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22 March 2011

Ms Nicole Woodrow Landcom Level 2 330 Church Street Parramatta NSW 2150

Dear Ms Woodrow

North Penrith Development - Testing of RTA and Penrith Council Requested Intersection Changes and Parking Strategy

A Transport Mobility and Accessibility Plan was produced to support an application made in October 2010 under Part 3A of the EP&A Act for a Concept Plan and Project Application for the North Penrith Development (NPD). Responses to the application and the TMAP have been received from residents and government agencies, including the Roads and Traffic Authority (RTA) and Penrith City Council (PCC). These latter two responses, amongst other issues, requested the following amendments:

- the upgrading of the intersection of Castlereagh Road, Grace Drive and Peachtree Road to permit all traffic movements (RTA and PCC)
- the installation of a one lane roundabout at the new intersection of Daniel Woodriff Drive and Grace Drive (RTA and PCC)
- an on-street parking strategy (PCC).

The results of our additional analysis to respond adequately to these requests are contained in this letter.

1. Full access at the intersection of Castlereagh Road, Peachtree Road and Grace Drive

The RTA and PCC have requested that the intersection of Castlereagh Road and Peachtree Road, which is currently restricted to left in/left out on the incomplete eastern approach, be upgraded to enable all traffic movements (excluding U-turns) when Grace Drive is extended through to connect Castlereagh Road and Daniel Woodriff Drive. This upgrade would improve access to the Commuter Car Park, as well as the NPD.





Allowing the additional traffic movements would have the following flow-on consequences:

- increased traffic on Grace Drive
- reduced traffic using the Castlereagh Road/Coreen Avenue/Mullins Street roundabout
- higher turning movements at the new intersection of Daniel Woodriff Drive and Grace Drive
- reduced turning movements at the intersection of Coreen Avenue and Daniel Woodriff Drive.

These roads, and the affected intersections, are shown in Figure 1.1.



Figure 1.1 North Penrith Development map showing intersections for further investigation

For the intersection of Castlereagh Road, Peachtree Road and Grace Drive itself, allowing the additional turns would:

- reduce the green-time available for the heavy southbound through-movement on Castlereagh Road
- require more complex phasing due to the opposing right turn movements on Peachtree Road and Grace Drive.



To test the impact of allowing the additional turning movements, the spreadsheets used to forecast traffic flows for the Transport Mobility and Accessibility Plan (TMAP) for the NPD Part 3A Application were adjusted to divert traffic along the new paths potentially created by the upgrade. It was assumed that all commuter car park traffic travelling via Castlereagh Road would use the Peachtree Road intersection instead of travelling via Coreen Avenue as motorists would save time and avoid congestion. Similarly, NPD generated traffic was also assumed to use the Grace Drive/Peachtree Road intersection if it provided a shorter travel distance for their trip.

Two traffic volume scenarios were assessed (as was done in the TMAP):

- 2026 existing traffic, plus base traffic growth, plus traffic from other developments and traffic generated by the North Penrith Development
- 2026 existing traffic, plus base traffic growth and traffic generated by the North Penrith Development.

The second traffic scenario was tested as developments included in the first scenario in the vicinity of North Penrith are not guaranteed to proceed. If they do not proceed, traffic forecasts including traffic from these developments, would be too high and may result in excessive road upgrades contrary to the traffic management targets in central Penrith.

The resulting traffic volumes were modelled in the SIDRA intersection analysis software. The intersection layouts assumed were:

- Castlereagh Road, Coreen Avenue and Mullins Street:
 - existing two-lane roundabout
- Castlereagh Road, Peachtree Road and Grace Drive:
 - add 150 m long right turn bay on Castlereagh Road, northbound
 - remove traffic island preventing right turns
 - delineate one left turn lane and one right turn lane on Grace Drive.

The results of the preliminary SIDRA modelling are shown in Table 1.1 for 2026 AM and PM peaks. Detailed results are provided in Appendix A.

Intersection	Intersection control	DoS		Average Delay (sec/veh)	LoS	95 th %ile Queue (m)				
2026 Base + Other Development + North Penrith Development traffic										
Castlereagh Road/	Roundabout	AM	1.83	820	LoS F	> 200				
Coreen Avenue	Roundabout	PM	1.41	827	LoS F	> 200				
Castlereagh Road/	Circala	AM	1.33	222	LoS F	> 200				
Peachtree Road	Signals	PM	1.14	94	LoS F	> 200				
2026 Base + North Penrith Development traffic										
Castlereagh Road/	Devedebevit	AM	1.00	112	LoS F	140				
Coreen Avenue	Roundabout	PM	0.83	20	LoS B	110				
Castlereagh Road/	Circala	AM	0.92	38	LoS C	> 200				
Peachtree Road	Signals	PM	0.81	33	LoS C	> 200				

Table 1.1 SIDRA intersection model results - 2026

The results for both intersections in the '2026 Base + Other Development + NPD' scenario indicate poor performance and long delays. A closer inspection of the traffic volumes forecast on Castlereagh Road show that they are too high to be accommodated in two lanes with interruptions to the flow at the intersections. This is due to the traffic generated by other developments, such as Penrith Lakes and Lakes Environs. These high flows would require the addition of an additional through lane in each direction at both intersections.

The results also show that the intersection of Castlereagh Road, Coreen Avenue and Mullins Street is experiencing excessive delays during the morning peak under the '2026 Base + NPD' scenario. The detailed results show that the Coreen Avenue approach is experiencing long delays, forecasting future poor performance. This is due to the large traffic volume southbound on Castlereagh Road. The volumes on Mullins Street, and the northbound right turn on Castlereagh Road, are not large enough to create sufficient gaps in the southbound flow for Coreen Avenue traffic to join the flow.

Our earlier analysis did not recommend this solution for the intersection because our original results indicated that it would delay the heavy southbound traffic movement, and it would not reduce the magnitude of the intersection upgrade required at the intersection of Castlereagh Road and Coreen Avenue. Allowing all movements at the Castlereagh Road/Peachtree Road intersection would have the benefits of reducing traffic on other roads and improving access to the Commuter Car Park and NPD (including the supermarket for semi-trailers). However, if this measure is to be added to the project, the following intersection upgrades would be required to reduce the forecast delays and achieve acceptable intersection performance. The proposed treatments to address the problems forecast from the requested upgrade would include:

2026 Base + Other Development + North Penrith Development traffic scenario

Castlereagh Road, Coreen Avenue and Mullins Street:



- convert two-lane roundabout to traffic signals
- additional through lanes on Castlereagh Road in each direction, 100 m on approach and departure around the intersection
- two 150 m long right turn bays on Castlereagh Road northbound
- one through lane, one 75 m long left turn bay and one 150 m long right turn bay on Coreen Avenue
- one 150 m long left turn slip lane and one 70 m long right turn bay on Castlereagh Road southbound
- one shared through right lane and one 60 m long shared left through lane on Mullins Road
- Castlereagh Road, Peachtree Road and Grace Drive:
 - additional through lanes on Castlereagh Road in each direction, 100 m on approach and departure
 - add 150 m long right turn bay on Castlereagh Road, northbound
 - remove traffic island preventing right turns
 - linemark one left turn lane and one right turn lane on Grace Drive.

2026 Base + North Penrith Development traffic scenario

- Castlereagh Road, Coreen Avenue and Mullins Street:
 - retain two-lane roundabout
 - install roundabout metering on northern approach (Castlereagh Road) to create gaps for Coreen Avenue traffic
 - install queue detector loops on Coreen Avenue to trigger metering
 - install queue detector loops on Castlereagh Road, southbound to cancel metering if queues grow too far.

The roundabout metering described above would prolong the life of the roundabout without the need for an expensive conversion to traffic signals. The problem of excessive delays on Coreen Avenue was only forecast to occur during the morning peak. For the rest of the day, the roundabout was estimated to remain effective.

The results of SIDRA intersection modelling with the upgrades listed above in place is shown in Table 1.2. Detailed results are provided in Appendix B.

Intersection	Intersection control	Time period	DoS	Average Delay (sec/veh)	LoS	95 th %ile Queue (m)			
2026 Base + Other Development + North Penrith Development traffic									
Castlereagh Road/	Roundabout	AM	0.84	30	LoS C	> 200			
Coreen Avenue	Roundabout	PM	0.91	39	LoS C	> 200			
Castlereagh Road/	Signala	AM	0.94	44	LoS D	> 200			
Peachtree Road	Signals	PM	0.86	31	LoS C	> 200			
2026 Base + North Penrith Development traffic									
Castlereagh Road/	Roundabout	AM	0.82	50	LoS D	> 200			
Coreen Avenue	(Metering)	PM	0.84	23	LoS B	> 200			

Table 1.2 SIDRA intersection model results – 2026 with upgrades

2. New intersection of Daniel Woodriff Drive and Grace Drive

This intersection would be located on the existing commuter car park access road (renamed Daniel Woodriff Drive) where the Peachtree Road extension (Grace Drive) connects. This intersection has been planned with a four-way give-way controlled intersection, with Daniel Woodriff receiving priority. PCC and the RTA have expressed concern that a roundabout would be required to adequately control traffic.

Upgrading the Castlereagh Road, Peachtree Road and Grace Drive intersection to permit all movements, is forecast to increase the turning volumes at this intersection.

The intersection has been modelled in SIDRA with both give-way and one-lane roundabout control. The results are shown in Table 2.1 (detailed results included in Appendix A and B respectively).

Table 2.1 SIDRA intersection model results – 2026 for the Daniel Woodriff Drive and Grace Drive intersection

Intersection	Intersection control	DoS Delay		Delay	LoS	95 th %ile Queue (m)
Daniel Woodriff Drive/	Give-way	AM	0.59	16	LoS B	41
Grace Drive	sign	PM	0.38	14	LoS A	17
Daniel Woodriff Drive/	Devue de la cust	AM	0.25	12	LoS A	14
Grace Drive	Roundabout	PM	0.28	11	LoS A	18

The results show that the intersection can operate well with give-way or roundabout control, and hence upgrading this intersection to a one-lane roundabout is not justified on traffic performance grounds.





Installing a one-lane roundabout at this intersection would have advantages and disadvantages, including:

Advantages

- speed reduction along the straight Daniel Woodriff Drive
- if built large enough to accommodate the turning movements of semi-trailers, it could provide improved access for servicing the new supermarket.

Disadvantages:

- less safety for pedestrians and cyclists
- additional cost
- increased land-take, in both Landcom land and the adjoining property.

3. Road network upgrade costing

A concept level engineering cost estimate has been prepared for each of the upgrades proposed. The rates, contingencies and method used are the same as for the TMAP. The estimated costs are shown in Table 3.1.

These are concept level budget costs only, prepared using standard unit rates based on previous projects. Rates for small-scale projects assume that they would be undertaken as part of a wider works program.

In addition, we have allowed a 77% mark-up to cover overheads, margin and contingency, as follows:

•	Traffic Control	8%
•	Public utility plant relocation and/or protection	5%
•	Contractor's Overheads	18%
•	Contractor's margin	10%
•	Design	4%
•	Project Management	7%
•	Risk and Contingency	25%
•	Total % allowance	77%



No allowance has been made for the cost of land acquisition or on-going maintenance costs (due to lack of information). All costs are in \$Australian dollars and are in 2010 values.

The estimates are based upon information made available to Parsons Brinckerhoff (PB) at the time of preparing the estimates. The estimates have been prepared for this specific Client and Project, and should not be used or relied on for any other use. PB accepts no liability for actual costs varying from those estimated.

Intersection	Recommended upgrades (in addition to existing layout)	Estimated cost
2026 Base + Other Dev	elopment + North Penrith Development traffic	
Castlereagh Road/ Coreen Avenue	 convert two-lane roundabout to traffic signals; 	\$3,240,000
Obleen Avenue	 additional through lanes on Castlereagh Road in each direction, 100 m on approach and departure 	
	 two 150 m long right turn bays on Castlereagh Road northbound 	
	 one through lane, one 75 m long left turn bay and one 150 m long right turn bay on Coreen Avenue 	
	 one 150 m long left turn slip lane and one 70 m long right turn bay on Castlereagh Road southbound 	
	 one shared through right lane and one 60 m long shared left through lane on Mullins Road 	
Castlereagh Road & Peachtree Road	 additional through lanes on Castlereagh Road in each direction, 100 m on approach and departure 	\$1,540,000
	 add 150 m long right turn bay on Castlereagh Road, northbound 	
	 remove traffic island preventing right turns 	
	 delineate one left turn lane and one right turn lane on Grace Drive 	
2026 Base + North Pen	ith Development traffic	
Castlereagh Road/	 retain two lane roundabout 	\$170,000
Coreen Avenue	 install roundabout metering on northern approach (Castlereagh Road) to create gaps for Coreen Avenue traffic 	
	 install queue detector loops on Coreen Avenue to trigger metering 	
	 install queue detector loops on Castlereagh Road, southbound to cancel metering if queues grow too far 	
Castlereagh Road &	 add 150 m long right turn bay on Castlereagh Road, northbound 	\$350,000
Peachtree Road	 remove traffic island preventing right turns 	
	 delineate one left turn lane and one right turn lane on Grace Drive 	
Both traffic scenarios		
Daniel Woodriff Drive/ Grace Drive	 install new in one lane roundabout (in comparison to the cost of the give-way intersection) 	+ \$350,000

Table 3.1	Description and estimated cost of intersection up	grades
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The need for these infrastructure upgrades is driven by several sources, including background growth, other developments, the expansion of the commuter car park and the NPD. For the 2026 Base + Other Development + North Penrith Development traffic scenario, the relative percentage of all three sources are used to calculate the contribution proportion. For the 2026 Base + North Penrith Development traffic scenario, only background growth and NPD traffic are used in the calculations. Both options have been calculated with no contribution from existing sources of traffic.

The need for the roundabout upgrade at the intersection of Daniel Woodriff Drive and Grace Drive has not been established on traffic capacity grounds, as a give-way controlled intersection would work just as well. The roundabout would be used by NPD and commuter car park traffic, so the contribution has been apportioned based on the amount of NPD traffic compared to total traffic. The remaining contribution would be made by PCC on behalf of the commuter car park traffic. The funding contributions for the additional upgrades are shown in Table 3.2.

Upgrade	Cost estimate	Apportionment	Contribution							
2026 Base + Other Development + North Penrith Development traffic										
Castlereagh Road/ Coreen Avenue	\$3,240,000	10%	\$318,000							
Castlereagh Road & Peachtree Road	\$1,540,000	14%	\$223,000							
2026 Base + North Penrith D	evelopment traffic									
Castlereagh Road/ Coreen Avenue	\$170,000	38%	\$65,000							
Castlereagh Road & Peachtree Road	\$350,000	57%	\$199,000							
Both traffic scenarios										
Daniel Woodriff Drive/ Grace Drive (Roundabout)	\$350,000	56%	\$196,000							

Table 3.2 Contribution to road and intersection upgrades

The full set of road upgrades for the 2026 Base plus NPD traffic scenario are shown in Table 3.3.



Upgrade	Cost estimate	Apportionment	Contribution
2026 Base + North Penrith Developme	ent traffic		
Parker Street/Oxford Street/Coreen Avenue & Richmond Road	\$300,000	40%	\$122,000
Coreen Avenue & Coombes Drive	\$25,000	58%	\$15,000
Coreen Avenue & Site Boulevard	\$770,000	100%	\$770,000
Coreen Avenue & Daniel Woodriff Drive	\$30,000	100%	\$30,000
Castlereagh Road/ Coreen Avenue	\$170,000	38%	\$65,000
Castlereagh Road & Peachtree Road	\$350,000	57%	\$199,000
Daniel Woodriff Drive/ Grace Drive (Roundabout)	\$350,000	56%	\$196,000
Total	\$1,645,000	73%	\$1,199,000

Table 3.3 Contribution to road and intersection upgrades

For comparison, the previous contribution for this scenario (estimated at the time of the TMAP production) was \$1,057,000.

4. NPD on-street parking strategy

The TMAP discussed a strategy for public off-street, private off-street and on-street parking. PCC have asked for a more substantial plan for on-street parking. The section below proposes an on-street parking strategy that restricts use by commuters, creates parking turn-over for businesses and allows for visitors to residential properties. The ultimate authority on parking decisions rests with the Penrith City Council and the local traffic committee.

The on-street parking strategy is built on the following objectives:

- provide for regulatory No Stopping zones
- provide for the needs of public transport services (bus stops and taxi ranks)
- loading activities will be accommodated off-street
- provide short-stay parking for people shopping, undertaking personal business or attending meetings
- protect on-street parking from being inefficiently taken up by long-stay commuters using Penrith Station.



The attached map shows the proposed on-street parking space distribution. This would need to be reviewed and submitted to the local traffic committee before being prepared and implemented during the construction of the estate. A description of the on-street parking types is provided in Table 4.1.

Parking type	Description
No Stopping	Statutory requirements and roads with limited width
Bus Zone	At bus stops
Taxi Zone	At taxi rank alongside Station Square
5 Minute Parking	Passenger drop-off
2 hour limit 8.30 pm–6.00 pm Monday to Friday, 8.30 am to 12.30 pm Saturday	Promote parking turn-over, activate village centre streets, support reduced off-street private parking provision
Loading Zone 8.30 pm–6.00 pm Monday to Friday, 8.30 am to 12.30 pm Saturday	On-street servicing requirements
4 hour limit 8.30 am–6.00 pm Monday to Friday	Parking adjacent to residential areas, allows visitors, tradespeople but discourages commuter parking

Table 4.1On-street parking types

Further information on the parking strategy was presented in the TMAP. It was originally proposed that the residential streets outside the Village Centre have unrestricted parking, with the possible application of 4 hour parking during weekday business hours if commuter parking intrusion became a problem. To avoid problems before they eventuate, a parking designation of 4 hour limit 8.30 am–6.00 pm Monday to Friday, unrestricted at other times, could be applied from the outset. This could be supplemented by a resident parking scheme, if this conforms with Council practice.

Parking within the Village Centre (on-street and public off-street parking) is recommended to be controlled by a 2 hour limit 8.30 pm–6.00 pm Monday to Friday, 8.30 am to 12.30 pm Saturday. Metered parking is not current practice in Penrith Town Centre, but could be introduced in the future. It is envisioned that parking meters would be utilised for on-street kerbside parking to manage excess parking demand once they were endorsed for use generally.

Please let me know if you have any questions regarding this information.

Yours sincerely

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Tom van Drempt Senior Transport Engineer Parsons Brinckerhoff Australia Pty Limited

Encls.



Parking Plan

On-street I





12/13 2108234A-LT_2950

Appendix A – SIDRA Results – No Upgrades

Castlereagh Rd / Peachtree Rd



AM Movement Summary

Mov ID	Turn	Demond	1.11.7	Den Cete	A	Level of	OF0/ Deel		Dren	Effective.	A
	Turn	Demand	ΠV	Deg. Satn	Average			of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	astlerea	agh Rd (S)									
1	L	118	5.4	0.937	37.7	LOS C	54.5	399.2	0.84	1.05	30.8
2	Т	1594	5.4	0.936	27.6	LOS B	56.0	410.2	0.85	0.87	32.4
3	R	189	4.3	1.310	377.0	LOS F	34.0	246.6	1.00	1.56	5.3
Approac	h	1901	5.3	1.310	62.9	LOS E	56.0	410.2	0.86	0.95	21.4
East: Pe	achtree	Rd (E)									
4	L	133	4.0	0.275	54.1	LOS D	9.3	67.6	0.83	0.79	24.1
5	Т	1	0.0	0.275	45.8	LOS D	9.3	67.6	0.83	0.68	24.6
6	R	28	3.7	0.393	89.0	LOS F	3.2	22.8	1.00	0.72	17.5
Approac	h	162	3.9	0.393	60.2	LOS E	9.3	67.6	0.86	0.78	22.6
North: C	astlerea	agh Rd (N)									
7	L	140	3.8	0.299	19.0	LOS B	5.2	37.6	0.41	0.73	39.5
8	Т	2857	3.8	1.325	357.6	LOS F	270.8	1957.0	1.00	2.22	5.5
9	R	122	3.4	0.562	77.1	LOS F	10.3	74.1	0.99	0.89	19.2
Approac	h	3119	3.8	1.325	331.5	LOS F	270.8	1957.0	0.97	2.10	5.9
West: Pe	eachtree	eRd(W)									
10	L	38	8.3	0.086	53.2	LOS D	3.1	23.3	0.79	0.74	24.4
11	Т	1	0.0	0.086	44.6	LOS D	3.1	23.3	0.79	0.60	23.4
12	R	40	7.9	0.569	90.6	LOS F	4.4	32.6	1.00	0.75	17.3
Approac	h	79	8.0	0.569	72.0	LOS F	4.4	32.6	0.90	0.75	20.2
All Vehic	les	5261	4.4	1.325	222.2	LOS F	270.8	1957.0	0.93	1.63	8.4

veh/h % v/c sec veh m per veh k 1 L 77 2.7 1.130 149.2 LOS F 129.2 928.1 1.00 1.38 1 2 T 2322 3.0 1.135 143.3 LOS F 158.6 1139.3 1.00 1.49 1 3 R 114 2.8 1.040 150.6 LOS F 136. 97.6 1.00 1.15 143.8 LOS F 158.6 1139.3 1.00 1.47 Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E) 2.7 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47	erage
South: Castlereagh Rd (S) 1 L 77 2.7 1.130 149.2 LOS F 129.2 928.1 1.00 1.38 2 T 2322 3.0 1.135 143.3 LOS F 158.6 1139.3 1.00 1.49 3 R 114 2.8 1.040 150.6 LOS F 136 97.6 1.00 1.15 Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E) 143.8 LOS F 158.6 1139.3 1.00 1.47	peed
1 L 77 2.7 1.130 149.2 LOS F 129.2 928.1 1.00 1.38 2 T 2322 3.0 1.135 143.3 LOS F 158.6 1139.3 1.00 1.49 3 R 114 2.8 1.040 150.6 LOS F 136. 97.6 1.00 1.15 Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E)	km/h
2 T 2322 3.0 1.135 143.3 LOS F 158.6 1139.3 1.00 1.49 3 R 114 2.8 1.040 150.6 LOS F 13.6 97.6 1.00 1.15 Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E) 158.6 1139.3 1.00 1.47	
3 R 114 2.8 1.040 150.6 LOS F 13.6 97.6 1.00 1.15 Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E) Image: Construct on the second	12.0
Approach 2513 3.0 1.135 143.8 LOS F 158.6 1139.3 1.00 1.47 East: Peachtree Rd (E)	11.9
East: Peachtree Rd (E)	11.7
	11.9
4 L 229 2.8 0.504 59.8 LOSE 15.7 112.8 0.91 0.83	
	22.7
5 T 1 0.0 0.490 51.5 LOS D 15.7 112.8 0.91 0.77	23.0
6 R 115 2.8 0.859 92.3 LOS F 10.9 78.1 1.00 0.93	17.0
Approach 345 2.7 0.859 70.6 LOS F 15.7 112.8 0.94 0.86	20.4
North: Castlereagh Rd (N)	
7 L 73 1.4 0.146 17.3 LOSB 2.6 18.7 0.37 0.71	40.7
8 T 1791 2.9 0.845 29.7 LOS C 54.0 387.1 0.91 0.85	31.5
9 R 91 2.3 0.826 92.0 LOS F 8.9 63.6 1.00 0.90	17.0
Approach 1954 2.8 0.845 32.1 LOS C 54.0 387.1 0.90 0.84	30.5
West: Peachtree Rd (W)	
10 L 163 2.6 0.370 57.8 LOSE 11.8 84.3 0.87 0.81	23.2
11 T 6 0.0 0.370 49.5 LOS D 11.8 84.3 0.87 0.72	22.1
12 R 142 2.9 1.060 165.0 LOS F 17.1 122.9 1.00 1.19	10.9
Approach 311 2.7 1.060 106.5 LOS F 17.1 122.9 0.93 0.98	15.3
All Vehicles 5122 2.9 1.135 94.0 LOS F 158.6 1139.3 0.95 1.16	16.4

Castlereagh Rd / Coreen Ave



AM Movement Summary

Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h		v/c	sec		veh			per veh	km/h
South: (Castlere	agh Rd (S)									
1	L	85	8.6	0.618	7.3	LOS A	7.7	56.6	0.65	0.61	48.5
2	Т	1487	5.9	0.616	5.9	LOS A	7.7	56.6	0.65	0.53	48.4
3	R	208	6.1	0.617	12.8	LOS A	7.7	56.5	0.65	0.74	46.2
Approa	ch	1781	6.1	0.616	6.8	LOS A	7.7	56.6	0.65	0.55	48.1
East: C	oreen A	ve (E)									
4	L	170	15.9	1.134	271.1	LOS F	25.6	203.4	1.00	2.32	7.1
5	Т	104	3.0	1.828	820.8	LOS F	91.5	685.5	1.00	3.53	2.6
6	R	161	11.1	1.830	827.9	LOS F	91.5	685.5	1.00	3.49	2.7
Approa	ch	444	11.1	1.828	612.3	LOS F	91.5	685.5	1.00	3.04	3.5
North: 0	Castlerea	agh Rd (N)									
7	L	366	4.0	1.156	155.6	LOS F	164.0	1199.2	1.00	4.20	11.4
8	Т	2709	5.5	1.156	154.5	LOS F	164.0	1200.0	1.00	4.20	11.5
9	R	51	18.8	1.148	162.5	LOS F	163.1	1200.0	1.00	4.08	12.0
Approa	ch	3126	5.6	1.156	154.8	LOS F	164.0	1200.0	1.00	4.19	11.5
West: N	/ullins R	d (W)									
10	L	41	5.1	0.149	17.0	LOS B	1.0	7.4	0.90	0.95	41.1
11	Т	32	6.7	0.255	17.1	LOS B	1.7	12.9	0.90	0.94	40.1
12	R	42	12.5	0.255	24.4	LOS B	1.7	12.9	0.91	0.98	37.8
Approa	ch	115	8.3	0.255	19.8	LOS B	1.7	12.9	0.90	0.96	39.5
All Vehi	icles	5466	6.2	1.828	140.9	LOS F	164.0	1200.0	0.88	2.84	12.5
	-	-									

Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average	Level of Service	95% Back		Prop. Queued	Effective Stop Rate	Average Speed
					Delay	Service	Vehicles	Distance	Queuea		
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Castlerea	agh Rd (S)									
1	L	98	3.2	1.419	381.8	LOS F	311.6	2234.5	1.00	8.11	5.2
2	Т	2571	2.8	1.411	381.2	LOS F	311.6	2234.5	1.00	7.90	5.3
3	R	286	7.7	1.410	389.1	LOS F	269.4	1948.0	1.00	7.54	5.6
Approac	h	2955	3.3	1.410	382.0	LOS F	311.6	2234.5	1.00	7.87	5.3
East: Co	reen Av	/e (E)									
4	L	157	3.4	0.498	17.5	LOS B	3.6	26.0	0.91	1.02	40.6
5	Т	127	6.6	0.801	23.8	LOS B	9.0	65.4	1.00	1.24	35.6
6	R	242	3.9	0.802	30.6	LOS C	9.0	65.4	1.00	1.24	34.2
Approac	h	526	4.4	0.801	25.1	LOS C	9.0	65.4	0.97	1.18	36.2
North: C	astlerea	agh Rd (N)									
7	L	234	2.7	0.823	11.4	LOS A	14.5	104.6	0.89	0.90	46.6
8	Т	1595	3.4	0.822	10.7	LOS A	14.5	104.6	0.90	0.92	46.3
9	R	58	5.5	0.827	18.6	LOS B	14.3	103.1	0.92	1.00	42.5
Approac	h	1886	3.3	0.822	11.0	LOS B	14.5	104.6	0.90	0.92	46.2
West: M	ullins R	d (W)									
10	L	102	2.1	0.580	37.6	LOS C	4.4	31.5	0.97	1.10	29.5
11	Т	85	2.5	0.508	24.6	LOS B	4.6	33.1	1.00	1.09	35.4
12	R	60	3.5	0.508	31.5	LOS C	4.6	33.1	1.00	1.09	34.1
Approac	h	247	2.6	0.581	31.6	LOS C	4.6	33.1	0.99	1.09	32.5
All Vehic		5615	3.4	1.410	208.5	LOS F	311.6	2234.5	0.96	4.61	9.0

Castlereagh Rd / Peachtree Rd



AM Movement Summary

Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service		of Queue	Prop. Queued	Effective Stop Rate	Average Speed
						Service	Vehicles	Distance	Queueu		
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (Castlere	agh Rd (S)									
1	L	118	5.4	0.680	19.1	LOS B	16.5	120.7	0.35	1.01	40.4
2	Т	1194	5.7	0.681	9.5	LOS A	17.5	128.5	0.36	0.35	46.1
3	R	221	4.3	0.920	95.5	LOS F	19.8	143.3	1.00	0.99	16.6
Approa	ch	1533	5.5	0.920	22.7	LOS B	19.8	143.3	0.46	0.50	36.4
East: Pe	eachtree	Rd (E)									
4	L	133	4.0	0.233	48.2	LOS D	8.8	63.7	0.78	0.79	25.8
5	Т	1	0.0	0.237	39.8	LOS C	8.8	63.7	0.78	0.64	26.5
6	R	28	3.7	0.393	89.0	LOS F	3.2	22.8	1.00	0.72	17.5
Approa	ch	162	3.9	0.393	55.3	LOS D	8.8	63.7	0.82	0.77	23.8
North: C	Castlerea	agh Rd (N)									
7	L	140	3.8	0.334	22.4	LOS B	5.9	42.6	0.47	0.74	37.2
8	Т	1791	3.4	0.914	47.0	LOS D	67.3	485.2	1.00	1.00	25.1
9	R	122	3.4	0.565	74.5	LOS F	10.2	73.2	0.98	0.80	19.7
Approad	ch	2053	3.4	0.914	46.9	LOS D	67.3	485.2	0.96	0.97	25.3
West: P	eachtre	e Rd (W)									
10	L	38	8.3	0.075	48.4	LOS D	2.9	22.0	0.75	0.74	25.8
11	Т	1	0.0	0.075	39.8	LOS C	2.9	22.0	0.75	0.57	24.8
12	R	40	7.9	0.569	90.6	LOS F	4.4	32.6	1.00	0.75	17.3
Approa	ch	79	8.0	0.569	69.6	LOS E	4.4	32.6	0.88	0.74	20.6
All Vehi	cles	3826	4.4	0.920	38.0	LOS C	67.3	485.2	0.75	0.77	28.6
DM N		mant C.									

Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South:	Castlere	agh Rd (S)									
1	L	77	2.7	0.757	25.5	LOS B	22.9	164.7	0.58	0.99	36.6
2	Т	1256	3.1	0.756	15.0	LOS B	23.7	170.5	0.59	0.55	40.7
3	R	114	2.8	0.812	79.2	LOS F	9.5	68.2	1.00	0.90	18.9
Approa	ich	1446	3.1	0.812	20.6	LOS B	23.7	170.5	0.62	0.60	37.1
East: P	eachtree	e Rd (E)									
4	L	229	2.8	0.425	47.5	LOS D	13.2	94.9	0.85	0.82	26.0
5	Т	1	0.0	0.425	39.2	LOS C	13.2	94.9	0.85	0.72	26.5
6	R	115	2.8	0.630	71.2	LOS F	9.0	64.5	1.00	0.81	20.4
Approa	ich	345	2.7	0.630	55.4	LOS D	13.2	94.9	0.90	0.81	23.8
North: (Castlerea	agh Rd (N)									
7	L	73	1.4	0.148	19.1	LOS B	2.7	19.1	0.43	0.72	39.3
8	Т	1423	2.9	0.806	32.3	LOS C	38.6	277.1	0.93	0.85	30.3
9	R	91	2.3	0.644	74.7	LOS F	7.6	54.0	1.00	0.80	19.7
Approa	ich	1586	2.8	0.805	34.1	LOS C	38.6	277.1	0.91	0.84	29.7
West: F	Peachtre	e Rd (W)									
10	L	163	2.6	0.304	45.8	LOS D	9.7	69.3	0.82	0.80	26.5
11	Т	1	0.0	0.297	37.5	LOS C	9.7	69.3	0.82	0.67	25.3
12	R	146	2.9	0.804	76.3	LOS F	11.5	82.8	1.00	0.90	19.4
Approa	ich	311	2.7	0.804	60.1	LOS E	11.5	82.8	0.90	0.84	22.6
All Veh	icles	3688	2.9	0.812	33.0	LOS C	38.6	277.1	0.79	0.74	30.6

Castlereagh Rd / Coreen Ave



AM Movement Summary

Mov ID	Turn	Demand	HV I	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South: (Castlere	agh Rd (S)									
1	L	85	8.6	0.561	8.0	LOS A	6.4	47.3	0.75	0.67	48.0
2	Т	1120	5.9	0.561	6.6	LOS A	6.4	47.3	0.75	0.59	47.6
3	R	208	6.1	0.562	13.5	LOS A	6.4	47.2	0.75	0.78	45.9
Approad	ch	1414	6.1	0.562	7.7	LOS A	6.4	47.3	0.75	0.63	47.3
East: Co	oreen Av	ve (E)									
4	L	179	15.9	0.788	50.0	LOS D	7.6	60.7	0.98	1.27	25.5
5	Т	104	3.0	1.002	104.4	LOS F	18.8	140.9	1.00	1.86	15.5
6	R	154	11.6	0.998	111.5	LOS F	18.8	140.9	1.00	1.84	15.8
Approad	ch	437	11.3	1.001	84.6	LOS F	18.8	140.9	0.99	1.62	18.5
North: C	Castlerea	agh Rd (N)									
7	L	280	3.4	0.733	9.1	LOS A	11.3	82.6	0.82	0.74	47.5
8	Т	1643	5.1	0.733	8.0	LOS A	11.4	83.5	0.82	0.72	47.2
9	R	51	18.8	0.732	16.0	LOS B	11.4	83.5	0.82	0.86	44.8
Approad	ch	1974	5.2	0.733	8.3	LOS B	11.4	83.5	0.82	0.72	47.2
West: N	Iullins R	d (W)									
10	L	41	5.1	0.117	14.0	LOS A	0.8	6.1	0.88	0.92	43.6
11	Т	32	6.7	0.200	13.8	LOS A	1.4	10.6	0.89	0.93	42.7
12	R	42	12.5	0.200	21.0	LOS B	1.4	10.6	0.89	0.98	40.0
Approad	ch	115	8.3	0.200	16.5	LOS B	1.4	10.6	0.89	0.95	41.9
All Vehi		3939	6.3	1.001	16.8	LOS B	18.8	140.9	0.81	0.79	40.1
ЛИ ИЛ	avom	ont Su	mma	F1/							

Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South: C	astlerea	agh Rd (S)									
1	L	98	3.2	0.830	11.5	LOS A	15.4	109.8	0.92	0.91	46.7
2	Т	1505	2.4	0.833	10.7	LOS A	15.4	109.8	0.93	0.92	46.0
3	R	286	7.7	0.832	18.4	LOS B	15.0	109.1	0.95	0.99	42.1
Approac	h	1889	3.3	0.832	11.9	LOS B	15.4	109.8	0.93	0.93	45.4
East: Co	reen Av	/e (E)									
4	L	157	3.4	0.364	12.3	LOS A	2.5	17.9	0.85	0.95	45.0
5	Т	127	6.6	0.475	10.4	LOS A	3.9	28.8	0.90	0.98	45.7
6	R	157	4.7	0.475	17.2	LOS B	3.9	28.8	0.90	1.03	42.6
Approac	Approach		4.8	0.475	13.5	LOS B	3.9	28.8	0.88	0.99	44.2
Approach 441 North: Castlereagh Rd (N)		agh Rd (N)									
7	L	226	2.8	0.718	10.4	LOS A	9.9	71.0	0.84	0.88	47.4
8	Т	1227	3.3	0.717	9.6	LOS A	9.9	71.0	0.84	0.88	47.0
9	R	58	5.5	0.715	17.4	LOS B	9.6	68.9	0.85	1.00	43.3
Approac	h	1512	3.3	0.718	10.0	LOS B	9.9	71.0	0.84	0.89	46.9
West: M	ullins R	d (W)									
10	L	102	2.1	0.391	18.5	LOS B	2.7	19.2	0.92	1.00	39.8
11	Т	85	2.5	0.364	13.3	LOS A	2.9	20.8	0.96	1.00	43.3
12	R	60	3.5	0.364	20.2	LOS B	2.9	20.8	0.96	1.02	40.7
Approac	h	247	2.6	0.392	17.1	LOS B	2.9	20.8	0.94	1.00	41.2
All Vehic	les	4089	3.4	0.832	11.7	LOS A	15.4	109.8	0.89	0.93	45.5

Daniel Woodriff Dr / Grace Dr



AM Movement Summary

	_	-						1.0			
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South:	Daniel W	/oodriff Dr (S)									
1	L	111	0.0	0.086	7.2	LOS A	0.7	4.8	0.55	0.31	41.9
2	Т	49	0.0	0.086	0.8	LOS A	0.7	4.8	0.55	0.00	42.6
3	R	1	0.0	0.088	7.6	LOS A	0.7	4.8	0.55	0.67	42.0
Approa	ch	161	0.0	0.086	5.3	LOS A	0.7	4.8	0.55	0.21	42.1
East: G	arace Dr	(E)									
4	L	1	0.0	0.048	10.1	LOS B	0.2	1.7	0.51	0.63	40.5
5	Т	26	0.0	0.049	8.9	LOS A	0.2	1.7	0.51	0.66	41.1
6	R	1	0.0	0.048	10.5	LOS B	0.2	1.7	0.51	0.75	40.4
Approa	ch	28	0.0	0.049	9.0	LOS B	0.2	1.7	0.51	0.66	41.1
North: [Daniel W	oodriff Dr (N)									
7	L	1	0.0	0.132	7.1	LOS A	1.0	7.2	0.31	0.58	43.0
8	Т	196	0.0	0.132	0.7	LOS A	1.0	7.2	0.31	0.00	46.1
9	R	38	0.0	0.132	7.5	LOS A	1.0	7.2	0.31	0.83	42.9
Approa	ch	235	0.0	0.132	1.8	LOS A	1.0	7.2	0.31	0.14	45.5
West: 0	Grace Dr	(W)									
10	L	3	0.0	0.632	15.8	LOS C	5.9	41.1	0.70	0.78	36.0
11	Т	12	0.0	0.579	14.5	LOS B	5.9	41.1	0.70	1.01	36.4
12	R	296	0.0	0.592	16.1	LOS C	5.9	41.1	0.70	1.09	36.0
Approa	ch	311	0.0	0.592	16.1	LOS C	5.9	41.1	0.70	1.08	36.0
All Vehi	icles	735	0.0	0.592	8.9	NA	5.9	41.1	0.54	0.57	40.1

Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South: D	Daniel W	oodriff Dr (S)									
1	L	288	0.0	0.227	7.1	LOS A	2.0	14.0	0.51	0.33	42.0
2	Т	139	0.0	0.227	0.7	LOS A	2.0	14.0	0.51	0.00	43.1
3	R	1	0.0	0.211	7.5	LOS A	2.0	14.0	0.51	0.67	42.1
Approad	ch	428	0.0	0.227	5.0	LOS A	2.0	14.0	0.51	0.23	42.3
East: G	race Dr	(E)									
4	L	1	0.0	0.033	12.4	LOS B	0.2	1.1	0.55	0.57	38.8
5	Т	13	0.0	0.033	11.1	LOS B	0.2	1.1	0.55	0.71	39.4
6	R	1	0.0	0.033	12.7	LOS B	0.2	1.1	0.55	0.75	38.7
Approad	ch	15	0.0	0.033	11.3	LOS B	0.2	1.1	0.55	0.70	39.3
North: D	Daniel W	oodriff Dr (N)									
7	L	1	0.0	0.081	8.6	LOS A	0.8	5.6	0.53	0.42	42.8
8	Т	142	0.0	0.078	2.2	LOS A	0.8	5.6	0.53	0.00	43.9
9	R	4	0.0	0.078	8.9	LOS A	0.8	5.6	0.53	0.90	42.7
Approad	ch	147	0.0	0.078	2.4	LOS A	0.8	5.6	0.53	0.03	43.8
West: G	Grace Dr	(W)									
10	L	36	0.0	0.377	13.2	LOS B	2.5	17.4	0.61	0.82	37.8
11	Т	20	0.0	0.377	11.9	LOS B	2.5	17.4	0.61	0.86	38.4
12	R	137	0.0	0.377	13.5	LOS B	2.5	17.4	0.61	0.94	37.8
Approad	ch	193	0.0	0.377	13.3	LOS B	2.5	17.4	0.61	0.91	37.8
All Vehi	cles	783	0.0	0.377	6.7	NA	2.5	17.4	0.54	0.36	41.3



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Appendix B – SIDRA Results – With Upgrades

Over a Century of Engineering Excellence

Castlereagh Rd / Peachtree Rd



AM Movement Summary

Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South: C	Castlere	agh Rd (S)									
1	L	118	5.4	0.656	27.7	LOS B	19.9	145.6	0.51	0.99	35.1
2	Т	1562	5.4	0.656	16.3	LOS B	20.5	150.2	0.52	0.48	39.9
3	R	221	4.3	0.920	100.3	LOS F	11.0	80.1	1.00	1.00	16.0
Approac	:h	1901	5.3	0.920	26.8	LOS B	20.5	150.2	0.57	0.57	33.8
East: Pe	achtree	Rd (E)									
4	L	133	4.0	0.285	53.5	LOS D	9.8	71.0	0.83	0.80	24.4
5	Т	11	0.0	0.286	45.1	LOS D	9.8	71.0	0.83	0.68	24.8
6	R	28	3.7	0.393	88.8	LOS F	3.2	22.8	1.00	0.72	17.6
Approac	:h	172	3.7	0.393	58.8	LOS E	9.8	71.0	0.86	0.78	22.9
North: C	Approach 172 North: Castlereagh Rd (N)										
7	L	140	3.8	0.321	21.1	LOS B	5.6	40.7	0.44	0.73	38.1
8	Т	2857	3.8	0.938	52.8	LOS D	76.9	556.0	1.00	1.04	23.5
9	R	122	3.4	0.632	79.5	LOS F	10.4	75.2	1.00	0.90	18.9
Approac	:h	3119	3.8	0.938	52.4	LOS D	76.9	556.0	0.98	1.02	23.7
West: P	eachtree	eRd(W)									
10	L	38	8.3	0.145	58.2	LOS E	4.8	34.8	0.84	0.77	23.3
11	Т	21	0.0	0.145	49.6	LOS D	4.8	34.8	0.84	0.65	22.3
12	R	40	7.9	0.569	90.3	LOS F	4.4	32.6	1.00	0.75	17.4
Approac	h	99	6.4	0.569	69.3	LOS E	4.8	34.8	0.90	0.74	20.3
All Vehic	cles	5291	4.4	0.938	43.7	LOS D	76.9	556.0	0.83	0.85	26.4

Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South:	Castlere	agh Rd (S)									
1	L	77	2.7	0.861	27.2	LOS B	36.1	259.0	0.66	1.00	35.8
2	Т	2322	3.0	0.860	16.0	LOS B	37.1	266.8	0.67	0.64	39.8
3	R	114	2.8	0.520	85.5	LOS F	5.7	41.0	1.00	0.75	18.0
Approa	ch	2513	3.0	0.860	19.5	LOS B	37.1	266.8	0.69	0.65	37.6
East: P	eachtree	Rd (E)									
4	L	229	2.8	0.508	57.6	LOS E	16.7	119.2	0.90	0.83	23.3
5	Т	21	0.0	0.509	49.3	LOS D	16.7	119.2	0.90	0.76	23.6
6	R	115	2.8	0.675	82.0	LOS F	10.2	73.0	1.00	0.82	18.6
Approa	ch	365	2.6	0.675	64.8	LOS E	16.7	119.2	0.93	0.83	21.6
North: (Castlerea	agh Rd (N)									
7	L	73	1.4	0.161	20.0	LOS B	3.0	21.1	0.42	0.72	38.7
8	Т	1791	2.9	0.641	30.4	LOS C	32.3	231.7	0.81	0.73	31.4
9	R	91	2.3	0.826	91.9	LOS F	8.9	63.6	1.00	0.90	17.1
Approa	ch	1954	2.8	0.826	32.8	LOS C	32.3	231.7	0.80	0.74	30.4
West: F	Peachtree	eRd(W)									
10	L	163	2.6	0.343	54.3	LOS D	11.7	83.6	0.84	0.81	24.1
11	Т	11	0.0	0.342	46.0	LOS D	11.7	83.6	0.84	0.70	23.0
12	R	146	2.9	0.861	90.2	LOS F	13.3	95.1	1.00	0.94	17.4
Approa	ch	320	2.6	0.861	70.4	LOS E	13.3	95.1	0.92	0.87	20.4
All Veh	icles	5152	2.9	0.861	30.9	LOS C	37.1	266.8	0.76	0.71	31.5

Castlereagh Rd / Coreen Ave



AM Movement Summary

				g. Satn	Average	Level of	95% Back	or Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		/eh/h					veh			per veh	km/h
South: Cas	stlereagh R	d (S)									
1	L	85	8.6	0.489	27.3	LOS B	20.7	152.9	0.66	0.93	35.8
2	Т	1487	5.9	0.489	18.7	LOS B	20.9	154.0	0.66	0.59	37.9
3	R	207	6.1	0.837	76.8	LOS F	10.7	79.0	1.00	0.87	19.4
Approach		1780	6.1	0.837	25.9	LOS B	20.9	154.0	0.70	0.64	34.0
East: Core	en Ave (E)										
4	L	179	15.9	0.820	75.5	LOS F	13.7	108.6	1.00	0.91	19.6
5	Т	104	3.0	0.417	57.1	LOS E	7.9	56.6	0.96	0.76	22.5
6	R	160	11.2	0.711	69.6	LOS E	11.9	91.0	1.00	0.85	20.8
Approach		443	11.2	0.820	69.1	LOS E	13.7	108.6	0.99	0.85	20.7
North: Cas	tlereagh R	d (N)									
7	L	366	4.0	0.237	9.0	LOS A	4.6	33.6	0.21	0.66	48.2
8	T :	2709	5.5	0.836	25.8	LOS B	47.2	345.8	0.90	0.84	33.4
9	R	35	15.2	0.236	70.7	LOS F	3.1	24.6	0.97	0.73	20.5
Approach		3111	5.4	0.836	24.4	LOS B	47.2	345.8	0.82	0.81	34.4
West: Mull	ins Rd (W)										
10	L	41	5.1	0.546	78.9	LOS F	4.3	31.3	1.00	0.75	19.0
11	Т	32	6.7	0.840	75.5	LOS F	6.4	49.0	1.00	0.89	18.5
12	R	41	12.8	0.839	84.4	LOS F	6.4	49.0	1.00	0.91	18.5
Approach		114	8.3	0.839	79.9	LOS F	6.4	49.0	1.00	0.84	18.7
All Vehicle	s	5447	6.2	0.839	29.7	LOS C	47.2	345.8	0.80	0.76	32.0

Mov ID	Tum	Demand	HV C	0eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Castlere	agh Rd (S)									
1	L	98	3.2	0.873	45.8	LOS D	50.5	362.6	0.97	0.97	27.8
2	Т	2571	2.8	0.872	34.1	LOS C	53.8	385.5	0.95	0.92	29.5
3	R	285	7.7	0.854	45.4	LOS D	9.4	70.5	0.99	0.87	26.9
Approac	h	2954	3.3	0.872	35.6	LOS C	53.8	385.5	0.96	0.92	29.2
East: Co	oreen Av	ve (E)									
4	L	157	3.4	0.562	64.3	LOS E	11.1	79.8	0.98	0.81	21.7
5	Т	127	6.6	0.443	54.7	LOS D	9.2	67.8	0.95	0.77	23.1
6	R	241	3.9	0.868	75.8	LOS F	18.0	130.0	1.00	0.96	19.6
Approac	:h	525	4.4	0.867	67.3	LOS E	18.0	130.0	0.98	0.87	21.0
North: C	astlerea	agh Rd (N)									
7	L	234	2.7	0.162	9.9	LOS A	3.9	27.9	0.25	0.66	47.3
8	Т	1595	3.4	0.647	32.3	LOS C	27.2	195.6	0.86	0.77	30.4
9	R	45	4.7	0.527	78.3	LOS F	4.3	30.9	1.00	0.74	19.1
Approac	h	1874	3.3	0.647	30.7	LOS C	27.2	195.6	0.79	0.75	31.4
West: M	Iullins R	d (W)									
10	L	102	2.1	0.659	74.0	LOS F	8.3	59.3	1.00	0.81	19.7
11	Т	85	2.5	0.909	78.4	LOS F	12.2	87.5	1.00	1.02	18.2
12	R	59	3.6	0.909	86.2	LOS F	12.2	87.5	1.00	1.02	18.3
Approac	h	246	2.6	0.910	78.5	LOS F	12.2	87.5	1.00	0.93	18.8
All Vehic	cles	5599	3.4	0.910	38.8	LOS C	53.8	385.5	0.91	0.86	28.1

Castlereagh Rd / Coreen Ave



AM Movement Summary

Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	astlere	agh Rd (S)									
1	L	85	8.6	0.520	7.4	LOS A	5.5	40.6	0.63	0.62	48.6
2	Т	1120	5.9	0.519	6.0	LOS A	5.5	40.6	0.63	0.54	48.4
3	R	208	6.1	0.518	12.9	LOS A	5.5	40.5	0.63	0.75	46.0
Approac	h	1414	6.1	0.519	7.1	LOS A	5.5	40.6	0.63	0.58	48.0
East: Co	reen A	/e (E)									
4	L	179	15.9	0.568	30.8	LOS C	6.9	54.8	1.00	1.11	32.8
5	Т	104	3.0	0.739	42.6	LOS D	11.4	85.6	1.00	1.29	27.3
6	R	154	11.6	0.741	49.7	LOS D	11.4	85.6	1.00	1.30	26.9
Approac	h	437	11.3	0.741	40.3	LOS D	11.4	85.6	1.00	1.22	29.1
North: C	North: Castlereagh Rd (N)										
7	L	280	3.4	0.821	14.6	LOS B	55.9	406.9	0.92	0.80	43.6
8	Т	1643	5.1	0.821	13.5	LOS A	55.9	407.0	0.93	0.79	43.8
9	R	51	18.8	0.818	21.5	LOS B	55.4	407.0	0.93	0.87	40.8
Approac	h	1974	5.2	0.821	13.8	LOS F	55.9	407.0	0.93	0.80	43.7
West: M	ullins R	d (W)									
10	L	41	5.1	0.105	13.4	LOS A	0.7	5.2	0.85	0.87	44.1
11	Т	32	6.7	0.179	13.1	LOS A	1.2	9.0	0.86	0.90	43.4
12	R	42	12.5	0.180	20.3	LOS B	1.2	9.0	0.86	0.97	40.4
Approac	h	115	8.3	0.180	15.8	LOS B	1.2	9.0	0.85	0.92	42.4
All Vehic	cles	3939	6.3	0.821	14.4	LOS A	55.9	407.0	0.83	0.77	42.6

PM Movement Summary

Flow Delay Service Vehicles Distance Queued Stop Rate Spee veh/h % v/c sec veh m per veh km/ South: Castlereagh Rd (S)
South: Castlereagh Rd (S) 1 L 98 3.2 0.838 11.6 LOS A 15.3 109.4 0.92 0.89 46. 2 T 1505 2.4 0.835 10.8 LOS A 15.3 109.4 0.92 0.89 46. 3 R 286 7.7 0.834 18.5 LOS B 14.9 108.1 0.94 0.97 42. Approach 1889 3.3 0.835 12.0 LOS B 15.3 109.4 0.93 0.91 45. East: Coreen Ave (E)
1 L 98 3.2 0.838 11.6 LOS A 15.3 109.4 0.92 0.89 46. 2 T 1505 2.4 0.835 10.8 LOS A 15.3 109.4 0.92 0.89 46. 3 R 286 7.7 0.834 18.5 LOS B 14.9 108.1 0.94 0.97 42. Approach 1889 3.3 0.835 12.0 LOS B 15.3 109.4 0.93 0.91 45. East: Coreen Ave (E)
2 T 1505 2.4 0.835 10.8 LOS A 15.3 109.4 0.93 0.90 46. 3 R 286 7.7 0.834 18.5 LOS B 14.9 108.1 0.94 0.97 42. Approach 1889 3.3 0.835 12.0 LOS B 15.3 109.4 0.93 0.91 45. East: Coreen Ave (E)
3 R 286 7.7 0.834 18.5 LOS B 14.9 108.1 0.94 0.97 42. Approach 1889 3.3 0.835 12.0 LOS B 15.3 109.4 0.93 0.91 45. East: Coreen Ave (E)
Approach 1889 3.3 0.835 12.0 LOS B 15.3 109.4 0.93 0.91 45. East: Coreen Ave (E)
East: Coreen Ave (E) 4 L 157 3.4 0.342 12.2 LOS A 2.6 18.9 0.88 0.94 45. 5 T 127 6.6 0.460 10.6 LOS A 4.3 31.3 0.93 0.98 45. 6 R 157 4.7 0.460 17.4 LOS B 4.3 31.3 0.93 1.00 42. Approach 441 4.8 0.460 13.6 LOS B 4.3 31.3 0.92 0.97 44. North: Castlereagh Rd (N) T T 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.97 42.
4 L 157 3.4 0.342 12.2 LOS A 2.6 18.9 0.88 0.94 45. 5 T 127 6.6 0.460 10.6 LOS A 4.3 31.3 0.93 0.98 45. 6 R 157 4.7 0.460 17.4 LOS B 4.3 31.3 0.93 0.98 45. Approach 441 4.8 0.460 17.4 LOS B 4.3 31.3 0.92 0.97 44. North: Castlereagh Rd (N) T T 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.97 42.
5 T 127 6.6 0.460 10.6 LOS A 4.3 31.3 0.93 0.98 45. 6 R 157 4.7 0.460 17.4 LOS B 4.3 31.3 0.93 1.00 42. Approach 441 4.8 0.460 13.6 LOS B 4.3 31.3 0.92 0.97 44. North: Castlereagh Rd (N) 7 L 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.96 42.
6 R 157 4.7 0.460 17.4 LOS B 4.3 31.3 0.93 1.00 42. Approach 441 4.8 0.460 13.6 LOS B 4.3 31.3 0.93 1.00 42. North: Castlereagh Rd (N) 7 L 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.97 42.
Approach 441 4.8 0.460 13.6 LOS B 4.3 31.3 0.92 0.97 44. North: Castlereagh Rd (N)
North: Castlereagh Rd (N) 7 L 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.97 42.
7 L 226 2.8 0.807 16.0 LOS B 46.4 333.9 0.94 0.96 42. 8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.96 42.
8 T 1227 3.3 0.807 15.3 LOS B 46.4 333.9 0.94 0.97 42.
9 R 58 5.5 0.807 23.3 LOS B 40.6 292.8 0.94 1.03 39.
Approach 1512 3.3 0.807 15.7 LOS F 46.4 333.9 0.94 0.97 42.
West: Mullins Rd (W)
10 L 102 2.1 0.391 18.1 LOS B 2.5 18.0 0.90 0.99 40.
11 T 85 2.5 0.365 13.0 LOS A 2.8 20.2 0.95 0.99 43.
<u>12</u> R 60 3.5 0.364 20.0 LOS B 2.8 20.2 0.95 1.02 40.
Approach 247 2.6 0.391 16.8 LOS B 2.8 20.2 0.93 1.00 41.
All Vehicles 4089 3.4 0.835 13.8 LOS A 46.4 333.9 0.93 0.95 43.

Note: Due to a reporting problem in SIDRA, the Castlereagh Rd (N) approach is reported with an overall Level of Service F. The actual Level of Service forecast is LoS A

Daniel Woodriff Dr / Grace Dr



AM Movement Summary

Mov ID	Turn	Demand	HV C	0eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Daniel Woodriff Dr		oodriff Dr (S)									
1	L	111	0.0	0.127	6.0	LOS A	1.0	6.7	0.24	0.53	43.1
2	Т	49	0.0	0.127	5.1	LOS A	1.0	6.7	0.24	0.44	43.6
3	R	1	0.0	0.132	9.3	LOS A	1.0	6.7	0.24	0.72	41.0
Approach		161	0.0	0.127	5.7	LOS A	1.0	6.7	0.24	0.50	43.2
East: G	East: Grace Dr (E)										
4	L	1	0.0	0.036	9.1	LOS A	0.3	1.8	0.63	0.66	41.4
5	Т	26	0.0	0.037	8.2	LOS A	0.3	1.8	0.63	0.62	41.7
6	R	1	0.0	0.036	12.4	LOS B	0.3	1.8	0.63	0.77	39.4
Approa	ch	28	0.0	0.037	8.4	LOS B	0.3	1.8	0.63	0.63	41.6
North: E	North: Daniel Woodrif										
7	L	1	0.0	0.263	7.9	LOS A	1.9	13.5	0.57	0.67	42.1
8	Т	196	0.0	0.247	7.1	LOS A	1.9	13.5	0.57	0.62	42.1
9	R	38	0.0	0.248	11.3	LOS B	1.9	13.5	0.57	0.79	40.1
Approach		235	0.0	0.247	7.8	LOS B	1.9	13.5	0.57	0.65	41.7
West: 6	Grace Dr	(W)									
10	L	3	0.0	0.226	5.9	LOS A	1.9	13.1	0.23	0.49	43.0
11	Т	12	0.0	0.227	5.1	LOS A	1.9	13.1	0.23	0.41	43.5
12	R	296	0.0	0.228	9.2	LOS A	1.9	13.1	0.23	0.64	40.7
Approa	ch	311	0.0	0.228	9.1	LOS A	1.9	13.1	0.23	0.63	40.8
All Vehi	icles	735	0.0	0.247	7.9	LOS A	1.9	13.5	0.35	0.61	41.6

Mov ID Turn Demand		Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h			sec		veh			per veh	km/h
South: Daniel Woodriff Dr (S)											
1	L	288	0.0	0.278	5.7	LOS A	2.5	17.7	0.13	0.53	43.6
2	Т	139	0.0	0.278	4.8	LOS A	2.5	17.7	0.13	0.43	44.2
3	R	1	0.0	0.263	9.0	LOS A	2.5	17.7	0.13	0.75	41.2
Approac	ch	428	0.0	0.278	5.4	LOS A	2.5	17.7	0.13	0.50	43.8
East: Gr	East: Grace Dr (E)										
4	L	1	0.0	0.015	7.2	LOS A	0.1	0.7	0.47	0.57	42.4
5	Т	13	0.0	0.015	6.4	LOS A	0.1	0.7	0.47	0.51	42.6
6	R	1	0.0	0.015	10.6	LOS B	0.1	0.7	0.47	0.72	40.5
Approac	ch	15	0.0	0.015	6.8	LOS B	0.1	0.7	0.47	0.53	42.4
North: D	Daniel W	oodriff Dr (N)									
7	L	1	0.0	0.132	6.6	LOS A	1.0	7.1	0.40	0.59	42.7
8	Т	142	0.0	0.135	5.8	LOS A	1.0	7.1	0.40	0.51	43.0
9	R	4	0.0	0.136	10.0	LOS A	1.0	7.1	0.40	0.76	41.0
Approac	ch	147	0.0	0.135	5.9	LOS A	1.0	7.1	0.40	0.52	42.9
West: G	Frace Dr	(W)									
10	L	36	0.0	0.170	6.6	LOS A	1.3	8.8	0.37	0.54	42.4
11	Т	20	0.0	0.169	5.7	LOS A	1.3	8.8	0.37	0.47	42.7
12	R	137	0.0	0.170	9.9	LOS A	1.3	8.8	0.37	0.67	40.4
Approac	ch	193	0.0	0.170	8.8	LOS A	1.3	8.8	0.37	0.63	41.0
All Vehicles		783	0.0	0.278	6.4	LOS A	2.5	17.7	0.24	0.53	42.9