
6	R2	14	100.0	14	100. 0	0.080	31.0	LOS C	0.5	7.1	0.69	0.69	0.69	21.7
Appro	oach	1944	9.0	1944	9.0	0.596	18.9	LOS B	24.4	172.9	0.70	0.63	0.70	37.9
West	: Broad	dway - W												
10	L2	627	1.2	627	1.2	0.777	37.0	LOS C	32.2	227.3	0.93	0.87	0.93	25.4
11	T1	1087	8.3	1087	8.3	0.477	27.4	LOS B	15.6	117.1	0.77	0.68	0.77	28.9
Appro	oach	1715	5.7	1715	5.7	0.777	30.9	LOS C	32.2	227.3	0.83	0.75	0.83	27.5
All Ve	ehicles	5341	5.6	5341	5.6	0.777	28.2	LOS B	32.2	227.3	0.80	0.72	0.80	29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate							
P1	South Full Crossing	761	57.5	LOS E	2.6	2.6	0.98	0.98							
P2	East Full Crossing	201	56.1	LOS E	0.7	0.7	0.96	0.96							
P3	North Full Crossing	1649	59.8	LOS E	5.9	5.9	1.02	1.02							
P4	West Full Crossing	375	56.5	LOS E	1.3	1.3	0.97	0.97							
P4B	West Slip/Bypass Lane Crossing	1579	59.6	LOS E	5.6	5.6	1.02	1.02							
All Pe	destrians	4565	58.9	LOS E			1.01	1.01							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 102 [Wattle St-Thomas Street Post Dev PM +Widened

♦♦ Network: N101 [Post Dev PM +Widened Crossing]

Crossing]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance ·	- Vehi	cles									
Mov ID	Turn	Demand F	-lows .	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles	Distance		Rate	Cycles S	speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Watt	e St - S												
2	T1	1061	2.3	1061	2.3	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
3a	R1	1053	0.4	1053	0.4	0.516	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	145	7.2	145	7.2	0.129	4.6	LOS A	0.0	0.0	0.00	0.50	0.00	29.9
Appro	bach	2259	1.7	2259	1.7	0.516	1.8	NA	0.0	0.0	0.00	0.23	0.00	46.3
East:	Thoma	as St												
6b	R3	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.28	1.83	21.8
Appro	bach	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.28	1.83	21.8
All Ve	hicles	2393	1.8	2393	1.8	0.714	4.1	NA	4.0	29.0	0.05	0.29	0.10	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Broadway-George St-Harris St-Regent St -Post

Dev PM+Road Narrowing]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Move	ement	Perform	ance	- Vehi	cles									ļ
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% B Que	eue	Prop. Queued	Effective Stop	Aver. A No.	e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
East:	Georg	e St												
4	L2	23	18.2	23	18.2	0.640	48.3	LOS D	8.0	98.8	0.93	0.80	0.96	16.2
5	T1	1078	14.5	1078	14.5	0.797	47.6	LOS D	16.6	117.8	0.98	0.90	1.07	10.8
Appro	bach	1101	14.5	1101	14.5	0.797	47.6	LOS D	16.6	117.8	0.98	0.90	1.06	10.9
North	: Harris	s St												
7	L2	248	4.7	248	4.7	0.279	20.7	LOS B	6.5	47.4	0.53	0.70	0.53	26.7
8	T1	1604	1.9	1604	1.9	0.782	7.8	LOS A	26.4	187.4	0.46	0.45	0.46	37.4
9	R2	915	1.5	915	1.5	0.782	38.3	LOS C	36.1	256.4	0.93	0.85	0.93	13.7
Appro	bach	2767	2.0	2767	2.0	0.782	19.0	LOS B	36.1	256.4	0.62	0.60	0.62	26.2
West	Broad	lway												
11	T1	975	9.0	975	9.0	0.748	47.3	LOS D	15.8	111.1	0.99	0.88	1.05	20.9
Appro	bach	975	9.0	975	9.0	0.748	47.3	LOS D	15.8	111.1	0.99	0.88	1.05	20.9
All Ve	hicles	4843	6.3	4843	6.3	0.797	31.2	LOS C	36.1	256.4	0.78	0.73	0.81	20.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians Mov Demand Average Level of Average Back of Queue Prop. Effective														
Mov ID	Description		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate									
P1	South Full Crossing	900	51.1	LOS E	2.8	2.8	0.98	0.98							
P2	East Full Crossing	848	51.0	LOS E	2.6	2.6	0.98	0.98							
P3	North Full Crossing	3368	57.2	LOS E	11.6	11.6	1.10	1.10							
P4	West Full Crossing	760	50.8	LOS E	2.3	2.3	0.98	0.98							
All Pe	destrians	5877	54.5	LOS E			1.05	1.05							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St -Post Dev PM+Road Narrowing]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% B Qu		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
North	nWest:	Harris St -	NW											
28	T1	2806	1.9	2806	1.9	0.729	0.8	LOS A	3.8	26.8	0.08	0.10	0.08	41.9
29	R2	166	2.5	166	2.5	0.729	5.1	LOS A	3.5	24.7	0.08	0.16	0.08	38.5
Appr	oach	2973	1.9	2973	1.9	0.729	1.0	LOS A	3.8	26.8	0.08	0.10	0.08	41.7
Sout	hWest:	Thomas S	St											
30	L2	43	26.8	43	26.8	0.705	54.3	LOS D	9.2	68.7	1.00	0.86	1.09	13.0
32	R2	126	1.7	126	1.7	0.705	54.7	LOS D	9.2	68.7	1.00	0.86	1.09	13.0
Appr	oach	169	8.1	169	8.1	0.705	54.6	LOS D	9.2	68.7	1.00	0.86	1.09	13.0
All Ve	ehicles	3142	2.2	3142	2.2	0.729	3.9	LOS A	9.2	68.7	0.13	0.14	0.13	29.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate							
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95							
All Pe	destrians	158	49.3	LOS E			0.95	0.95							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd -Post Dev PM+Road

Narrowing]

New Site

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
Sout	nEast:	Harris St -	SE											
22	T1	38	30.6	38	30.6	0.034	11.2	LOS A	1.2	10.5	0.63	0.48	0.63	39.4
Appro	oach	38	30.6	38	30.6	0.034	11.2	LOS A	1.2	10.5	0.63	0.48	0.63	39.4
North	nEast: I	Ultimo Rd												
24	L2	543	2.3	543	2.3	0.680	46.0	LOS D	13.4	95.7	0.95	0.84	0.97	8.1
26	R2	163	1.3	163	1.3	0.351	42.4	LOS C	7.3	51.9	0.88	0.78	0.88	25.4
Appro	oach	706	2.1	706	2.1	0.680	45.2	LOS D	13.4	95.7	0.94	0.82	0.95	13.7
North	West:	Harris St -	NW											
27	L2	134	0.0	134	0.0	0.700	16.2	LOS B	26.2	185.7	0.64	0.62	0.64	38.6
28	T1	2437	1.8	2437	1.8	0.700	12.5	LOS A	29.1	207.2	0.68	0.63	0.68	37.3
Appro	bach	2571	1.7	2571	1.7	0.700	12.7	LOS A	29.1	207.2	0.67	0.63	0.67	37.4
All Ve	ehicles	3315	2.1	3315	2.1	0.700	19.6	LOS B	29.1	207.2	0.73	0.67	0.73	31.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St -Post Dev

PM+Road Narrowing]

♦♦ Network: N101 [Post Dev PM + Road Narrowing]

New Site

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 123 seconds (Site User-Given Phase Times)

Mov	ement	t Perform	nance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
South	n: Aber	crombie S	t											
1	L2	129	2.4	129	2.4	0.631	40.4	LOS C	20.9	148.4	0.89	0.80	0.89	28.4
2	T1	1494	1.3	1494	1.3	0.631	35.8	LOS C	21.1	149.6	0.89	0.79	0.89	14.1
3	R2	59	1.8	59	1.8	0.631	40.3	LOS C	21.1	149.6	0.89	0.79	0.89	14.0
Appro	oach	1682	1.4	1682	1.4	0.631	36.3	LOS C	21.1	149.6	0.89	0.79	0.89	15.9
East:	Broad	way - E												
5	T1	1931	8.4	1931	8.4	0.596	18.8	LOS B	24.4	172.9	0.70	0.63	0.70	38.0
6	R2	14	100.0	14	100. 0	0.080	31.0	LOS C	0.5	7.1	0.69	0.69	0.69	21.7
Appro	oach	1944	9.0	1944	9.0	0.596	18.9	LOS B	24.4	172.9	0.70	0.63	0.70	37.9
West	: Broad	dway - W												
10	L2	627	1.2	627	1.2	0.777	37.0	LOS C	32.2	227.3	0.93	0.87	0.93	25.4
11	T1	1087	8.3	1087	8.3	0.477	27.4	LOS B	15.6	117.1	0.77	0.68	0.77	28.9
Appro	oach	1715	5.7	1715	5.7	0.777	30.9	LOS C	32.2	227.3	0.83	0.75	0.83	27.5
All Ve	ehicles	5341	5.6	5341	5.6	0.777	28.2	LOS B	32.2	227.3	0.80	0.72	0.80	29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

	ment Performance - Pedest							
Mov	Description	Demand	Average		Average Back		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	761	57.5	LOS E	2.6	2.6	0.98	0.98
P2	East Full Crossing	201	56.1	LOS E	0.7	0.7	0.96	0.96
P3	North Full Crossing	1649	59.8	LOS E	5.9	5.9	1.02	1.02
P4	West Full Crossing	375	56.5	LOS E	1.3	1.3	0.97	0.97
P4B	West Slip/Bypass Lane	1579	59.6	LOS E	5.6	5.6	1.02	1.02
	Crossing							
All Pe	destrians	4565	58.9	LOS E			1.01	1.01

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 102 [Wattle St-Thomas Street -Post Dev PM+Road

Narrowing]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ance ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% B Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total		Total	ΗV				Vehicles	Distance		Rate	Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	h: Watt	e St - S												
2	T1	1061	2.3	1061	2.3	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
3a	R1	1053	0.4	1053	0.4	0.516	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	145	7.2	145	7.2	0.129	4.6	LOS A	0.0	0.0	0.00	0.50	0.00	29.9
Appro	bach	2259	1.7	2259	1.7	0.516	1.8	NA	0.0	0.0	0.00	0.23	0.00	46.3
East:	Thoma	as St												
6b	R3	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.29	1.83	21.8
Appro	bach	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.29	1.83	21.8
All Ve	ehicles	2393	1.8	2393	1.8	0.714	4.1	NA	4.0	29.0	0.05	0.29	0.10	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Site: 101 [Broadway-George St-Harris St-Regent St -Post Dev PM +All Modifications]

+Mid-block crossing on Harris St +Road Narrowing of Harris St +Widened intersection crossing (addition of 1sec intergreen time) Site Category: -

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
East:	Georg	je St												
4	L2	23	18.2	23	18.2	0.640	48.3	LOS D	8.0	98.8	0.93	0.80	0.96	16.2
5	T1	1078	14.5	1078	14.5	0.797	47.6	LOS D	16.6	117.8	0.98	0.90	1.07	10.8
Appro	bach	1101	14.5	1101	14.5	0.797	47.6	LOS D	16.6	117.8	0.98	0.90	1.06	10.9
North	: Harri	s St												
7	L2	246	4.7	246	4.7	0.282	23.0	LOS B	8.5	62.0	0.69	0.75	0.69	21.9
8	T1	1604	1.9	1604	1.9	0.795	8.6	LOS A	19.6	138.7	0.51	0.49	0.51	33.0
9	R2	917	1.5	917	1.5	0.795	28.1	LOS B	19.6	138.7	0.86	0.84	0.87	10.7
Appro	bach	2767	2.0	2767	2.0	0.795	16.3	LOS B	19.6	138.7	0.64	0.63	0.64	23.3
West	: Broad	dway												
11	T1	975	9.0	975	9.0	0.748	47.3	LOS D	15.8	111.1	0.99	0.88	1.05	20.9
Appro	bach	975	9.0	975	9.0	0.748	47.3	LOS D	15.8	111.1	0.99	0.88	1.05	20.9
All Ve	ehicles	4843	6.3	4843	6.3	0.797	29.7	LOS C	19.6	138.7	0.79	0.74	0.82	18.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	Distance	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	ped/h 900	sec 51.1	LOS E	ped 2.8	m 2.8	0.98	0.98
P2	East Full Crossing	848	51.0	LOS E	2.6	2.6	0.98	0.98
P3	North Full Crossing	3368	57.2	LOS E	11.6	11.6	1.10	1.10
P4	West Full Crossing	760	50.8	LOS E	2.3	2.3	0.98	0.98
All Pe	destrians	5877	54.5	LOS E			1.05	1.05

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St Midblock Crossing -Post Dev PM +All Modifications]

+Mid-block crossing on Harris St

+Road Narrowing of Harris St

+Widened intersection crossing (addition of 1sec intergreen time)

Site Category: -

Pedestrian Crossing (Signals) - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Movem	ent P	erforma	ance - Vehic	les									
Mov Tu ID	ırn D	emand F Total	Flows Arrival I HV Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Bac Queu Vehicles Di	е	Prop. Queued	Effective Stop Rate	Aver. A No. Cycles S	e
		veh/h	% veh/h	%	v/c	sec		venicies bi	m		Trate		km/h
NorthWe	est: Ha	arris St - I	N										
8 T	1	2933	1.9 2933	1.9	0.823	4.4	LOS A	6.4	45.4	0.11	0.14	0.15	34.0
Approac	h	2933	1.9 2933	1.9	0.823	4.4	LOS A	6.4	45.4	0.11	0.14	0.15	34.0
All Vehic	les	2933	1.9 2933	1.9	0.823	4.4	LOS A	6.4	45.4	0.11	0.14	0.15	34.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pedest	rians						
Mov		Demand	Average	Level of a	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	SouthEast Full Crossing	245	49.7	LOS E	0.7	0.7	0.96	0.96
All Pe	destrians	245	49.7	LOS E			0.96	0.96

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Thomas St -Post Dev PM +All Modifications]

+Mid-block crossing on Harris St +Road Narrowing of Harris St +Widened intersection crossing (addition of 1sec intergreen time) Site Category: -Signals - Fixed Time Coordinated - Cycle Time = 110 seconds (Network)

Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles	Distance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	nWest:	Harris St -	NW											
28	T1	2806	1.9	2806	1.9	0.695	0.7	LOS A	3.3	23.3	0.07	0.09	0.07	42.4
29	R2	166	2.5	166	2.5	0.695	5.0	LOS A	3.3	23.2	0.07	0.15	0.07	39.1
Appr	oach	2973	1.9	2973	1.9	0.695	0.9	LOS A	3.3	23.3	0.07	0.09	0.07	42.2
Sout	hWest:	Thomas S	st											
30	L2	43	26.8	43	26.8	0.708	55.6	LOS D	9.2	69.0	1.00	0.86	1.09	12.8
32	R2	126	1.7	126	1.7	0.708	55.9	LOS D	9.2	69.0	1.00	0.86	1.09	12.8
Appr	oach	169	8.1	169	8.1	0.708	55.9	LOS D	9.2	69.0	1.00	0.86	1.09	12.8
All V	ehicles	3142	2.2	3142	2.2	0.708	3.9	LOS A	9.2	69.0	0.12	0.13	0.12	29.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pedestr	ians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P8	SouthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	158	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Harris St-Ultimo Rd -Post Dev PM +All Modifications]

+Mid-block crossing on Harris St +Road Narrowing of Harris St +Widened intersection crossing (addition of 1sec intergreen time) Site Category: -Signals - Fixed Time Coordinated Cycle Time = 110 seconds (Network User-Given Cycle Time)

Movement Performance - Vehicles Mov Turn Demand Flows Arrival Flows Deg. Average Level of 95% Back of Prop. Effective Aver. Averag ID Satn Delay Service Queue Queued Stop No Total Total Vehicles Distance Rate Cycles Speed veh/h veh/h km/h SouthEast: Harris St - SE 38 30.6 0.034 19.3 LOS B 1.9 16.6 0.99 0.76 0.99 34.2 22 T1 38 30.6 Approach 38 30.6 38 30.6 0.034 19.3 LOS B 1.9 16.6 0.99 0.76 0.99 34.2 NorthEast: Ultimo Rd 24 L2 543 46.0 LOS D 13.4 0.95 0.97 543 2.3 2.3 0.680 95.7 0.84 22.5 26 R2 163 1.3 163 1.3 0.351 42.4 LOS C 7.3 51.9 0.88 0.78 0.88 31.5 706 13.4 95.7 0.94 0.95 25.2 Approach 2.1 706 2.1 0.680 45.2 LOS D 0.82 NorthWest: Harris St - NW 27 L2 134 0.0 134 0.0 0.700 11.2 LOS A 16.8 118.9 0.41 0.43 0.41 45.0 28 Τ1 2437 1.8 2437 1.8 0.700 LOS A 16.8 118.9 0.37 0.36 0.37 43.1 5.8 Approach 2571 1.7 2571 1.7 0.700 6.1 LOS A 16.8 118.9 0.37 0.36 0.37 43.3 All Vehicles 3315 2.1 3315 2.1 0.700 14.6 LOS B 16.8 118.9 0.50 0.46 0.50 36.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P5	SouthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6	NorthEast Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6B	NorthEast Slip/Bypass Lane Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P7	NorthWest Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	49.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Broadway-Wattle St-Abercrombie St PD -Post Dev PM +All Modifications]

♦♦ Network: N101 [Post Dev PM + All Modifications]

+Mid-block crossing on Harris St +Road Narrowing of Harris St +Widened intersection crossing (addition of 1sec intergreen time) Site Category: -Signals - Fixed Time Isolated Cycle Time = 123 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles Mov Turn Demand Flows Arrival Flows Deg. Average Level of 95% Back of Prop. Effective Aver. Averag ID Satn Delay Service Queue Queued Stop Total Total Vehicles Distance Rate Cycles Speed veh/h veh/h km/h South: Abercrombie St 2.4 129 2.4 0.631 40.4 LOS C 20.9 148.4 0.89 0.80 0.89 28.4 1 L2 129 2 Τ1 LOS C 0.89 0.89 1494 1.3 1494 1.3 0.631 35.8 21.1 149.6 0.79 14.1 3 R2 59 1.8 59 1.8 0.631 40.3 LOS C 21.1 149.6 0.89 0.79 0.89 14.0 Approach 1682 1.4 1682 1.4 0.631 36.3 LOS C 21.1 149.6 0.89 0.79 0.89 15.9 East: Broadway - E 0.70 5 T1 1931 8.4 1931 8.4 0.596 18.8 LOS B 24.4 172.9 0.70 0.63 38.0 100. 6 0.080 0.69 0.69 R2 14 100.0 31.0 LOS C 0.5 7.1 0.69 21.7 14 0 Approach 1944 9.0 1944 0.596 18.9 LOS B 24.4 172.9 0.70 0.63 0.70 37.9 9.0 West: Broadway - W 37.0 LOS C 0.93 10 L2 627 1.2 627 1.2 0.777 32.2 227.3 0.93 0.87 25.4 11 T1 1087 8.3 1087 8.3 0.477 27.4 LOS B 15.6 117.1 0.77 0.68 0.77 28.9 Approach 1715 5.7 1715 5.7 0.777 30.9 LOS C 32.2 227.3 0.83 0.75 0.83 27.5 All Vehicles 5341 5.6 5341 28.2 LOS B 32.2 0.80 0.80 5.6 0.777 227.3 0.72 29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedes	trians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	761	57.5	LOS E	2.6	2.6	0.98	0.98
P2	East Full Crossing	201	56.1	LOS E	0.7	0.7	0.96	0.96
P3	North Full Crossing	1649	59.8	LOS E	5.9	5.9	1.02	1.02
P4	West Full Crossing	375	56.5	LOS E	1.3	1.3	0.97	0.97
P4B	West Slip/Bypass Lane Crossing	1579	59.6	LOS E	5.6	5.6	1.02	1.02
All Pe	destrians	4565	58.9	LOS E			1.01	1.01

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 102 [Wattle St-Thomas Street -Post Dev PM +All Modifications]

+Mid-block crossing on Harris St +Road Narrowing of Harris St +Widened intersection crossing (addition of 1sec intergreen time) Site Category: -Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov Turn ID		Demand Flows Arrival Flows				Deg. Satn	Average Delay	Level of Service	95% Back of Queue		Prop. Queued	Effective Stop	Aver. Averag No. e	
		Total		Total	HV					Distance		Rate	Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South: Wattle St - S														
2	T1	1061	2.3	1061	2.3	0.280	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	50.0
3a	R1	1053	0.4	1053	0.4	0.516	3.3	LOS A	0.0	0.0	0.00	0.42	0.00	44.0
3	R2	145	7.2	145	7.2	0.129	4.6	LOS A	0.0	0.0	0.00	0.50	0.00	29.9
Approach		2259	1.7	2259	1.7	0.516	1.8	NA	0.0	0.0	0.00	0.23	0.00	46.3
East: Thomas St														
6b	R3	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.29	1.83	21.8
Appro	bach	134	3.1	134	3.1	0.714	43.4	LOS D	4.0	29.0	0.94	1.29	1.83	21.8
All Vehicles		2393	1.8	2393	1.8	0.714	4.1	NA	4.0	29.0	0.05	0.29	0.10	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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