

RAINBOW BEACH

EARTHWORKS REPORT



Luke & Company Pty Ltd
June, 2010

EARTHWORKS REPORT

Prepared for:

Part 3A Concept Plan – Rainbow Beach (MP 06_0085)

Part 3A Project Application – Rainbow Beach (MP 07_0001)

**Part Lot 1232 DP 1142133, Lot 5 DP 25886 and
Lots 1,2,3 & 4 DP 1150758
Rainbow Beach, BONNY HILLS**

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On Behalf of St Vincent's Foundation Pty Ltd

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Appendices

Appendix 1 Extract from Cardno Pty Ltd - *Water Engineering and Environment Report: Appendix A: Acid Sulphate Soil Management Plan* – April 2010

Note: This is a repetition of the document contained within the Cardno Water Engineering & Environment Report and is used here for reference and ease of use of this Earthworks report.

1. Scope of Document

1.1 Scope of Document

This document is relevant to two Applications pursuant to Part 3A of the EPA Act 1979 as follows:

MP 06_0085	Rainbow Beach Concept Plan
MP 07_001	Open Space Corridor and Constructed Wetland, Rainbow Beach

1.2 Scope of Part 3A Applications

The Concept Plan application seeks consent for:

- The delineation of the limits of the residential subdivision;
- The location of the three adopted intersections with Ocean Drive;
- The location of an additional intersection with Ocean Drive currently under investigation by Council;
- The delineation of the future school sites;
- The delineation of the Greater Lake Cathie/Bonny Hills Village Centre;
- The delineation of the eco-tourist development site; and
- The delineation of the Open Space, Drainage and Wildlife Habitat Corridor.

The Project Application will seek consent for the following elements:

- Open space, drainage and wildlife habitat corridors;
- Earthworks required for constructed wetlands and to create filled reclaimed areas;
- Stormwater treatment and management; and
- District Sporting Fields.

Consent for the Project Application and the completion of the associated physical works therein, will advance the project to the point where:

- urban and residential development areas are reclaimed to inundation free levels and are appropriately protected and vegetated;
- the open space, habitat and drainage corridor areas of the site are rehabilitated with typical indigenous coastal habitats appropriate to the location and incorporating passive recreational facilities (eg pathways, cycleways, park seating, children's playgrounds and picnic areas);
- the District Sporting Fields are filled, vegetated and readied for transfer to Port Macquarie-Hastings Council.

Urban and residential development consistent with the Concept Plan will then be completed in a series of stages, each of which will be the subject of future applications. The Concept Plan is shown in Figure 1.

1.3 DGRs addressed in the report

The Director has issued Director General's Requirements (DGRs) for the Concept Plan Application and the Project Application.

The following Concept Plan (CP) Application and Project Application (PA) DGRs are addressed in this report:

- | | |
|---------------|--|
| <i>CP 2.3</i> | <i>Consider alternative sources of fill for the residential subdivision.</i> |
| <i>PA 3.1</i> | <i>Provide a detailed survey showing existing and proposed levels and quantities of fill required.</i> |
| <i>PA 3.2</i> | <i>Describe the methods for excavation, transportation and spreading of fill. Address potential alternative sources of fill required for the residential subdivision and playing fields.</i> |

2. Introduction

The subject land is located at Rainbow Beach approximately 18km south of Port Macquarie. The site is situated between the coastal villages of Lake Cathie to the north and Bonny Hills to the south. The majority of the site is undeveloped and is used primarily for agricultural activities such as cattle grazing. The location of the subject land is shown in Figure 2.

It is proposed to excavate and create a constructed wetland of approximately 10 hectares to the north east of the existing lagoon which will restore the natural flows to Duchess Gully while mitigating the effects downstream from increased runoff from developed areas. The material excavated will be utilised as fill to reclaim low-lying parts of the site for future residential areas, playing fields and a school site.

Potential acid sulphate soil (PASS) conditions are present within the site. The profile of the excavation for the wetland is designed to negate interception of identified high to very high PASS. A description of the implications for the development proposals of the presence of Acid Sulphate Soil and Potential Acid Sulphate Soil and their management, together with the Acid Sulphate Soils Management Plan is contained in the Cardno Water Engineering and Environment Report (April 2010).

The design process has used an iterative approach to balance the excavation and fill volumes while meeting the objectives of:

- stormwater treatment and detention,
- water sensitive urban design principles,
- creation of recreational opportunities and wildlife habitat,
- minimise the effect on existing fauna habitat, groundwater levels and potential acid sulphate soils, and avoiding disturbance of a designated “Aboriginal archaeological heritage exclusion zone”.

External sources of fill material as an alternative to the excavation of the wetland are discussed. The investigations carried out have determined there is no suitable source of fill nearby, and it is unacceptable in terms of the environmental and economic considerations to haul such a large volume (in excess of 400,000 cubic metres involving approximately 40,000 truck movements) of fill material long distances over the local road network.

3. Existing Conditions

3.1 The Subject Land

The subject land is as Part Lot 1232 DP 1142133, Lots 1, 2, 3 & 4 DP 1150758 and Lot 5 DP 25886. The subject land is approximately 177.4 hectares in area. The land is mostly cleared and has primarily been used for cattle grazing over many years. The mostly cleared nature of the land is shown in the aerial photograph shown in Figure 3. This aerial photo was taken in July 2009.

The subject land is characterised by high land along the northern Ocean Drive frontage falling to a broad low-lying area, subject to periodic inundation during intense storm events. The land is traversed from west to east by "Duchess Gully" which has been subject to transformation and redirection by previous owners.

Extensive investigations for the proposed development have been undertaken over many years. A previous owner carried out excavation and filling works in the early 1980's to facilitate development of a residential and tourist resort which resulted in the creation of a 4.9 hectare lagoon. Some years previously, works were carried out to divert flows from Duchess Gully to maximize the use of the land for grazing.

3.2 Existing Topography - Terrain Model

The existing ground levels range from RL 4.2m AHD on the low lying areas rising up to RL 23.3m AHD in the more elevated sections of the subject land.

4. Design Profiles

4.1 Proposed Finished Surface Levels of Future Urban Areas

The proposed finished surface levels for future urban areas within the Concept Plan are based on the following criteria:

- Minimum level at boundary of residential allotments of 0.5 metres above the calculated 1 in 100 year ARI inundation levels at that location. This concurs with Port Macquarie - Hastings Council Auspec-1 – Stormwater Drainage Design (Section D5.12: Item 4). Note: In conjunction with the 300mm freeboard to the front of the lot, this provides 800mm freeboard above the 1 in 100 year ARI inundation levels.
- Maximum depth of water on any roadway of 200mm during a 1 in 100 year ARI event. This is to ensure the roadway remains within the low hazard category as per Port Macquarie - Hastings Council Auspec-1 – Stormwater Drainage Design (Section D5.12: Item 3) during the 1 in 100 year ARI event.
- Minimum grade on residential allotments of 1.0%.
- Minimum longitudinal grade on residential streets of 0.5%.
- Stormwater and stormwater treatment train designs in line with Ecologically Sustainable Development (ESD), Water Sensitive Urban Design (WSUD) and Total Water Cycle Management (TCM) and principles as per Port Macquarie - Hastings Council AUSPEC-1 (Section D07 – Stormwater Management).
- Minimum grade on sports fields of 0.5%.

4.2 Proposed Extent of Fill for Future Urban Areas

The proposed extent of fill, for the future urban areas within the Concept Plan, is designed to achieve a balance between the available excavated material from the Constructed Wetland and the volume of fill required for the reclamation of the future urban areas to inundation free levels. A balance between the volumes of cut and fill is desirable and obviates the need to source fill material from an alternative source – as fully described within Section 7 of this report

4.3 Filling Requirement

“Project Application – Open Space Corridor and Constructed Wetland” MP 07_0001 provides for the reclamation of future urban areas to inundation-free levels.

Figure 4 shows the proposed extent and finished surface levels of the areas to be filled, based on the criteria outlined (Section 4.2). Comparing the proposed design profiles to the existing topography (Section 3.2), the volume of fill required to reclaim future urban areas to inundation free levels can be calculated.

From the measured dimensions the total fill requirement is calculated to be 417,200 cubic metres. This total can be broken down further to 347,300 cubic metres of structural fill and 69,900 cubic metres of non-structural fill (Table 1).

4.5 Excavation Quantities – Constructed Wetlands

“Project Application – Open Space Corridor and Constructed Wetland” MP 07_0001, provides for the earthworks required for the Constructed Wetland and the reclamation of the future urban areas to inundation-free levels.

A comparison between the proposed design profiles and the existing topography (Section 3.2), allows calculation of the volume of material to be excavated from the Constructed Wetlands.

The quantity of material to be excavated from the Constructed Wetlands has been calculated to be 461,120 cubic metres (Table 1). The excavated material can be categorised as follows:

Sandy alluvium (structural fill)	437,120m ³
Topsoil, average 150 thick (non-structural fill)	24,100m ³
TOTAL	461,120m ³

In addition to the 461,120 cubic metres of excavated material produced during excavation of the Constructed Wetlands, a further 57,360 cu m of topsoil (non-structural fill) shall be stripped from Fill Areas 1, 2 and 3 prior to the depositing of fill. This will result in a total of 528,480 cubic metres of bulk excavated material or **418,800 cubic metres** of net excavated material allowing for compaction losses.

4.6 Earthworks Balance

The tables below summarise the earthwork volumes required for the constructed wetlands and to reclaim future urban areas to flood free levels. These tables indicate that an earthworks balance is achieved utilising the current design.

Table 1: Earthworks Volumes

FILL VOLUMES

Location <i>(Refer to figure 4)</i>	Structural Fill <i>cu m</i>	Non-structural Fill (topsoil) <i>cu m</i>
Northern School Site – Site 1	27,300	3,340
North West Fill Area – Site 2	152,000	20,200
North East Fill Area – Site 3	168,000	14,700
Perimeter roads outer barriers	-	11,250
District Sporting Fields – Site 4	-	20,400
Total volume of consolidated fill required from measured dimensions	347,300	69,900

Total Fill 417,200 cu m

EXCAVATION VOLUMES

Location <i>Refer to figure 4</i>	Structural Fill <i>cu m</i>	Non-structural Fill (topsoil) <i>cu m</i>
Constructed Wetlands	437,120	24,000
Topsoil stripped from Fill Areas 1, 2 & 3	-	57,360
Total volume of excavated material	437,120	81,360
Allowance for compaction and other losses	20%	15%
Net material available for fill	349,700	69,100

Total 418,800 cu m

5. Acid Sulphate Soils

The majority of the areas proposed for excavation, lie within an area classified as Class 4 in Hastings LEP 2001 – Acid Sulphate Soil Maps. Port Macquarie - Hastings DCP 34 – Acid Sulphate Soils determine that Class 4 areas require all applications for excavation beyond 2 metres below the natural ground level, as in the case of the proposed wetland lagoon excavation, to be supported by an Acid Sulphate Soils Management Plan.

The Acid Sulphate Soil Management Plan (ASSMP) applicable to the site is included as Annexure A to the Cardno Water and Environment Engineering Report.

The Cardno Water and Environment Engineering Report (Section 4.8) details the:

- Site investigations undertaken for Acid Sulphate Soils.
- Clear demarcation between surface soils with “low” potential acidity (PASS) requiring little or no treatment and soils at deeper levels with “high” potential acidity requiring management and significant treatment. This boundary corresponds consistently with the occurrence of distinctive deeper clays of estuarine origin.
- Implications for the development proposals.
- ASS management and treatment.

Figure 5 shows the earthworks cross section in relation to the PASS profile.

The ASSMP stipulates that high PASS material shall be avoided during excavation works. Constructed wetland bed levels may differ to design, if areas of high PASS material are encountered. Fill may be reclaimed in other areas of the wetland if free of high PASS material, or alternatively any minor shortfall of fill may be imported to meet demand.

Additional boreholes are proposed prior to excavation, of the proposed wetland area, to confirm the high PASS profile.

6. Construction Methodology

Site management procedures are outlined in the Cardno Water Engineering and Environment Report, Appendix A - Acid Sulphate Soil Management Plan (ASSMP).

6.1 Proposed Excavation Methods

It is proposed that the excavation procedures will follow the ASSMP guidelines for excavation techniques as per the following extract from the Cardno Water Engineering and Environment Report: Appendix A, Section A6.3.

Excavation shall be undertaken in stages to ensure that groundwater drawdown associated with dewatering of the excavation areas is minimised.

Only a single excavation cell within the waterways shall be completely dewatered at a time. Each excavation cell will be separated by a section of unexcavated material and once excavation of each cell is complete the cell will be re-flooded to reduce the potential for groundwater drawdown associated with the dewatering and excavation of the adjacent cell.

Excavated slopes shall be inspected on a daily basis and shall be treated by surface sprays and liming to prevent any acid formation.

All excavation and treatment areas shall be isolated from external areas by perimeter drains and/or bunds. All waters collected on the site shall be contained, collected, tested and treated prior to disposal to external areas. All excavated material shall be subjected to appropriate testing, and treated where required.

All soils including sands and the clay strata shall be sampled during excavation. The sampling frequency shall be 1 sample per 500 cubic metres. The soils Acid Neutralising Capacity (ANC) shall also be undertaken throughout the construction phase. The samples shall be tested in accordance with the Acid Sulfate

Soils, Laboratory Methods Guidelines. The location and depth of all test samples shall be accurately recorded on a plan of the works area.

6.2 Proposed Transportation Methods

The material will be transported by motorised scrapers and/or dumper trucks, depending on the method chosen for excavation. The transport route shall be entirely within the property such that road registered vehicles are not required. The transport route shall also be selected to ensure any material lost during transport shall be contained within the acid sulphate soil treatment areas.

6.3 Proposed Spreading Methods

The base of fill areas where treated PASS are to be placed, will be treated prior to the spreading and compacting of fill as per ASSMP guidelines for the treatment of fill areas (Cardno Water Engineering and Environment Report: Appendix A, Section A6.6). The material will be spread and compacted using graders and rollers in accordance with the requirements of AS3798-2007 – *Guidelines on earthworks for commercial and residential developments*.

7. Alternative Sources of Fill Material

7.1 Alternative Sources of Fill Material

The volume of structural fill material required for the proposed roads, residential building sites and the school site is calculated to be 347,300 cubic metres. Applying a bulking factor of at least 20% indicates approximately 416,000 cu metres of 'structural' fill would need to be imported to the site by road transport.

Extensive enquiries have been made, and only two possible external sources of this quantity of fill material have been identified, namely:

- i) Hurd's Quarry, Ocean Drive, Bonny Hills located approx. 5km from the site. This quarry has suitable material but it is of road

pavement quality, not generally used for bulk filling, and is very expensive. Other material of lesser quality is available from the quarry but not in the quantities required.

- ii) Bago Quarry, Milligans Road, Wauchope, operated by Volcanic Resources Pty Ltd. The quarry advises to have sufficient fill material available, but is located approximately 21km from the site.

The location of the identified two possible sources of fill material is shown on Figure 6.

7.2 Alternate Fill Procurement Costs

The procurement of fill material from the closer Hurd's Quarry is not considered financially feasible due to the insufficient quantity of material in the quarry suitable for use as bulk filling, and thus the need to use the more expensive road pavement quality material.

A costing exercise has been carried out based on the delivery of material from the more distant Bago Quarry.

The estimated cost of material supplied to the site by Bago Quarry (as at June 2007) is \$17 per cubic metre (loose truck measure). Therefore the total cost of obtaining structural fill material from Bago Quarry is estimated to exceed \$5.8 million, which is economically prohibitive.

7.3 Environmental, Traffic and General Amenity Impacts

Obtaining fill material from Bago Quarry would have a significant impact on the environment, traffic and general amenity of areas along the haulage route. The material would be hauled using "truck and dog trailer" combinations which deliver approximately 22 cubic metres (loose truck measure) per trip. More than 40,000 trips would be required to supply the required amount of fill.

The route is firstly along the gravel surface of Milligans Road. The increased truck movements on this road would result in increased dust impacting on adjacent flora and fauna.

The route then uses Bago Road resulting in an increased maintenance requirement on that road.

From Bago Road, trucks will then turn onto the Pacific Highway and into Houston Mitchell Drive. At both intersections additional truck turning movements will increase the hazard to other vehicles at these intersections.

Trucks would then utilise Houston Mitchell Drive, which has a poor alignment and surface condition in some sections. The increased truck movements on this road would result in reduced safety to other road users and an increase in the required maintenance of the road.

7.4 Alternate Sources Summary

From the above, a number of alternative sources of fill material were investigated and only one preferable source has been identified - Bago Quarry.

Transportation of the fill material from Bago Quarry would involve an estimated 40,000 truck movements. These trips result in some 840,000 kilometres of travel by heavy vehicles, and consequent significant energy usage, emissions, damage to local roads and reduced safety for other road users.

It is concluded that the estimated economic, social and environmental cost of importing fill material is prohibitive.

The use of material sourced from the Constructed Wetland, which is more environmentally and economically feasible compared to the importation of material, as well as fulfilling a role in water quality improvement and mitigation of inundation.

8. Conclusion

- 8.1 The two Part 3A applications will advance the project to the point where future urban areas are reclaimed to inundation free levels.
- 8.2 The geotechnical investigations have determined that sufficient suitable structural material is available from the on-site excavation of the Constructed Wetlands to achieve the required fill levels in the proposed location.
- 8.3 The Cardno Water and Environment Engineering Report determined that the Constructed Wetland fulfils a role in water quality improvement and mitigation of flooding. The Constructed Wetland is also an integral component of the Open Space, Drainage and Wildlife Habitat Corridor.
- 8.4 The existing and proposed levels are shown on the "Terrain Model" and also the "Proposed Design Profile" Figures included within this report. **(DGR: PA 3.1)**
- 8.5 Alternative sources of fill material have been investigated, but sources of fill are limited and neither environmentally nor socially acceptable. The importation of fill has also been shown to be environmentally and economically prohibitive. **(DGR: CP 2.3 & PA 3.2)**
- 8.6 The majority of the areas for excavation, within the proposed development, lie within an area classified as Class 4 in Hastings LEP 2001 – Acid Sulphate Soil Maps.
- 8.7 The proposed methods for excavation, transportation and spreading of fill addressed in this report will comply with the Acid Sulphate Soil Management Plan (ASSMP) applicable to the site, included as Annexure A to the Cardno Water and Environment Engineering Report. **(DGR: PA 3.2)**